

A P P E N D I X A

L O S M E T H O D O L O G Y



APPENDIX A

LEVEL OF SERVICE

The description and procedures for calculating capacity and level of service are found in Transportation Research Board, Highway Capacity Manual 2000. Highway Capacity Manual 2000 represents the latest research on capacity and quality of service for transportation facilities.

Quality of service requires quantitative measures to characterize operational conditions within a traffic stream. Level of service is a quality measure describing operational conditions within a traffic stream, generally in terms of such service measures as speed and travel time, freedom to maneuver, traffic interruptions, and comfort and convenience.

Six levels of service are defined for each type of facility that has analysis procedures available. Letters designate each level, from A to F, with level-of-service A representing the best operating conditions and level-of-service F the worst. Each level of service represents a range of operating conditions and the driver's perception of these conditions. Safety is not included in the measures that establish service levels.

A general description of service levels for various types of facilities is shown in Table A-1

Table A-1: Level of Service Description

	Uninterrupted Flow	Interrupted Flow
Facility Type	Freeways Multi-lane Highways Two-lane Highways Urban Streets	Signalized Intersections Unsignalized Intersections Two-way Stop Control All-way Stop Control
LOS		
A	Free-flow	Very low delay.
B	Stable flow. Presence of other users noticeable.	Low delay.
C	Stable flow. Comfort and convenience starts to decline.	Acceptable delay.
D	High density stable flow.	Tolerable delay.
E	Unstable flow.	Limit of acceptable delay.
F	Forced or breakdown flow.	Unacceptable delay

Source: Highway Capacity Manual 2000

Urban Streets

The term "urban streets" refers to urban arterials and collectors, including those in downtown areas.

Arterial streets are roads that primarily serve longer through trips. However, providing access to abutting commercial and residential land uses is also an important function of arterials.

Collector streets provide both land access and traffic circulation within residential, commercial and industrial areas. Their access function is more important than that of arterials, and unlike arterials their operation is not always dominated by traffic signals.

Downtown streets are signalized facilities that often resemble arterials. They not only move through traffic but also provide access to local businesses for passenger cars, transit buses, and trucks. Pedestrian conflicts and lane obstructions created by stopping or standing buses, trucks and parking vehicles that cause turbulence in the traffic flow are typical of downtown streets.

The speed of vehicles on urban streets is influenced by three main factors, street environment, interaction among vehicles and traffic control. As a result, these factors also affect quality of service.

The street environment includes the geometric characteristics of the facility, the character of roadside activity and adjacent land uses. Thus, the environment reflects the number and width of lanes, type of median, driveway density, spacing between signalized intersections, existence of parking, level of pedestrian activity and speed limit.

The interaction among vehicles is determined by traffic density, the proportion of trucks and buses, and turning movements. This interaction affects the operation of vehicles at intersections and, to a lesser extent, between signals.

Traffic control (including signals and signs) forces a portion of all vehicles to slow or stop. The delays and speed changes caused by traffic control devices reduce vehicle speeds, however, such controls are needed to establish right-of-way.

The average travel speed for through vehicles along an urban street is the determinant of the operating level of service. The travel speed along a segment, section or entire length of an urban street is dependent on the running speed between signalized intersections and the amount of control delay incurred at signalized intersections.

Level-of-service A describes primarily free-flow operations. Vehicles are completely unimpeded in their ability to maneuver within the traffic stream. Control delay at signalized intersections is minimal.

Level-of-service B describes reasonably unimpeded operations. The ability to maneuver within the traffic stream is only slightly restricted, and control delays at signalized intersections are not significant.

Level-of-service C describes stable operations, however, ability to maneuver and change lanes in midblock location may be more restricted than at level-of-service B. Longer queues, adverse signal coordination, or both may contribute to lower travel speeds.

Level-of-service D borders on a range in which in which small increases in flow may cause substantial increases in delay and decreases in travel speed. Level-of-service D may be due to adverse signal progression, inappropriate signal timing, high volumes, or a combination of these factors.

Level-of-service E is characterized by significant delays and lower travel speeds. Such operations are caused by a combination of adverse progression, high signal density, high volumes, extensive delays at critical intersections, and inappropriate signal timing.

Level-of-service F is characterized by urban street flow at extremely low speeds. Intersection congestion is likely at critical signalized locations, with high delays, high volumes, and extensive queuing.

The methodology to determine level of service stratifies urban streets into four classifications. The classifications are complex, and are related to functional and design categories. Table A-II describes the functional and design categories, while Table A-III relates these to the urban street classification.

Once classified, the urban street is divided into segments for analysis. An urban street segment is a one-way section of street encompassing a series of blocks or links terminating at a signalized intersection. Adjacent segments of urban streets may be combined to form larger street sections, provided that the segments have similar demand flows and characteristics.

Levels of service are related to the average travel speed of vehicles along the urban street segment or section.

Travel times for existing conditions are obtained by field measurements. The maximum-car technique is used. The vehicle is driven at the posted speed limit unless impeded by actual traffic conditions. In the maximum-car technique, a safe level of vehicular operation is maintained by observing proper following distances and by changing speeds at reasonable rates of acceleration and deceleration. The maximum-car technique provides the best base for measuring traffic performance.

An observer records the travel time and locations and duration of delay. The beginning and ending points are the centers of intersections. Delays include times waiting in queues at signalized intersections. The travel speed is determined by dividing the length of the segment by the travel time. Once the travel speed on the arterial is determined, the level of service is found by comparing the speed to the criteria in Table A-IV. Level-of-service criteria vary for the different classifications of urban street, reflecting differences in driver expectations.

Table A-II: Functional and Design Categories for Urban Streets

Criterion	Functional Category			
	Principal Arterial		Minor Arterial	
Mobility function	Very important		Important	
Access function	Very minor		Substantial	
Points connected	Freeways, important activity centers, major traffic generators		Principal arterials	
Predominant trips served	Relatively long trips between major points and through trips entering, leaving, and passing through city		Trips of moderate length within relatively small geographical areas	
Criterion	Design Category			
	High-Speed	Suburban	Intermediate	Urban
Driveway access density	Very low density	Low density	Moderate density	High density
Arterial type	Multilane divided; undivided or two-lane with shoulders	Multilane divided: undivided or two-lane with shoulders	Multilane divided or undivided; one way, two lane	Undivided one way; two way, two or more lanes
Parking	No	No	Some	Usually
Separate left-turn lanes	Yes	Yes	Usually	Some
Signals per mile	0.5 to 2	1 to 5	4 to 10	6 to 12
Speed limits	45 to 55 mph	40 to 45 mph	30 to 40 mph	25 to 35 mph
Pedestrian activity	Very little	Little	Some	Usually
Roadside development	Low density	Low to medium density	Medium to moderate density	High density

Source: Highway Capacity Manual 2000

Table A-III: Urban Street Class based on Function and Design Categories

Design Category	Functional Category	
	Principal Arterial	Minor Arterial
High-Speed	I	Not applicable
Suburban	II	II
Intermediate	II	III or IV
Urban	III or IV	IV

Source: Highway Capacity Manual 2000

Table A-IV: Urban Street Levels of Service by Class

Urban Street Class	I	II	III	IV
Range of Free Flow Speeds (mph)	45 to 55	35 to 45	30 to 35	25 to 35
Typical Free Flow Speed (mph)	50	40	33	30
Level of Service	Average Travel Speed (mph)			
A	>42	>35	>30	>25
B	>34	>28	>24	>19
C	>27	>22	>18	>13
D	>21	>17	>14	>9
E	>16	>13	>10	>7
F	≤16	≤13	≤10	≤7

Source: Highway Capacity Manual 2000

Interrupted Flow

One of the more important elements limiting, and often interrupting the flow of traffic on a highway is the intersection. Flow on an interrupted facility is usually dominated by points of fixed operation such as traffic signals, stop and yield signs. These all operate quite differently and have differing impacts on overall flow.

Signalized Intersections

The capacity of a highway is related primarily to the geometric characteristics of the facility, as well as to the composition of the traffic stream on the facility. Geometrics are a fixed, or non-varying, characteristic of a facility.

At the signalized intersection, an additional element is introduced into the concept of capacity: time allocation. A traffic signal essentially allocates time among conflicting traffic movements seeking use of the same physical space. The way in which time is allocated has a significant impact on the operation of the intersection and on the capacity of the intersection and its approaches.

Level of service for signalized intersections is defined in terms of control delay, which is a measure of driver discomfort, frustration, fuel consumption, and increased travel time. The delay experienced by a motorist is made up of a number of factors that relate to control, traffic and incidents. Total delay is the difference between the travel time actually experienced and the reference travel time that would result during base conditions, *i. e.*, in the absence of traffic control, geometric delay, any incidents, and any other vehicles. Specifically, level of service criteria for traffic signals are stated in terms of average control delay per vehicle, typically for a 15-minute analysis period. Delay is a complex measure and depends on a number of variables, including the quality of progression, the cycle length, the ratio of green time to cycle length and the volume to capacity ratio for the lane group.

For each intersection analyzed the average control delay per vehicle per approach is determined for the peak hour. A weighted average of control delay per vehicle is then determined for the intersection. A level of service designation is given to the control delay to better describe the level of operation. A description of levels of service for signalized intersections can be found in Table A-V

Table A-V: Description of Level of Service for Signalized Intersections

Level of Service	Description
A	Very low control delay, up to 10 seconds per vehicle. Progression is extremely favorable, and most vehicles arrive during the green phase. Many vehicles do not stop at all. Short cycle lengths may tend to contribute to low delay values.
B	Control delay greater than 10 and up to 20 seconds per vehicle. There is good progression or short cycle lengths or both. More vehicles stop causing higher levels of delay.
C	Control delay greater than 20 and up to 35 seconds per vehicle. Higher delays are caused by fair progression or longer cycle lengths or both. Individual cycle failures may begin to appear. Cycle failure occurs when a given green phase does not serve queued vehicles, and overflow occurs. The number of vehicles stopping is significant, though many still pass through the intersection without stopping.
D	Control delay greater than 35 and up to 55 seconds per vehicle. The influence of congestions becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, or high volumes. Many vehicles stop, the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.
E	Control delay greater than 55 and up to 80 seconds per vehicle. The limit of acceptable delay. High delays usually indicate poor progression, long cycle lengths, and high volumes. Individual cycle failures are frequent.
F	Control delay in excess of 80 seconds per vehicle. Unacceptable to most drivers. Oversaturation, arrival flow rates exceed the capacity of the intersection. Many individual cycle failures. Poor progression and long cycle lengths may also be contributing factors to higher delay.

Source: Highway Capacity Manual 2000

The use of control delay, which may also be referred to as signal delay, was introduced in the 1997 update to the *Highway Capacity Manual*, and represents a departure from previous updates. In the third edition, published in 1985 and the 1994 update to the third edition, delay only included stopped delay. Thus, the level of service criteria listed in Table A-V differs from earlier criteria.

Unsignalized Intersections

The current procedures on unsignalized intersections were first introduced in the 1997 update to the *Highway Capacity Manual* and represent a revision of the methodology published in the 1994 update to the 1985 *Highway Capacity Manual*. The revised procedures use control delay as a measure of effectiveness to determine level of service. Delay is a measure of driver discomfort, frustration, fuel consumption, and increased travel time. The delay experienced by a motorist is made up of a number of factors that relate to control, traffic and incidents. Total delay is the difference between the travel time actually experienced and the reference travel time that would result during base conditions, *i. e.*, in the absence of traffic control, geometric delay, any incidents, and any other vehicles. Control delay is the increased time of travel for a vehicle approaching and passing through an unsignalized intersection, compared with a free-flow vehicle if it were not required to slow or stop at the intersection.

Two-Way Stop Controlled Intersections

Two-way stop controlled intersections in which stop signs are used to assign the right-of-way, are the most prevalent type of intersection in the United States. At two-way stop-controlled intersections the stop-controlled approaches are referred as the minor street approaches and can be either public streets or private driveways. The approaches that are not controlled by stop signs are referred to as the major street approaches.

The capacity of movements subject to delay are determined using the "critical gap" method of capacity analysis. Expected average control delay based on movement volume and movement capacity is calculated. A level of service designation is given to the expected control delay for each minor movement. Level of service is not defined for the intersection as a whole. Control delay is the increased time of travel for a vehicle approaching and passing through a stop-controlled intersection, compared with a free-flow vehicle if it were not required

to slow or stop at the intersection. A description of levels of service for two-way stop-controlled intersections is found in Table A-VI.

Table A-VI: Description of Level of Service for Two-Way Stop Controlled Intersections

Level of Service	Description
A	Very low control delay less than 10 seconds per vehicle for each movement subject to delay.
B	Low control delay greater than 10 and up to 15 seconds per vehicle for each movement subject to delay.
C	Acceptable control delay greater than 15 and up to 25 seconds per vehicle for each movement subject to delay.
D	Tolerable control delay greater than 25 and up to 35 seconds per vehicle for each movement subject to delay.
E	Limit of tolerable control delay greater than 35 and up to 50 seconds per vehicle for each movement subject to delay.
F	Unacceptable control delay in excess of 50 seconds per vehicle for each movement subject to delay.

Source: Highway Capacity Manual 2000

A P P E N D I X B

L O S C A L C U L A T I O N S H E E T S



Menlo Park GP Circulation Update

Vistro File: J:\...\Menlo Park AM_update.vistro
Report File: J:\...\Menlo Park_AM Results.pdf

Scenario 1: Existing AM
1/9/2015

Intersection Analysis Summary

ID	Intersection Name	Control Type	Method	Worst Mvmt	V/C	Delay (s/veh)	LOS
1	Marsh Rd (SR 84)/US 101 SB Offramp	Signalized	HCM2000	SWBT	1.021	53.4	D
2	Marsh Rd/Rolison Rd-Scott Dr	Signalized	HCM2000	SWBT	0.704	95.4	F
3	Marsh Rd/Florence St-Bohannon Dr	Signalized	HCM2000	NBL	0.739	67.2	E
4	Marsh Rd/Bay Rd	Signalized	HCM2000	SBL	0.585	32.2	C
9	Middlefield Rd/Ravenswood Ave	Signalized	HCM2000	NEBL	0.647	35.5	D
10	Middlefield Rd/Ringswood Ave	Signalized	HCM2000	NWBL	0.429	33.7	C
15	Bayfront Expy (SR 84)/University Ave (SR 109)	Signalized	HCM2000	NWBL	0.957	22.1	C
16	Bayfront Expy (SR 84)/Willow Rd (SR 114)	Signalized	HCM2000	SBR	0.814	33.7	C
17	Willow Rd (SR 114)/Hamilton Ave	Signalized	HCM2000	SBL	0.557	27.6	C
18	Willow Rd (SR 114)/Ivy Dr	Signalized	HCM2000	NBL	0.543	19.0	B
19	Willow Rd (SR 114)/O'Brien Dr	Signalized	HCM2000	SBL	0.547	14.5	B
20	Willow Rd (SR 114)/Newbridge St	Signalized	HCM2000	SBL	0.785	40.6	D
21	Willow Rd/Bay Rd	Signalized	HCM2000	NEBL	0.675	20.7	C
22	Willow Rd/Durham St-VA Med Entrance	Signalized	HCM2000	WBL	0.761	14.3	B
23	Willow Rd/Coleman Ave	Signalized	HCM2000	EBL	0.954	33.8	C
24	Willow Rd/Gilbert Ave	Signalized	HCM2000	WBT	0.683	18.3	B
25	Middlefield Rd-Willow Rd	Signalized	HCM2000	NEBT	0.623	48.7	D
26	Ravenswood Ave/Laurel St	Signalized	HCM2000	SEBT	0.958	20.4	C
28	Oak Grove Ave/Laurel St	Signalized	HCM2000	SEBT	0.751	15.6	B
29	El Camino Real (SR 82)/Encinal Ave-Menlo College Entrance	Signalized	HCM2000	NWBL	0.646	18.1	B
30	El Camino Real (SR 82)/Glenwood Ave-Valparaiso Ave	Signalized	HCM2000	NWBL	0.823	35.4	D

	Ave						
31	El Camino Real (SR 82)/Oak Grove Ave	Signalized	HCM2000	NEBL	0.728	30.6	C
32	El Camino Real (SR 82)/Santa Cruz Ave	Signalized	HCM2000	NEBR	0.658	11.3	B
33	El Camino Real (SR 82)/Ravenswood Ave-Menlo Ave	Signalized	HCM2000	NWBL	0.824	37.8	D
34	El Camino Real (SR 82)/Roble Ave	Signalized	HCM2000	SEBL	0.569	8.7	A
35	El Camino Real (SR 82)/Middle Ave	Signalized	HCM2000	NWBL	0.685	16.8	B
36	El Camino Real (SR 82)/Cambridge Ave	Signalized	HCM2000	NEBL	0.591	4.3	A
38	Santa Cruz Ave/University Dr (S)	Signalized	HCM2000	SWBL	0.601	16.9	B
39	Santa Cruz Ave/Sand Hill Rd	Signalized	HCM2000	SEBL	0.782	52.6	D
58	University Avenue and Adams Drive	Two-way stop	HCM2000	EBL	0.196	146.8	F
74	University Ave/O'Brien Dr	Signalized	HCM2000	EBR	0.569	3.7	A
88	Valparaiso Ave/ University Dr	Signalized	HCM2000	NWBT	0.675	19.8	B
103	Addison Wesley/Sand Hill Rd	Signalized	HCM2000	EBT	0.668	155.5	F
107	Alpine Rd/Santa Cruz Ave&Junipero Serra Blvd	Signalized	HCM2000	WBR	0.784	46.4	D
110	Marsh Road and US 101 NB Ramps	Signalized	HCM2000	NWBL	0.681	17.1	B
132	Oak Ave/Sand Hill Rd	Signalized	HCM2000	SEBR	0.789	18.2	B
156	Saga Ln/Sand Hill Rd	Signalized	HCM2000	EBT	0.605	54.3	D
157	Branner Dr/Sand Hill Rd	Signalized	HCM2000	EBT	0.575	44.3	D
162	Sharon Park Dr/ Sand Hill Rd	Signalized	HCM2000	SWBL	0.650	29.4	C
163	Bayfront Expy/Marsh Rd	Signalized	HCM2000	WBL	1.173	140.9	F
181	Santa Cruz Ave/Elder Ave	Signalized	HCM2000	NEBL	0.545	15.4	B
195	Bayfront Expy/Chilco St	Signalized	HCM2000	NBL	0.843	18.9	B
196	Bayfront Expy/Chrysler Drive	Signalized	HCM2000	NBL	0.883	15.1	B
207	Chilco St/Constitution Dr	All-way stop	HCM2000	SBT		11.6	B
209	Jefferson Dr/Constitution Dr	Two-way stop	HCM2000	NEBT	0.000	9.6	A
213	Chrysler Dr/Independence Dr	Two-way stop	HCM2000	NWBT	0.016	10.1	B
214	Chrysler Dr/Jefferson Dr	Two-way stop	HCM2000	NWBT	0.000	12.2	B

215	Chrysler Dr/Constitution Dr	All-way stop	HCM2000	EBT		8.8	A
233	Sand Hill Road and Sand Hill Circle	Signalized	HCM2000	NBT	0.435	63.7	E
234	Sand Hill Rd/Hwy 280 NB Off-Ramp	Signalized	HCM2000	EBT	0.731	233.2	F

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value; for all other control types, they are taken for the whole intersection.

**Intersection Level Of Service Report
#1: Marsh Rd (SR 84)/US 101 SB Offramp**

Control Type:	Signalized	Delay (sec / veh):	53.4
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	1.021

Intersection Setup

Name	Marsh Road		Marsh Road (SR 84)		US 101 SB Offramp	
Approach	Northeastbound		Southwestbound		Southeastbound	
Lane Configuration	⇕⇕		⇕⇕		⇐⇐⇐⇐	
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	1
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	420.00
Speed [mph]	35.00		35.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	no		no		no	

Volumes

Name	Marsh Road		Marsh Road (SR 84)		US 101 SB Offramp	
Base Volume Input [veh/h]	0	784	1747	279	953	320
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	3.30	3.60	2.15	5.10	3.40
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	784	1747	279	953	320
Peak Hour Factor	1.0000	0.9700	0.9700	1.0000	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	202	450	70	246	82
Total Analysis Volume [veh/h]	0	808	1801	279	982	330
Presence of On-Street Parking		no	no		no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		0		0	
Bicycle Volume [bicycles/h]	4		0		0	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	80
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	57.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permissive	Permissive	Permissive	Permissive	Protected	Overlap
Signal Group	0	2	6	0	4	1
Auxiliary Signal Groups						1,4
Lead / Lag	-	-	-	-	Lead	-
Minimum Green [s]	0	8	8	0	6	4
Maximum Green [s]	0	36	36	0	36	30
Amber [s]	0.0	3.6	3.6	0.0	3.1	3.1
All red [s]	0.0	0.5	0.5	0.0	1.0	0.0
Split [s]	0	46	38	0	34	8
Vehicle Extension [s]	0.0	2.0	2.0	0.0	2.0	1.5
Walk [s]	0	0	7	0	7	0
Pedestrian Clearance [s]	0	0	16	0	22	0
I1, Start-Up Lost Time [s]	0.0	2.0	2.0	0.0	2.0	2.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		yes	yes		no	no
Maximum Recall		no	no		no	no
Pedestrian Recall		no	no		no	no
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	20.0	0.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	L	R
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	44	36	32	40
g / C, Green / Cycle	0.55	0.45	0.40	0.50
(v / s)_i Volume / Saturation Flow Rate	0.20	0.52	0.29	0.21
Total Saturation Flow Adjustment	1.05	0.92	0.88	0.82
s, saturation flow rate [veh/h]	4000	3492	3335	1562
c, Capacity [veh/h]	2200	1571	1334	781
d1, Uniform Delay [s]	10.15	22.00	20.41	12.68
k, delay calibration	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.47	73.77	3.65	1.68
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.37	1.15	0.74	0.42
d, Delay for Lane Group [s/veh]	10.62	95.77	24.06	14.35
Lane Group LOS	B	F	C	B
Critical Lane Group	no	yes	yes	yes
50th-Percentile Queue Length [veh]	5.99	41.42	11.62	5.29
50th-Percentile Queue Length [ft]	149.71	1035.62	290.57	132.22
95th-Percentile Queue Length [veh]	11.39	66.29	19.73	10.30
95th-Percentile Queue Length [ft]	284.74	1657.26	493.33	257.47

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	0.00	10.62	95.77	0.00	24.06	14.35
Movement LOS		B	F		C	B
d_A, Approach Delay [s/veh]	10.62		95.77		21.62	
Approach LOS	B		F		C	
d_I, Intersection Delay [s/veh]	53.41					
Intersection LOS	D					
Intersection V/C	1.021					

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	1	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#2: Marsh Rd/Rolison Rd-Scott Dr**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 95.4
 Level Of Service: F
 Volume to Capacity (v/c): 0.704

Intersection Setup

Name	Marsh Road			Marsh Road			Scott Drive			Rolison Road		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	2	0	0	1	0	1	0	0	0
Pocket Length [ft]	155.00	100.00	100.00	350.00	100.00	100.00	60.00	100.00	35.00	100.00	100.00	100.00
Speed [mph]	35.00			35.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Marsh Road			Marsh Road			Scott Drive			Rolison Road		
Base Volume Input [veh/h]	28	1184	14	439	1422	296	15	4	71	239	12	5
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	3.60	3.00	7.10	3.90	4.00	1.00	0.00	0.00	12.70	1.70	0.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	59	0	0	0
Total Hourly Volume [veh/h]	28	1184	14	439	1422	296	15	4	12	239	12	5
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	7	312	4	116	374	78	4	1	3	63	3	1
Total Analysis Volume [veh/h]	29	1246	15	462	1497	312	16	4	13	252	13	5
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	1			0			1			2		
Bicycle Volume [bicycles/h]	2			0			0			1		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	160
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	70.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Split	Split	Split	Split	Split	Split
Signal Group	5	2	2	1	6	6	8	3	8	4	4	4
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lead	-	-	-	-	-	Lag	-	-
Minimum Green [s]	4	10	10	4	10	10	0	6	0	4	4	4
Maximum Green [s]	4	16	16	4	16	16	0	30	0	16	16	16
Amber [s]	3.0	4.0	4.0	3.0	4.0	4.0	0.0	3.2	0.0	3.2	3.2	3.2
All red [s]	1.0	1.0	1.0	1.0	1.0	1.0	0.0	1.0	0.0	1.0	1.0	1.0
Split [s]	8	53	53	29	74	74	0	41	0	37	37	37
Vehicle Extension [s]	2.5	3.5	3.5	2.0	3.5	3.5	0.0	2.5	0.0	3.0	3.0	3.0
Walk [s]	0	7	7	0	7	7	0	8	0	8	8	8
Pedestrian Clearance [s]	0	21	21	0	21	21	0	28	0	24	24	24
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	0.0	2.0	0.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	yes		no	yes			no			no	
Pedestrian Recall	no	no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	0.0	20.0	0.0	20.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	R	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	6	51	27	72	39	39	35	35
g / C, Green / Cycle	0.04	0.32	0.17	0.45	0.24	0.24	0.22	0.22
(v / s)_i Volume / Saturation Flow Rate	0.02	0.25	0.14	0.53	0.01	0.01	0.14	0.01
Total Saturation Flow Adjustment	0.92	0.88	0.89	0.89	0.96	0.67	0.93	0.96
s, saturation flow rate [veh/h]	1742	5015	3374	3387	1827	2536	1775	1815
c, Capacity [veh/h]	65	1599	569	1524	445	618	388	397
d1, Uniform Delay [s]	75.37	49.60	64.05	44.00	46.26	45.99	56.91	49.32
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	20.34	4.03	11.93	90.99	0.19	0.06	8.15	0.22
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

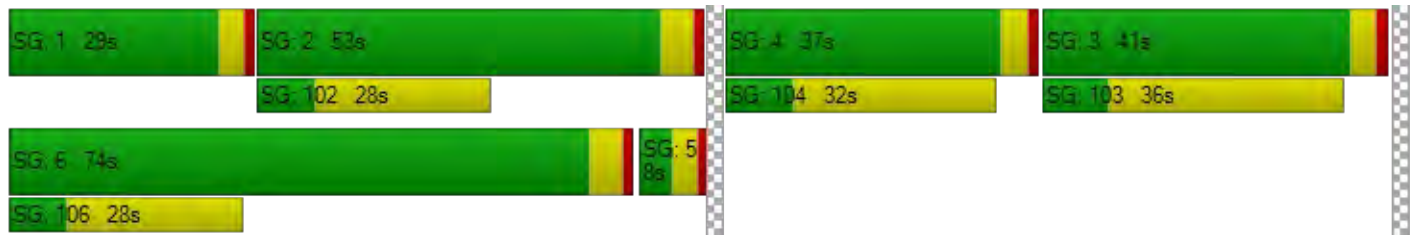
X, volume / capacity	0.44	0.79	0.81	1.19	0.04	0.02	0.65	0.05
d, Delay for Lane Group [s/veh]	95.70	53.63	75.98	134.99	46.45	46.05	65.06	49.53
Lane Group LOS	F	D	E	F	D	D	E	D
Critical Lane Group	yes	no	no	yes	yes	no	yes	no
50th-Percentile Queue Length [veh]	1.46	22.29	12.51	67.73	0.73	0.27	11.70	0.67
50th-Percentile Queue Length [ft]	36.38	557.37	312.84	1693.24	18.13	6.64	292.40	16.84
95th-Percentile Queue Length [veh]	3.42	35.93	21.05	108.37	1.79	0.68	19.84	1.67
95th-Percentile Queue Length [ft]	85.41	898.25	526.16	2709.18	44.69	16.93	496.03	41.67

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	95.70	53.63	53.63	75.98	134.99	134.99	46.45	46.45	46.05	65.06	49.53	49.53
Movement LOS	F	D	D	E	F	F	D	D	D	E	D	D
d_A, Approach Delay [s/veh]	54.58			122.99			46.29			64.03		
Approach LOS	D			F			D			E		
d_I, Intersection Delay [s/veh]	95.37											
Intersection LOS	F											
Intersection V/C	0.704											

Sequence

Ring 1	1	2	4	3	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#3: Marsh Rd/Florence St-Bohannon Dr**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 67.2
 Level Of Service: E
 Volume to Capacity (v/c): 0.739

Intersection Setup

Name	Marsh Road			Marsh Road			Florence Street			Bohannon Drive		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	↵↵↵			↵↵↵			↵↵↵			↵↵		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	1	0	0	1	0	0
Pocket Length [ft]	145.00	100.00	100.00	135.00	100.00	100.00	155.00	100.00	100.00	90.00	100.00	100.00
Speed [mph]	35.00			35.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Marsh Road			Marsh Road			Florence Street			Bohannon Drive		
Base Volume Input [veh/h]	99	767	90	27	1068	435	447	47	198	27	16	25
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	4.00	1.60	5.60	7.40	5.10	3.00	6.50	8.50	4.50	25.90	37.50	28.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	15	0	0	0
Total Hourly Volume [veh/h]	99	767	90	27	1068	435	447	47	183	27	16	25
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	26	202	24	7	281	114	118	12	48	7	4	7
Total Analysis Volume [veh/h]	104	807	95	28	1124	458	471	49	193	28	17	26
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	6			5			3			3		
Bicycle Volume [bicycles/h]	10			5			2			1		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	160
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	50.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Split	Split	Split	Split	Split	Split
Signal Group	5	2	2	1	6	6	4	3	4	8	4	8
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lag	-	-	Lag	-	-	-	-	-
Minimum Green [s]	4	12	12	4	12	12	8	8	8	0	8	0
Maximum Green [s]	30	30	30	30	30	30	30	30	30	0	30	0
Amber [s]	3.0	3.6	3.6	3.0	3.6	3.6	3.2	3.0	3.2	0.0	3.2	0.0
All red [s]	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	0.0	1.0	0.0
Split [s]	12	79	79	8	75	75	37	36	37	0	37	0
Vehicle Extension [s]	2.0	5.0	5.0	2.0	5.0	5.0	2.5	2.5	2.5	0.0	2.5	0.0
Walk [s]	0	7	7	0	7	7	7	7	7	0	7	0
Pedestrian Clearance [s]	0	19	19	0	16	16	25	25	25	0	25	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	yes		no	yes			no			no	
Pedestrian Recall	no	no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	R	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	10	77	6	73	34	34	35	35
g / C, Green / Cycle	0.06	0.48	0.04	0.46	0.21	0.21	0.22	0.22
(v / s)_i Volume / Saturation Flow Rate	0.06	0.26	0.02	0.48	0.16	0.13	0.02	0.03
Total Saturation Flow Adjustment	0.91	0.92	0.88	0.86	0.84	0.80	0.75	0.65
s, saturation flow rate [veh/h]	1736	3500	1681	3283	3190	1514	1434	1243
c, Capacity [veh/h]	108	1684	63	1498	678	322	314	272
d1, Uniform Delay [s]	74.79	29.00	75.37	43.50	59.28	56.86	49.80	50.58
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	75.84	1.22	21.06	39.65	8.12	8.03	0.56	1.24
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

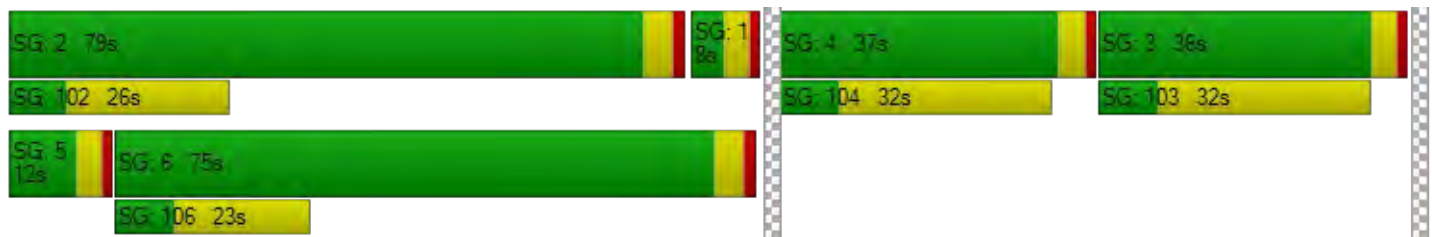
X, volume / capacity	0.96	0.54	0.44	1.06	0.77	0.60	0.09	0.16
d, Delay for Lane Group [s/veh]	150.64	30.23	96.43	83.15	67.40	64.89	50.36	51.82
Lane Group LOS	F	C	F	F	E	E	D	D
Critical Lane Group	yes	no	no	yes	yes	no	no	yes
50th-Percentile Queue Length [veh]	6.51	16.46	1.41	52.24	13.67	8.83	1.07	1.67
50th-Percentile Queue Length [ft]	162.85	411.56	35.18	1306.06	341.65	220.66	26.65	41.87
95th-Percentile Queue Length [veh]	12.19	26.95	3.31	83.59	22.75	15.63	2.57	3.88
95th-Percentile Queue Length [ft]	304.82	673.80	82.84	2089.74	568.86	390.82	64.16	96.95

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	150.64	30.23	30.23	96.43	83.15	83.15	67.40	67.40	64.89	50.36	51.82	51.82
Movement LOS	F	C	C	F	F	F	E	E	E	D	D	D
d_A, Approach Delay [s/veh]	42.67			83.38			66.72			51.24		
Approach LOS	D			F			E			D		
d_I, Intersection Delay [s/veh]	67.17											
Intersection LOS	E											
Intersection V/C	0.739											

Sequence

Ring 1	1	2	4	3	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#4: Marsh Rd/Bay Rd**

Control Type:	Signalized	Delay (sec / veh):	32.2
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.585

Intersection Setup

Name	Marsh Road			Marsh Road			Bay Road			Bay Road		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	⇌⇌			⇌⇌			+			+		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	1	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	260.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	35.00			35.00			25.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Marsh Road			Marsh Road			Bay Road			Bay Road		
Base Volume Input [veh/h]	1	803	82	268	867	31	114	78	7	45	19	173
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	1.20	2.40	7.10	6.20	3.20	3.50	2.60	0.00	0.00	5.30	4.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	1	803	82	268	867	31	114	78	7	45	19	173
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	211	22	71	228	8	30	21	2	12	5	46
Total Analysis Volume [veh/h]	1	845	86	282	913	33	120	82	7	47	20	182
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0			6			4			0		
Bicycle Volume [bicycles/h]	4			4			6			8		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	80
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	8.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	2	6	2	5	2	6	4	4	4	8	4	8
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lead	-	-	Lag	-	-	-	-	-
Minimum Green [s]	6	6	6	4	6	6	6	6	6	0	6	0
Maximum Green [s]	30	29	30	30	30	29	19	19	19	0	19	0
Amber [s]	3.6	3.6	3.6	3.1	3.6	3.6	3.6	3.6	3.6	0.0	3.6	0.0
All red [s]	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	0.0	1.0	0.0
Split [s]	48	50	48	19	48	50	32	32	32	0	32	0
Vehicle Extension [s]	3.0	3.0	3.0	2.0	3.0	3.0	2.5	2.5	2.5	0.0	2.5	0.0
Walk [s]	7	0	7	0	7	0	7	7	7	0	7	0
Pedestrian Clearance [s]	12	0	12	0	12	0	20	20	20	0	20	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no		no	no			no			no	
Maximum Recall		yes		no	yes			no			no	
Pedestrian Recall		no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	L	C	C	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	0.00	2.00	2.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	48	17	46	30	30
g / C, Green / Cycle	0.48	0.17	0.46	0.30	0.30
(v / s)_i Volume / Saturation Flow Rate	0.27	0.17	0.28	0.20	0.17
Total Saturation Flow Adjustment	0.91	0.89	0.89	0.56	0.78
s, saturation flow rate [veh/h]	3449	1685	3387	1068	1479
c, Capacity [veh/h]	1639	284	1543	317	439
d1, Uniform Delay [s]	19.06	41.95	20.78	31.03	30.01
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	1.44	51.97	1.83	10.28	5.22
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.57	0.99	0.61	0.66	0.57
d, Delay for Lane Group [s/veh]	20.49	93.92	22.61	41.30	35.23
Lane Group LOS	C	F	C	D	D
Critical Lane Group	no	yes	no	yes	no
50th-Percentile Queue Length [veh]	11.30	12.05	12.16	6.12	6.78
50th-Percentile Queue Length [ft]	282.52	301.24	303.98	153.06	169.60
95th-Percentile Queue Length [veh]	19.26	20.36	20.52	11.60	12.60
95th-Percentile Queue Length [ft]	481.51	509.05	513.08	289.88	315.03

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	20.49	20.49	20.49	93.92	22.61	22.61	41.30	41.30	41.30	35.23	35.23	35.23
Movement LOS	C	C	C	F	C	C	D	D	D	D	D	D
d_A, Approach Delay [s/veh]	20.49			38.99			41.30			35.23		
Approach LOS	C			D			D			D		
d_I, Intersection Delay [s/veh]	32.23											
Intersection LOS	C											
Intersection V/C	0.585											

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	5	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#9: Middlefield Rd/Ravenswood Ave**

Control Type:	Signalized	Delay (sec / veh):	35.5
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.647

Intersection Setup

Name	Ravenswood Avenue		Middlefield Road		Middlefield Road	
Approach	Northeastbound		Northwestbound		Southeastbound	
Lane Configuration						
Turning Movement	Left	Right	Left	Thru	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	1	0	0	0	0
Pocket Length [ft]	100.00	120.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00		35.00		35.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Ravenswood Avenue		Middlefield Road		Middlefield Road	
Base Volume Input [veh/h]	85	522	446	364	420	75
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	11.80	4.20	3.10	2.50	3.30	6.70
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	1000	0	0	0	0
Total Hourly Volume [veh/h]	85	0	446	364	420	75
Peak Hour Factor	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	24	0	124	101	117	21
Total Analysis Volume [veh/h]	94	0	496	404	467	83
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	2		21		0	
Bicycle Volume [bicycles/h]	22		39		37	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	160
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	61.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Protected	Permissive	Protected	Permissive	Permissive	Permissive
Signal Group	3	2	1	6	2	4
Auxiliary Signal Groups						
Lead / Lag	Lead	-	Lead	-	-	-
Minimum Green [s]	4	10	4	10	10	0
Maximum Green [s]	30	26	19	42	26	0
Amber [s]	3.2	3.6	3.0	3.6	3.6	0.0
All red [s]	1.0	1.0	1.0	1.0	1.0	0.0
Split [s]	24	71	65	136	71	0
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	0.0
Walk [s]	7	7	0	7	7	0
Pedestrian Clearance [s]	12	12	0	12	12	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	no		no	no	no	
Maximum Recall	no		no	yes	yes	
Pedestrian Recall	no		no	no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	6.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	R	L	C	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	22	22	63	134	69
g / C, Green / Cycle	0.14	0.14	0.39	0.84	0.43
(v / s)_i Volume / Saturation Flow Rate	0.06	0.00	0.28	0.22	0.31
Total Saturation Flow Adjustment	0.85	0.82	0.92	0.98	0.95
s, saturation flow rate [veh/h]	1614	1550	1751	1854	1798
c, Capacity [veh/h]	222	213	689	1552	775
d1, Uniform Delay [s]	63.19	59.51	41.03	2.70	37.28
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	5.82	0.00	6.38	0.41	5.44
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00

Lane Group Results

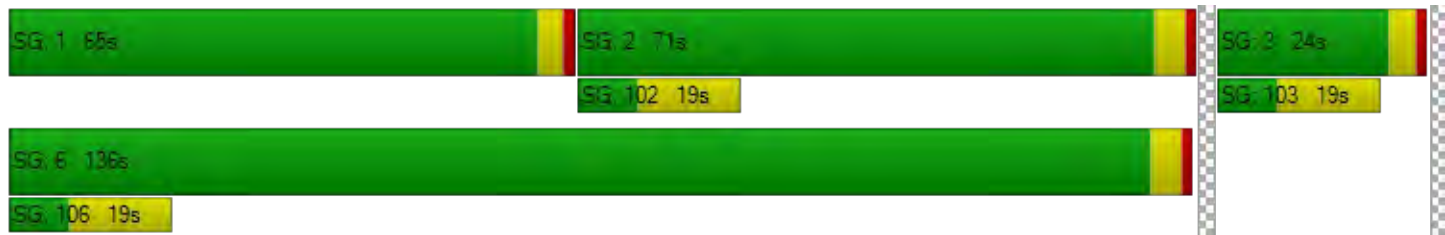
X, volume / capacity	0.42	0.00	0.72	0.26	0.71
d, Delay for Lane Group [s/veh]	69.02	59.51	47.40	3.11	42.73
Lane Group LOS	E	E	D	A	D
Critical Lane Group	yes	no	yes	no	yes
50th-Percentile Queue Length [veh]	4.25	0.00	21.65	4.54	23.17
50th-Percentile Queue Length [ft]	106.31	0.00	541.31	113.60	579.27
95th-Percentile Queue Length [veh]	8.62	0.00	34.93	9.10	37.30
95th-Percentile Queue Length [ft]	215.51	0.00	873.21	227.55	932.46

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	69.02	59.51	47.40	3.11	42.73	42.73
Movement LOS	E	E	D	A	D	D
d_A, Approach Delay [s/veh]	69.02		27.52		42.73	
Approach LOS	E		C		D	
d_I, Intersection Delay [s/veh]	35.46					
Intersection LOS	D					
Intersection V/C	0.647					

Sequence

Ring 1	1	2	3	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#10: Middlefield Rd/Ringswood Ave**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 33.7
 Level Of Service: C
 Volume to Capacity (v/c): 0.429

Intersection Setup

Name	D Street			Ringwood Avenue			Middlefield Road			Middlefield Road		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	↵↵			↵↵			↵↵↵			↵↵		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	0	0	1	1	0	1	1	0	0
Pocket Length [ft]	30.00	100.00	100.00	100.00	100.00	250.00	175.00	100.00	100.00	125.00	100.00	100.00
Speed [mph]	20.00			30.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	D Street			Ringwood Avenue			Middlefield Road			Middlefield Road		
Base Volume Input [veh/h]	5	3	12	136	44	251	55	559	84	216	653	62
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	8.30	4.40	0.00	4.00	0.00	3.20	0.00	4.60	4.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	222	0	0	96	0	0	0
Total Hourly Volume [veh/h]	5	3	12	136	44	29	55	559	0	216	653	62
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	1	1	3	36	12	8	14	147	0	57	172	16
Total Analysis Volume [veh/h]	5	3	13	143	46	31	58	588	0	227	687	65
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	6			27			5			11		
Bicycle Volume [bicycles/h]	6			23			39			34		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	160
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	61.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	2	8	2	6	8	6	5	2	8	1	6	4
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	10	6	10	10	6	10	4	10	6	4	10	0
Maximum Green [s]	30	16	30	30	16	30	30	30	16	30	30	0
Amber [s]	3.6	3.2	3.6	3.6	3.2	3.6	3.5	3.6	3.2	3.0	3.6	0.0
All red [s]	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	0.0
Split [s]	62	49	62	97	49	97	14	62	49	49	97	0
Vehicle Extension [s]	3.6	2.9	3.6	3.0	2.9	3.0	3.0	3.6	2.9	3.0	3.0	0.0
Walk [s]	7	7	7	7	7	7	0	7	7	0	7	0
Pedestrian Clearance [s]	12	21	12	12	21	12	0	12	21	0	12	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	yes		no	yes	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	C	R	L	C	R	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	47	47	47	47	12	60	60	47	95
g / C, Green / Cycle	0.29	0.29	0.29	0.29	0.08	0.38	0.38	0.29	0.59
(v / s)_i Volume / Saturation Flow Rate	0.01	0.01	0.13	0.02	0.03	0.17	0.00	0.13	0.22
Total Saturation Flow Adjustment	0.47	0.87	0.77	0.80	0.95	0.92	0.85	0.91	0.90
s, saturation flow rate [veh/h]	885	1650	1459	1512	1805	3505	1615	1726	3433
c, Capacity [veh/h]	260	485	428	444	135	1315	606	507	2039
d1, Uniform Delay [s]	40.13	40.29	45.84	40.74	70.72	37.55	31.25	45.95	16.91
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.14	0.13	3.27	0.30	9.61	1.10	0.00	2.85	0.52
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.02	0.03	0.44	0.07	0.43	0.45	0.00	0.45	0.37
d, Delay for Lane Group [s/veh]	40.27	40.42	49.12	41.04	80.33	38.65	31.25	48.79	17.42
Lane Group LOS	D	D	D	D	F	D	C	D	B
Critical Lane Group	no	no	yes	no	no	yes	no	yes	no
50th-Percentile Queue Length [veh]	0.17	0.54	7.54	1.07	2.77	11.35	0.00	9.05	10.17
50th-Percentile Queue Length [ft]	4.27	13.56	188.58	26.65	69.25	283.79	0.00	226.14	254.15
95th-Percentile Queue Length [veh]	0.44	1.35	13.74	2.57	6.02	19.33	0.00	15.95	17.60
95th-Percentile Queue Length [ft]	10.96	33.85	343.44	64.17	150.59	483.37	0.00	398.86	439.92

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	40.27	40.42	40.42	49.12	49.12	41.04	80.33	38.65	31.25	48.79	17.42	17.42
Movement LOS	D	D	D	D	D	D	F	D	C	D	B	B
d_A, Approach Delay [s/veh]	40.38			47.98			42.39			24.70		
Approach LOS	D			D			D			C		
d_I, Intersection Delay [s/veh]	33.74											
Intersection LOS	C											
Intersection V/C	0.429											

Sequence




Ring 1	1	2	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#15: Bayfront Expy (SR 84)/University Ave (SR 109)**

Control Type:	Signalized	Delay (sec / veh):	22.1
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.957

Intersection Setup

Name	Bayfront Expy (SR84)		Bayfront Expy (SR84)		University Avenue (SR109)	
Approach	Northeastbound		Southwestbound		Northwestbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	1	2	0	1	2
Pocket Length [ft]	100.00	430.00	830.00	100.00	175.00	1000.00
Speed [mph]	55.00		55.00		35.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Bayfront Expy (SR84)		Bayfront Expy (SR84)		University Avenue (SR109)	
Base Volume Input [veh/h]	873	85	1495	3915	185	359
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	5.40	3.50	1.60	3.10	2.20	3.60
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	873	85	1495	3915	185	359
Peak Hour Factor	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	227	22	389	1020	48	93
Total Analysis Volume [veh/h]	909	89	1557	4078	193	374
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		4		8	
Bicycle Volume [bicycles/h]	1		0		0	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	160
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	9.00

Phasing & Timing

Control Type	Permissive	Permissive	Protected	Permissive	Protected	Overlap
Signal Group	2	2	1	6	4	1
Auxiliary Signal Groups						1,4
Lead / Lag	-	-	Lead	-	Lead	-
Minimum Green [s]	4	4	4	4	4	4
Maximum Green [s]	16	16	4	24	16	4
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	44	44	102	146	14	102
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	0	1	1	0
Pedestrian Clearance [s]	1	1	0	1	1	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no		no	no	no	no
Maximum Recall	no		no	no	no	no
Pedestrian Recall	no		no	no	no	no
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	6.0	6.0	20.0	6.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	L	C	L	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	40	40	98	142	10	112
g / C, Green / Cycle	0.25	0.25	0.61	0.89	0.06	0.70
(v / s)_i Volume / Saturation Flow Rate	0.19	0.06	0.45	0.81	0.06	0.09
Total Saturation Flow Adjustment	0.86	0.82	0.91	0.88	0.90	0.73
s, saturation flow rate [veh/h]	4910	1560	3450	5020	3430	4139
c, Capacity [veh/h]	1228	390	2113	4455	214	2897
d1, Uniform Delay [s]	55.22	47.72	21.89	5.40	74.50	7.92
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	4.04	1.36	2.34	3.96	40.06	0.09
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.74	0.23	0.74	0.92	0.90	0.13
d, Delay for Lane Group [s/veh]	59.27	49.08	24.23	9.35	114.57	8.01
Lane Group LOS	E	D	C	A	F	A
Critical Lane Group	no	no	no	yes	no	yes
50th-Percentile Queue Length [veh]	16.05	3.41	29.67	54.17	5.94	2.33
50th-Percentile Queue Length [ft]	401.19	85.14	741.79	1354.31	148.47	58.34
95th-Percentile Queue Length [veh]	26.32	7.17	47.55	86.68	11.31	5.20
95th-Percentile Queue Length [ft]	658.10	179.31	1188.82	2166.92	282.82	129.92

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	59.27	49.08	24.23	9.35	114.57	8.01
Movement LOS	E	D	C	A	F	A
d_A, Approach Delay [s/veh]	58.36		13.46		44.28	
Approach LOS	E		B		D	
d_I, Intersection Delay [s/veh]	22.11					
Intersection LOS	C					
Intersection V/C	0.957					

Sequence

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#16: Bayfront Expy (SR 84)/Willow Rd (SR 114)**

Control Type:	Signalized	Delay (sec / veh):	33.7
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.814

Intersection Setup

Name	Willow Road (SR 114)			Willow Road			Bayfront Expy (SR 84)			Bayfront Expy (SR 84)		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	3	0	0	1	0	1	2	0	1	2	0	0
Pocket Length [ft]	265.00	100.00	100.00	45.00	100.00	80.00	165.00	100.00	140.00	1000.00	100.00	100.00
Speed [mph]	40.00			20.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road (SR 114)			Willow Road			Bayfront Expy (SR 84)			Bayfront Expy (SR 84)		
Base Volume Input [veh/h]	128	380	361	16	59	42	217	713	155	732	2780	24
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	10.90	4.20	10.20	37.50	30.50	40.50	4.60	6.20	12.30	6.70	3.80	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	16	0	0	106	0	0	0
Total Hourly Volume [veh/h]	128	380	361	16	59	26	217	713	49	732	2780	24
Peak Hour Factor	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	35	104	99	4	16	7	60	196	13	201	764	7
Total Analysis Volume [veh/h]	141	418	397	18	65	29	238	784	54	804	3055	26
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	5			2			12			2		
Bicycle Volume [bicycles/h]	17			6			0			0		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	12.00

Phasing & Timing

Control Type	Permiss	Permiss	Overlap	Split	Split	Split	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	8	8	1	4	4	4	5	2	2	1	6	6
Auxiliary Signal Groups			1,8									
Lead / Lag	Lag	-	-	Lag	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	4	16	16	16	4	16	16	4	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	23	23	63	8	8	8	15	36	36	63	84	84
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	0	1	1	1	0	1	1	0	1	1
Pedestrian Clearance [s]	1	1	0	1	1	1	0	1	1	0	1	1
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no	no		no		no	no		no	no	
Maximum Recall		no	no		no		no	no		no	no	
Pedestrian Recall		no	no		no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	6.0	6.0	20.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	R	L	C	R	L	C	R	L	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	2.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	0.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	19	19	82	4	4	4	11	32	32	59	80	80
g / C, Green / Cycle	0.15	0.15	0.63	0.03	0.03	0.03	0.08	0.25	0.25	0.45	0.62	0.62
(v / s)_i Volume / Saturation Flow Rate	0.09	0.12	0.12	0.01	0.02	0.03	0.07	0.13	0.04	0.24	0.52	0.02
Total Saturation Flow Adjustment	0.86	0.91	0.88	0.69	0.73	0.60	0.88	1.02	0.76	0.86	1.03	0.85
s, saturation flow rate [veh/h]	1628	3472	3362	1313	2772	1149	3351	5836	1438	3285	5846	1615
c, Capacity [veh/h]	238	507	2121	40	85	35	284	1437	354	1491	3598	994
d1, Uniform Delay [s]	51.88	53.87	10.05	61.91	62.53	62.64	58.63	42.67	38.38	25.67	20.14	9.77
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	10.42	14.08	0.20	31.78	46.98	102.38	24.71	1.49	0.91	1.40	2.71	0.05
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.59	0.82	0.19	0.45	0.76	0.82	0.84	0.55	0.15	0.54	0.85	0.03
d, Delay for Lane Group [s/veh]	62.30	67.96	10.24	93.69	109.50	165.03	83.34	44.17	39.29	27.07	22.85	9.82
Lane Group LOS	E	E	B	F	F	F	F	D	D	C	C	A
Critical Lane Group	no	yes	no	no	no	yes	yes	no	no	no	yes	no
50th-Percentile Queue Length [veh]	5.50	9.73	3.78	0.76	1.63	1.46	5.73	10.15	1.66	12.20	40.34	0.41
50th-Percentile Queue Length [ft]	137.54	243.22	94.40	18.98	40.87	36.62	143.23	253.63	41.39	305.10	1008.57	10.16
95th-Percentile Queue Length [veh]	10.63	16.96	7.82	1.87	3.79	3.44	10.99	17.57	3.84	20.59	64.56	1.03
95th-Percentile Queue Length [ft]	265.83	423.90	195.40	46.67	94.86	85.90	274.72	439.14	95.94	514.74	1614.03	25.63

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	62.30	67.96	10.24	93.69	109.50	165.03	83.34	44.17	39.29	27.07	22.85	9.82
Movement LOS	E	E	B	F	F	F	F	D	D	C	C	A
d_A, Approach Delay [s/veh]	43.16			121.34			52.59			23.64		
Approach LOS	D			F			D			C		
d_I, Intersection Delay [s/veh]	33.71											
Intersection LOS	C											
Intersection V/C	0.814											

Sequence

Ring 1	1	2	-	4	-	8	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#17: Willow Rd (SR 114)/Hamilton Ave**

Control Type:	Signalized	Delay (sec / veh):	27.6
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.557

Intersection Setup

Name	Willow Road (SR 114)			Willow Road (SR 114)			Hamilton Avenue			Hamilton Avenue		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	↵ ↑			↵ ↑			↑			↑		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	0	0	0	0	0	0
Pocket Length [ft]	190.00	100.00	100.00	115.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	40.00			30.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road (SR 114)			Willow Road (SR 114)			Hamilton Avenue			Hamilton Avenue		
Base Volume Input [veh/h]	124	807	71	88	795	57	78	22	50	26	24	23
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.60	6.30	7.00	9.10	8.40	10.50	1.30	4.50	6.00	23.10	12.50	30.40
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	124	807	71	88	795	57	78	22	50	26	24	23
Peak Hour Factor	0.8500	0.8500	0.8500	0.8500	0.8500	0.8500	0.8500	0.8500	0.8500	0.8500	0.8500	0.8500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	36	237	21	26	234	17	23	6	15	8	7	7
Total Analysis Volume [veh/h]	146	949	84	104	935	67	92	26	59	31	28	27
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	11			16			10			7		
Bicycle Volume [bicycles/h]	15			7			8			1		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	9.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	5	2	2	1	6	6	4	4	4	8	8	8
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	4	16	16	4	16	16	16	16	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	23	77	77	19	73	73	34	34	34	34	34	34
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	0	1	1	0	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	0	1	1	0	1	1	1	1	1	1	1	1
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	no		no	no			no			no	
Pedestrian Recall	no	no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	6.0	6.0	20.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	2.00	2.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	19	73	15	69	30	30
g / C, Green / Cycle	0.15	0.56	0.12	0.53	0.23	0.23
(v / s)_i Volume / Saturation Flow Rate	0.08	0.31	0.06	0.30	0.13	0.06
Total Saturation Flow Adjustment	0.94	0.88	0.87	0.87	0.70	0.73
s, saturation flow rate [veh/h]	1777	3362	1654	3304	1330	1381
c, Capacity [veh/h]	260	1888	191	1754	307	319
d1, Uniform Delay [s]	51.63	18.04	54.28	20.54	44.36	41.02
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	8.54	1.15	10.72	1.36	7.68	2.07
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.56	0.55	0.54	0.57	0.58	0.27
d, Delay for Lane Group [s/veh]	60.17	19.19	65.00	21.90	52.04	43.09
Lane Group LOS	E	B	E	C	D	D
Critical Lane Group	yes	no	no	yes	yes	no
50th-Percentile Queue Length [veh]	5.61	14.12	4.07	14.59	6.51	2.79
50th-Percentile Queue Length [ft]	140.21	353.08	101.85	364.83	162.71	69.77
95th-Percentile Queue Length [veh]	10.80	23.43	8.32	24.14	12.18	6.06
95th-Percentile Queue Length [ft]	270.01	585.87	208.06	603.43	304.60	151.56

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	60.17	19.19	19.19	65.00	21.90	21.90	52.04	52.04	52.04	43.09	43.09	43.09
Movement LOS	E	B	B	E	C	C	D	D	D	D	D	D
d_A, Approach Delay [s/veh]	24.26			25.95			52.04			43.09		
Approach LOS	C			C			D			D		
d_I, Intersection Delay [s/veh]	27.56											
Intersection LOS	C											
Intersection V/C	0.557											

Sequence

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#18: Willow Rd (SR 114)/Ivy Dr**

Control Type:	Signalized	Delay (sec / veh):	19.0
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.543

Intersection Setup

Name	Willow Road (SR 114)		Willow Road (SR 114)		Ivy Drive	
Approach	Northbound		Southbound		Eastbound	
Lane Configuration	↩		↩		↩↪	
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	0	1	0
Pocket Length [ft]	85.00	100.00	100.00	100.00	135.00	100.00
Speed [mph]	40.00		40.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Willow Road (SR 114)		Willow Road (SR 114)		Ivy Drive	
Base Volume Input [veh/h]	129	1141	842	9	17	147
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.30	5.70	10.30	22.20	0.00	6.10
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	129	1141	842	9	17	147
Peak Hour Factor	0.8500	0.8500	0.8500	0.8500	0.8500	0.8500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	38	336	248	3	5	43
Total Analysis Volume [veh/h]	152	1342	991	11	20	173
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	14		18		7	
Bicycle Volume [bicycles/h]	16		6		3	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	9.00

Phasing & Timing

Control Type	Protected	Permissive	Permissive	Permissive	Split	Split
Signal Group	5	2	6	6	4	4
Auxiliary Signal Groups						
Lead / Lag	Lead	-	-	-	Lag	-
Minimum Green [s]	4	4	4	4	4	4
Maximum Green [s]	4	24	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	24	99	75	75	31	31
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	0	1	1	1	1	1
Pedestrian Clearance [s]	0	1	1	1	1	1
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no	no	no		no	
Maximum Recall	no	no	no		no	
Pedestrian Recall	no	no	no		no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	6.0	6.0	6.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	C	L	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	20	95	71	27	27
g / C, Green / Cycle	0.15	0.73	0.55	0.21	0.21
(v / s)_i Volume / Saturation Flow Rate	0.09	0.39	0.31	0.01	0.11
Total Saturation Flow Adjustment	0.93	0.90	0.86	0.95	0.80
s, saturation flow rate [veh/h]	1764	3423	3274	1805	1522
c, Capacity [veh/h]	271	2501	1788	375	316
d1, Uniform Delay [s]	50.93	7.75	19.29	41.26	46.04
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	8.11	0.83	1.27	0.27	6.66
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.56	0.54	0.56	0.05	0.55
d, Delay for Lane Group [s/veh]	59.03	8.58	20.57	41.53	52.70
Lane Group LOS	E	A	C	D	D
Critical Lane Group	yes	no	yes	no	yes
50th-Percentile Queue Length [veh]	5.80	13.29	14.17	0.62	6.35
50th-Percentile Queue Length [ft]	145.06	332.24	354.28	15.51	158.73
95th-Percentile Queue Length [veh]	11.10	22.20	23.51	1.54	11.94
95th-Percentile Queue Length [ft]	277.54	554.88	587.66	38.52	298.55

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	59.03	8.58	20.57	20.57	41.53	52.70
Movement LOS	E	A	C	C	D	D
d_A, Approach Delay [s/veh]	13.71		20.57		51.54	
Approach LOS	B		C		D	
d_I, Intersection Delay [s/veh]	18.98					
Intersection LOS	B					
Intersection V/C	0.543					

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#19: Willow Rd (SR 114)/O'Brien Dr**

Control Type: Signalized
Analysis Method: HCM2000
Analysis Period: 15 minutes

Delay (sec / veh): 14.5
Level Of Service: B
Volume to Capacity (v/c): 0.547

Intersection Setup

Name	Willow Road (SR 114)		Willow Road (SR 114)		O'Brien Drive	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	1	1	0	1	0
Pocket Length [ft]	100.00	60.00	100.00	100.00	50.00	100.00
Speed [mph]	40.00		40.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Willow Road (SR 114)		Willow Road (SR 114)		O'Brien Drive	
Base Volume Input [veh/h]	1201	304	54	969	195	58
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	5.40	5.30	7.40	9.70	10.30	8.60
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	1201	304	54	969	195	58
Peak Hour Factor	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	330	84	15	266	54	16
Total Analysis Volume [veh/h]	1320	334	59	1065	214	64
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		1		21	
Bicycle Volume [bicycles/h]	27		1		0	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	9.00

Phasing & Timing

Control Type	Permissive	Permissive	Protected	Permissive	Split	Split
Signal Group	2	2	1	6	8	8
Auxiliary Signal Groups						
Lead / Lag	-	-	Lead	-	Lag	-
Minimum Green [s]	4	4	4	4	4	4
Maximum Green [s]	16	16	4	24	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	92	92	13	105	25	25
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	0	1	1	1
Pedestrian Clearance [s]	1	1	0	1	1	1
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no		no	no	no	
Maximum Recall	no		no	no	no	
Pedestrian Recall	no		no	no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	6.0	6.0	20.0	6.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	L	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	88	88	9	101	21
g / C, Green / Cycle	0.68	0.68	0.07	0.78	0.16
(v / s)_i Volume / Saturation Flow Rate	0.38	0.22	0.04	0.32	0.09
Total Saturation Flow Adjustment	0.90	0.81	0.88	0.87	0.81
s, saturation flow rate [veh/h]	3432	1534	1681	3298	3097
c, Capacity [veh/h]	2323	1038	116	2562	500
d1, Uniform Delay [s]	11.02	8.67	58.36	4.78	50.20
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	1.01	0.82	14.91	0.50	4.40
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.57	0.32	0.51	0.42	0.56
d, Delay for Lane Group [s/veh]	12.04	9.49	73.27	5.28	54.61
Lane Group LOS	B	A	E	A	D
Critical Lane Group	yes	no	yes	no	yes
50th-Percentile Queue Length [veh]	15.31	5.70	2.38	7.93	5.55
50th-Percentile Queue Length [ft]	382.65	142.38	59.45	198.31	138.73
95th-Percentile Queue Length [veh]	25.21	10.94	5.28	14.32	10.71
95th-Percentile Queue Length [ft]	630.16	273.38	132.06	357.88	267.69

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	12.04	9.49	73.27	5.28	54.61	54.61
Movement LOS	B	A	E	A	D	D
d_A, Approach Delay [s/veh]	11.52		8.85		54.61	
Approach LOS	B		A		D	
d_I, Intersection Delay [s/veh]	14.46					
Intersection LOS	B					
Intersection V/C	0.547					

Sequence

Ring 1	1	2	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#20: Willow Rd (SR 114)/Newbridge St**

Control Type:	Signalized	Delay (sec / veh):	40.6
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.785

Intersection Setup

Name	Willow Road (SR 114)			Willow Road (SR 114)			Newbridge Street			Newbridge Street		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	1	0	1	0	0	0
Pocket Length [ft]	390.00	100.00	100.00	185.00	100.00	100.00	100.00	100.00	175.00	100.00	100.00	100.00
Speed [mph]	40.00			25.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road (SR 114)			Willow Road (SR 114)			Newbridge Street			Newbridge Street		
Base Volume Input [veh/h]	151	1407	166	51	1132	10	37	169	281	273	120	42
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	5.70	6.60	2.00	10.00	30.00	10.80	4.10	1.80	2.90	7.50	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	44	0	0	34
Total Hourly Volume [veh/h]	151	1407	166	51	1132	10	37	169	237	273	120	8
Peak Hour Factor	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	41	378	45	14	304	3	10	45	64	73	32	2
Total Analysis Volume [veh/h]	162	1513	178	55	1217	11	40	182	255	294	129	9
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	24			39			14			15		
Bicycle Volume [bicycles/h]	11			5			6			1		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	12.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Split	Split	Split	Split	Split	Split
Signal Group	5	2	2	1	6	6	4	4	4	8	8	8
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	4	16	16	4	16	16	16	16	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	19	72	72	10	63	63	30	30	30	18	18	18
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	0	1	1	0	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	0	1	1	0	1	1	1	1	1	1	1	1
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	no		no	no			no			no	
Pedestrian Recall	no	no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	6.0	6.0	20.0	6.0	6.0	20.0	20.0	20.0	20.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	L	C	R	L	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	15	68	6	59	26	26	26	14	14	14
g / C, Green / Cycle	0.12	0.52	0.05	0.45	0.20	0.20	0.20	0.11	0.11	0.11
(v / s)_i Volume / Saturation Flow Rate	0.09	0.35	0.03	0.37	0.02	0.10	0.16	0.09	0.07	0.01
Total Saturation Flow Adjustment	0.93	0.85	0.93	0.86	0.86	0.96	0.83	0.90	0.93	0.85
s, saturation flow rate [veh/h]	1770	4819	1770	3284	1629	1825	1586	3407	1767	1615
c, Capacity [veh/h]	204	2521	82	1491	326	365	317	367	190	174
d1, Uniform Delay [s]	55.99	22.78	61.04	30.97	42.65	46.21	49.57	56.64	55.83	52.04
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	26.37	1.44	36.42	5.29	0.77	4.80	19.10	16.68	17.72	0.56
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

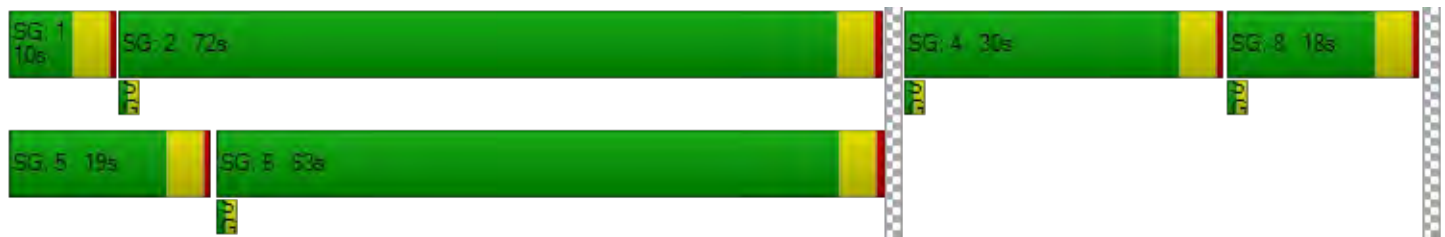
X, volume / capacity	0.79	0.67	0.67	0.82	0.12	0.50	0.80	0.80	0.68	0.05
d, Delay for Lane Group [s/veh]	82.36	24.22	97.45	36.26	43.42	51.01	68.66	73.33	73.55	52.61
Lane Group LOS	F	C	F	D	D	D	E	E	E	D
Critical Lane Group	yes	no	no	yes	no	no	yes	yes	no	no
50th-Percentile Queue Length [veh]	7.16	19.14	2.42	24.91	1.28	6.54	10.90	6.76	5.36	0.32
50th-Percentile Queue Length [ft]	178.94	478.40	60.46	622.81	31.97	163.60	272.62	169.08	133.93	7.88
95th-Percentile Queue Length [veh]	13.16	31.03	5.36	40.03	3.04	12.24	18.68	12.57	10.41	0.80
95th-Percentile Queue Length [ft]	329.06	775.85	134.01	1000.77	75.90	305.96	466.97	314.24	260.17	20.01

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	82.36	24.22	24.22	97.45	36.26	36.26	43.42	51.01	68.66	73.33	73.55	52.61
Movement LOS	F	C	C	F	D	D	D	D	E	E	E	D
d_A, Approach Delay [s/veh]	29.30			38.88			59.81			72.96		
Approach LOS	C			D			E			E		
d_I, Intersection Delay [s/veh]	40.60											
Intersection LOS	D											
Intersection V/C	0.785											

Sequence

Ring 1	1	2	-	4	-	8	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#21: Willow Rd/Bay Rd**

Control Type:	Signalized	Delay (sec / veh):	20.7
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.675

Intersection Setup

Name	Willow Road		Willow Road		Bay Road	
Approach	Northeastbound		Southwestbound		Southeastbound	
Lane Configuration	⇐		⇐		⇐⇐	
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	0	1	0
Pocket Length [ft]	80.00	100.00	100.00	100.00	175.00	100.00
Speed [mph]	30.00		35.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Willow Road		Willow Road		Bay Road	
Base Volume Input [veh/h]	67	1278	1161	445	363	60
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.50	2.40	3.00	1.80	3.30	1.40
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	299	0	77
Total Hourly Volume [veh/h]	67	1278	1161	146	363	0
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	17	329	299	38	94	0
Total Analysis Volume [veh/h]	69	1318	1197	151	374	0
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		2		9	
Bicycle Volume [bicycles/h]	14		6		2	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	100
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	12.00

Phasing & Timing

Control Type	Protected	Permissive	Permissive	Permissive	Split	Split
Signal Group	5	2	6	6	4	4
Auxiliary Signal Groups						
Lead / Lag	Lead	-	-	-	Lag	-
Minimum Green [s]	4	4	4	4	4	4
Maximum Green [s]	4	24	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	10	64	54	54	36	36
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	0	1	1	1	1	1
Pedestrian Clearance [s]	0	1	1	1	1	1
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no	no	no		no	
Maximum Recall	no	no	no		no	
Pedestrian Recall	no	no	no		no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	6.0	6.0	6.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	C	R	L	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	6	60	50	50	32	32
g / C, Green / Cycle	0.06	0.60	0.50	0.50	0.32	0.32
(v / s)_i Volume / Saturation Flow Rate	0.04	0.37	0.34	0.10	0.21	0.00
Total Saturation Flow Adjustment	0.94	0.93	0.92	0.83	0.92	0.84
s, saturation flow rate [veh/h]	1778	3533	3512	1586	1747	1593
c, Capacity [veh/h]	107	2120	1756	793	559	510
d1, Uniform Delay [s]	45.96	12.76	18.96	13.81	29.42	23.12
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	26.47	1.38	2.16	0.53	6.24	0.00
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.65	0.62	0.68	0.19	0.67	0.00
d, Delay for Lane Group [s/veh]	72.43	14.15	21.12	14.35	35.66	23.12
Lane Group LOS	E	B	C	B	D	C
Critical Lane Group	yes	no	yes	no	yes	no
50th-Percentile Queue Length [veh]	2.30	14.36	15.58	2.56	10.54	0.00
50th-Percentile Queue Length [ft]	57.62	359.11	389.61	64.07	263.45	0.00
95th-Percentile Queue Length [veh]	5.14	23.79	25.63	5.64	18.14	0.00
95th-Percentile Queue Length [ft]	128.54	594.87	640.64	140.89	453.54	0.00

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	72.43	14.15	21.12	14.35	35.66	23.12
Movement LOS	E	B	C	B	D	C
d_A, Approach Delay [s/veh]	17.05		20.37		35.66	
Approach LOS	B		C		D	
d_I, Intersection Delay [s/veh]	20.72					
Intersection LOS	C					
Intersection V/C	0.675					

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#22: Willow Rd/Durham St-VA Med Entrance**

Control Type:	Signalized	Delay (sec / veh):	14.3
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.761

Intersection Setup

Name	Willow Road			Willow Road			VA Medical Center			Durham Street		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	0	0	0	1	0	0
Pocket Length [ft]	90.00	100.00	100.00	170.00	100.00	100.00	100.00	100.00	100.00	50.00	100.00	100.00
Speed [mph]	25.00			25.00			10.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road			Willow Road			VA Medical Center			Durham Street		
Base Volume Input [veh/h]	26	1040	16	36	925	89	40	3	10	48	12	70
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	6	0	0	0
Total Hourly Volume [veh/h]	26	1040	16	36	925	89	40	3	4	48	12	70
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	7	274	4	9	243	23	11	1	1	13	3	18
Total Analysis Volume [veh/h]	27	1095	17	38	974	94	42	3	4	51	13	74
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	14			10			10			20		
Bicycle Volume [bicycles/h]	0			0			0			0		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	100
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	5	2	2	1	6	6	4	4	4	8	8	8
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	5	4	4	5	4	4	4	4	4	4	4	4
Maximum Green [s]	30	16	16	30	16	16	16	16	16	16	16	16
Amber [s]	3.0	3.5	3.5	3.0	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	1.0	0.5	0.5	1.0	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	9	79	79	9	79	79	12	12	12	12	12	12
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	0	1	1	0	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	0	1	1	0	1	1	1	1	1	1	1	1
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	no		no	no			no			no	
Pedestrian Recall	no	no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	6.0	6.0	6.0	20.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	R	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	2.00	0.00	2.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	5	75	5	75	8	8	8	8
g / C, Green / Cycle	0.05	0.75	0.05	0.75	0.08	0.08	0.08	0.08
(v / s)_i Volume / Saturation Flow Rate	0.01	0.31	0.02	0.57	0.04	0.00	0.06	0.05
Total Saturation Flow Adjustment	0.95	0.95	0.95	0.99	0.54	0.85	0.49	0.87
s, saturation flow rate [veh/h]	1805	3609	1805	1875	1026	1615	926	1658
c, Capacity [veh/h]	90	2707	90	1406	82	129	74	133
d1, Uniform Delay [s]	45.81	4.52	46.10	7.26	44.26	42.43	44.79	44.66
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	8.30	0.46	13.78	3.90	23.79	0.44	41.43	22.60
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.30	0.41	0.42	0.76	0.55	0.03	0.69	0.66
d, Delay for Lane Group [s/veh]	54.11	4.98	59.87	11.16	68.05	42.87	86.21	67.26
Lane Group LOS	D	A	E	B	E	D	F	E
Critical Lane Group	yes	no	no	yes	no	no	yes	no
50th-Percentile Queue Length [veh]	0.82	6.95	1.19	21.69	1.45	0.11	1.77	2.87
50th-Percentile Queue Length [ft]	20.49	173.68	29.66	542.31	36.22	2.80	44.17	71.72
95th-Percentile Queue Length [veh]	2.01	12.85	2.83	34.99	3.40	0.29	4.07	6.21
95th-Percentile Queue Length [ft]	50.18	321.17	70.85	874.77	85.06	7.21	101.68	155.16

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	54.11	4.98	4.98	59.87	11.16	11.16	68.05	68.05	42.87	86.21	67.26	67.26
Movement LOS	D	A	A	E	B	B	E	E	D	F	E	E
d_A, Approach Delay [s/veh]	6.14			12.84			66.00			74.26		
Approach LOS	A			B			E			E		
d_I, Intersection Delay [s/veh]	14.26											
Intersection LOS	B											
Intersection V/C	0.761											

Sequence

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#23: Willow Rd/Coleman Ave**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 33.8
 Level Of Service: C
 Volume to Capacity (v/c): 0.954

Intersection Setup

Name	Willow Road			Willow Road			Coleman Avenue			Coleman Avenue		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	0	0	0	0	0	0
Pocket Length [ft]	85.00	100.00	100.00	115.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	25.00			30.00			30.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road			Willow Road			Coleman Avenue			Coleman Avenue		
Base Volume Input [veh/h]	22	864	3	1	805	120	191	6	51	4	4	5
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	4.50	4.70	0.00	0.00	3.90	3.30	1.00	0.00	0.00	0.00	0.00	20.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	22	864	3	1	805	120	191	6	51	4	4	5
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	6	227	1	0	212	32	50	2	13	1	1	1
Total Analysis Volume [veh/h]	23	909	3	1	847	126	201	6	54	4	4	5
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	13			64			10			11		
Bicycle Volume [bicycles/h]	24			12			10			1		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	126
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	2	2	2	6	6	6	4	4	4	8	8	8
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	16	16	16	16	16	16	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	81	81	81	81	81	81	45	45	45	45	45	45
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	1	1	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	1	1	1	1	1	1	1	1	1	1	1	1
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no			no			no			no	
Maximum Recall		no			no			no			no	
Pedestrian Recall		no			no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	6.0	6.0	6.0	6.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	2.00	2.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	77	77	77	77	41	41
g / C, Green / Cycle	0.61	0.61	0.61	0.61	0.33	0.33
(v / s)_i Volume / Saturation Flow Rate	0.14	0.50	0.00	0.54	0.29	0.01
Total Saturation Flow Adjustment	0.09	0.95	0.13	0.94	0.47	0.89
s, saturation flow rate [veh/h]	170	1814	248	1793	900	1687
c, Capacity [veh/h]	104	1108	151	1096	293	549
d1, Uniform Delay [s]	11.02	19.16	9.57	20.83	40.38	28.89
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	4.84	6.94	0.08	10.73	30.83	0.08
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.22	0.82	0.01	0.89	0.89	0.02
d, Delay for Lane Group [s/veh]	15.85	26.10	9.65	31.56	71.20	28.97
Lane Group LOS	B	C	A	C	E	C
Critical Lane Group	no	no	no	yes	yes	no
50th-Percentile Queue Length [veh]	0.45	30.80	0.02	36.98	11.58	0.33
50th-Percentile Queue Length [ft]	11.13	769.89	0.41	924.38	289.40	8.31
95th-Percentile Queue Length [veh]	1.12	49.34	0.04	59.18	19.66	0.84
95th-Percentile Queue Length [ft]	28.00	1233.46	1.05	1479.57	491.62	21.07

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	15.85	26.10	26.10	9.65	31.56	31.56	71.20	71.20	71.20	28.97	28.97	28.97
Movement LOS	B	C	C	A	C	C	E	E	E	C	C	C
d_A, Approach Delay [s/veh]	25.85			31.54			71.20			28.97		
Approach LOS	C			C			E			C		
d_I, Intersection Delay [s/veh]	33.83											
Intersection LOS	C											
Intersection V/C	0.954											

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#24: Willow Rd/Gilbert Ave**

Control Type:	Signalized	Delay (sec / veh):	18.3
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.683

Intersection Setup

Name	Willow Road			Willow Road			Gilbert Avenue			Gilbert Avenue		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	↔			↔			↔			↔		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	1	0	0	1	0	0
Pocket Length [ft]	75.00	100.00	100.00	90.00	100.00	100.00	55.00	100.00	100.00	90.00	100.00	100.00
Speed [mph]	25.00			25.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road			Willow Road			Gilbert Avenue			Gilbert Avenue		
Base Volume Input [veh/h]	3	745	40	27	826	4	37	70	17	78	112	100
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	5.20	10.00	7.40	3.60	0.00	2.70	0.00	0.00	2.60	0.00	2.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	3	745	40	27	826	4	37	70	17	78	112	100
Peak Hour Factor	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	1	194	10	7	215	1	10	18	4	20	29	26
Total Analysis Volume [veh/h]	3	776	42	28	860	4	39	73	18	81	117	104
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	3			20			4			5		
Bicycle Volume [bicycles/h]	23			28			16			15		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	126
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	2	2	2	6	6	6	4	4	4	8	8	8
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	16	16	16	16	16	16	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	97	97	97	97	97	97	29	29	29	29	29	29
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	1	1	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	1	1	1	1	1	1	1	1	1	1	1	1
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no			no			no			no	
Maximum Recall		no			no			no			no	
Pedestrian Recall		no			no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	L	C	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	2.00	0.00	2.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	93	93	93	93	25	25	25	25
g / C, Green / Cycle	0.74	0.74	0.74	0.74	0.20	0.20	0.20	0.20
(v / s)_i Volume / Saturation Flow Rate	0.01	0.46	0.06	0.47	0.06	0.05	0.07	0.13
Total Saturation Flow Adjustment	0.24	0.94	0.25	0.96	0.36	0.97	0.61	0.93
s, saturation flow rate [veh/h]	465	1792	472	1833	682	1844	1155	1766
c, Capacity [veh/h]	343	1323	349	1353	135	366	229	350
d1, Uniform Delay [s]	4.35	7.95	4.59	8.18	42.93	42.58	43.53	46.27
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.05	2.18	0.45	2.32	5.29	1.62	4.24	8.36
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.01	0.62	0.08	0.64	0.29	0.25	0.35	0.63
d, Delay for Lane Group [s/veh]	4.40	10.13	5.04	10.50	48.23	44.20	47.77	54.63
Lane Group LOS	A	B	A	B	D	D	D	D
Critical Lane Group	no	no	no	yes	no	no	no	yes
50th-Percentile Queue Length [veh]	0.03	16.53	0.33	17.99	1.30	2.92	2.72	8.20
50th-Percentile Queue Length [ft]	0.84	413.14	8.33	449.83	32.58	73.02	68.04	204.91
95th-Percentile Queue Length [veh]	0.09	27.05	0.84	29.28	3.09	6.30	5.93	14.71
95th-Percentile Queue Length [ft]	2.19	676.18	21.12	732.03	77.23	157.54	148.34	367.63

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	4.40	10.13	10.13	5.04	10.50	10.50	48.23	44.20	44.20	47.77	54.63	54.63
Movement LOS	A	B	B	A	B	B	D	D	D	D	D	D
d_A, Approach Delay [s/veh]	10.11			10.32			45.41			52.79		
Approach LOS	B			B			D			D		
d_I, Intersection Delay [s/veh]	18.35											
Intersection LOS	B											
Intersection V/C	0.683											

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#25: Middlefield Rd-Willow Rd**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 48.7
 Level Of Service: D
 Volume to Capacity (v/c): 0.623

Intersection Setup

Name	Willow Road			Willow Road			Middlefield Road			Middlefield Road		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	T T T			T T T			T T T			T T T		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	1	1	0	1	1	0	1	1	0	0
Pocket Length [ft]	75.00	100.00	215.00	155.00	100.00	160.00	125.00	100.00	70.00	270.00	100.00	100.00
Speed [mph]	30.00			25.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road			Willow Road			Middlefield Road			Middlefield Road		
Base Volume Input [veh/h]	28	201	139	458	75	391	80	368	256	320	356	15
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	5.00	3.60	2.60	2.70	3.80	2.50	0.50	5.50	5.30	3.70	13.30
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	119	0	0	1000	0	0	1000	0	0	0
Total Hourly Volume [veh/h]	28	201	20	458	75	0	80	368	0	320	356	15
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	7	53	5	122	20	0	21	98	0	85	95	4
Total Analysis Volume [veh/h]	30	214	21	487	80	0	85	391	0	340	379	16
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	3			7			11			3		
Bicycle Volume [bicycles/h]	33			43			24			24		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Split	Split	Split	Split	Split	Split	Split
Signal Group	8	8	8	4	7	4	6	6	6	2	5	2
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	-	-	-	Lag	-	-	-	-	-
Minimum Green [s]	4	4	4	0	5	0	4	4	4	0	5	0
Maximum Green [s]	16	16	16	0	30	0	16	16	16	0	30	0
Amber [s]	3.5	3.5	3.5	0.0	3.0	0.0	3.5	3.5	3.5	0.0	3.0	0.0
All red [s]	0.5	0.5	0.5	0.0	1.0	0.0	0.5	0.5	0.5	0.0	1.0	0.0
Split [s]	29	29	29	0	39	0	27	27	27	0	35	0
Vehicle Extension [s]	3.0	3.0	3.0	0.0	3.0	0.0	3.0	3.0	3.0	0.0	3.0	0.0
Walk [s]	1	1	1	0	1	0	1	1	1	0	1	0
Pedestrian Clearance [s]	1	1	1	0	1	0	1	1	1	0	1	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	0.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	0.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0	0.0
Minimum Recall		no			no			no			no	
Maximum Recall		no			no			no			no	
Pedestrian Recall		no			no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	0.0	20.0	0.0	0.0	0.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	R	C	R	L	C	R	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	25	25	25	35	35	23	23	23	31
g / C, Green / Cycle	0.19	0.19	0.19	0.27	0.27	0.18	0.18	0.18	0.24
(v / s)_i Volume / Saturation Flow Rate	0.02	0.12	0.01	0.17	0.00	0.05	0.11	0.00	0.15
Total Saturation Flow Adjustment	0.95	0.95	0.82	0.89	0.82	0.93	0.95	0.81	0.85
s, saturation flow rate [veh/h]	1805	1810	1559	3377	1556	1761	3600	1531	4862
c, Capacity [veh/h]	347	348	300	909	419	312	637	271	1159
d1, Uniform Delay [s]	43.12	48.09	42.98	41.71	34.71	46.27	49.40	44.03	44.41
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.49	7.90	0.45	3.22	0.00	2.15	4.38	0.00	2.65
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.09	0.61	0.07	0.62	0.00	0.27	0.61	0.00	0.63
d, Delay for Lane Group [s/veh]	43.61	55.99	43.43	44.93	34.71	48.42	53.79	44.03	47.06
Lane Group LOS	D	E	D	D	C	D	D	D	D
Critical Lane Group	no	yes	no	yes	no	no	yes	no	yes
50th-Percentile Queue Length [veh]	0.96	8.14	0.67	10.82	0.00	2.90	7.87	0.00	10.06
50th-Percentile Queue Length [ft]	23.91	203.47	16.72	270.47	0.00	72.44	196.85	0.00	251.49
95th-Percentile Queue Length [veh]	2.32	14.62	1.65	18.55	0.00	6.26	14.23	0.00	17.44
95th-Percentile Queue Length [ft]	58.00	365.51	41.37	463.82	0.00	156.48	355.71	0.00	436.01

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	43.61	55.99	43.43	44.93	44.93	34.71	48.42	53.79	44.03	47.06	47.06	47.06
Movement LOS	D	E	D	D	D	C	D	D	D	D	D	D
d_A, Approach Delay [s/veh]	53.60			44.93			52.83			47.06		
Approach LOS	D			D			D			D		
d_I, Intersection Delay [s/veh]	48.66											
Intersection LOS	D											
Intersection V/C	0.623											

Sequence

Ring 1	-	5	-	6	-	7	-	8	-	-	-	-	-	-	-	-
Ring 2	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#26: Ravenswood Ave/Laurel St**

Control Type:	Signalized	Delay (sec / veh):	20.4
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.958

Intersection Setup

Name	Ravenswood Avenue			Ravenswood Avenue			Laurel Street			Laurel Street		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	1	0	0	0	0	0
Pocket Length [ft]	85.00	100.00	100.00	75.00	100.00	100.00	95.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Ravenswood Avenue			Ravenswood Avenue			Laurel Street			Laurel Street		
Base Volume Input [veh/h]	15	618	67	20	471	11	138	113	13	136	188	65
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	1.30	3.00	0.00	4.90	0.00	6.50	0.90	7.70	11.00	2.10	3.10
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	15	618	67	20	471	11	138	113	13	136	188	65
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	4	159	17	5	121	3	36	29	3	35	48	17
Total Analysis Volume [veh/h]	15	637	69	21	486	11	142	116	13	140	194	67
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	36			37			51			41		
Bicycle Volume [bicycles/h]	52			17			11			37		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	60
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	2	2	2	6	6	6	8	8	8	4	4	4
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	16	16	16	16	16	16	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	32	32	32	32	32	32	28	28	28	28	28	28
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	1	1	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no			no		no	no			no	
Maximum Recall		no			no		no	no			no	
Pedestrian Recall		no			no		no	no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	6.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	L	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	2.00	0.00	2.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	28	28	28	28	24	24	24
g / C, Green / Cycle	0.47	0.47	0.47	0.47	0.40	0.40	0.40
(v / s)_i Volume / Saturation Flow Rate	0.02	0.38	0.07	0.28	0.17	0.07	0.32
Total Saturation Flow Adjustment	0.34	0.97	0.16	0.95	0.44	0.98	0.66
s, saturation flow rate [veh/h]	647	1848	306	1805	842	1855	1251
c, Capacity [veh/h]	302	862	143	842	337	742	501
d1, Uniform Delay [s]	8.74	13.81	9.16	11.78	12.99	11.61	15.89
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.31	8.53	2.17	3.02	3.84	0.51	12.68
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

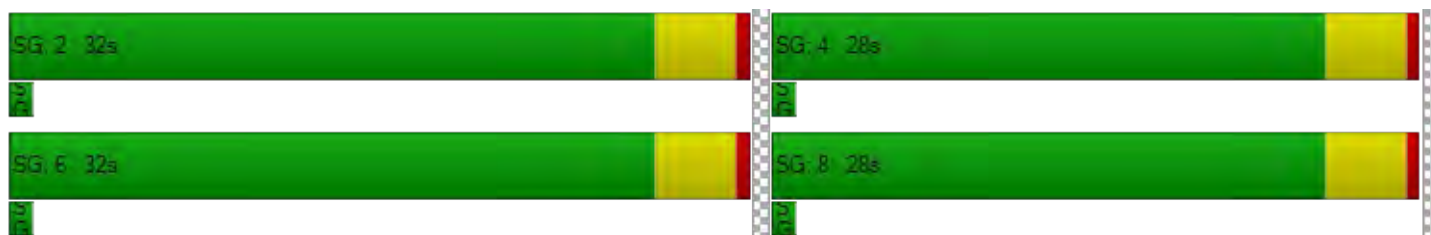
X, volume / capacity	0.05	0.82	0.15	0.59	0.42	0.17	0.80
d, Delay for Lane Group [s/veh]	9.05	22.34	11.33	14.80	16.84	12.12	28.57
Lane Group LOS	A	C	B	B	B	B	C
Critical Lane Group	no	yes	no	no	no	no	yes
50th-Percentile Queue Length [veh]	0.16	13.18	0.24	7.17	2.00	1.53	7.76
50th-Percentile Queue Length [ft]	3.90	329.60	5.96	179.17	49.94	38.33	193.93
95th-Percentile Queue Length [veh]	0.40	22.04	0.61	13.18	4.54	3.58	14.06
95th-Percentile Queue Length [ft]	10.02	550.96	15.21	329.41	113.39	89.53	351.38

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	9.05	22.34	22.34	11.33	14.80	14.80	16.84	12.12	12.12	28.57	28.57	28.57
Movement LOS	A	C	C	B	B	B	B	B	B	C	C	C
d_A, Approach Delay [s/veh]	22.06			14.66			14.59			28.57		
Approach LOS	C			B			B			C		
d_I, Intersection Delay [s/veh]	20.36											
Intersection LOS	C											
Intersection V/C	0.958											

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#28: Oak Grove Ave/Laurel St**

Control Type:	Signalized	Delay (sec / veh):	15.6
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.751

Intersection Setup

Name	Oak Grove Avenue			Oak Grove Avenue			Laurel Street			Laurel Street		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	⊕			⊕			⊕			⊕		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	25.00			25.00			25.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Oak Grove Avenue			Oak Grove Avenue			Laurel Street			Laurel Street		
Base Volume Input [veh/h]	11	258	85	30	345	61	30	107	16	72	200	52
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	9.10	3.10	16.50	0.00	1.40	0.00	0.00	2.00	3.80	6.70	3.70	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	11	258	85	30	345	61	30	107	16	72	200	52
Peak Hour Factor	0.8200	0.8200	0.8200	0.8200	0.8200	0.8200	0.8200	0.8200	0.8200	0.8200	0.8200	0.8200
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	3	79	26	9	105	19	9	33	5	22	61	16
Total Analysis Volume [veh/h]	13	315	104	37	421	74	37	130	20	88	244	63
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	24			51			27			65		
Bicycle Volume [bicycles/h]	28			39			21			32		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	60
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	2	2	2	6	6	6	8	8	8	4	4	4
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	16	16	16	16	16	16	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	33	33	33	33	33	33	27	27	27	27	27	27
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	1	1	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no			no			no			no	
Maximum Recall		no			no			no			no	
Pedestrian Recall		no			no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	2.00	2.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	29	29	23	23
g / C, Green / Cycle	0.48	0.48	0.38	0.38
(v / s)_i Volume / Saturation Flow Rate	0.25	0.30	0.11	0.25
Total Saturation Flow Adjustment	0.92	0.92	0.86	0.84
s, saturation flow rate [veh/h]	1753	1756	1636	1596
c, Capacity [veh/h]	847	849	627	612
d1, Uniform Delay [s]	10.63	11.49	12.88	15.16
k, delay calibration	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	2.19	3.49	1.21	5.19
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00

Lane Group Results

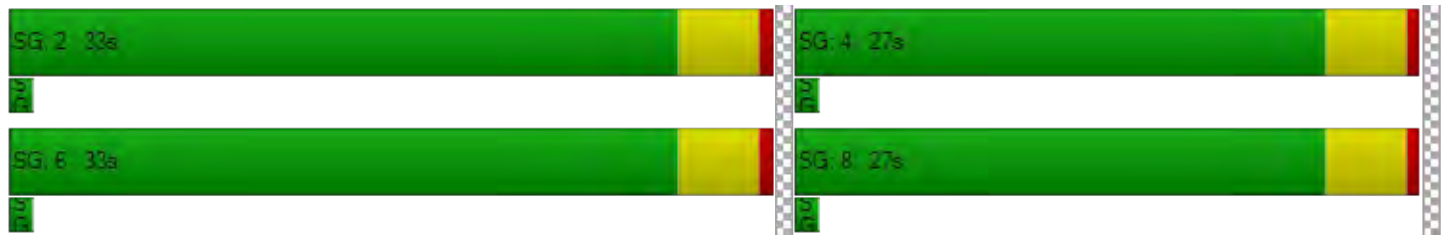
X, volume / capacity	0.51	0.63	0.30	0.65
d, Delay for Lane Group [s/veh]	12.81	14.98	14.10	20.35
Lane Group LOS	B	B	B	C
Critical Lane Group	no	yes	no	yes
50th-Percentile Queue Length [veh]	5.72	7.82	2.43	6.46
50th-Percentile Queue Length [ft]	143.02	195.52	60.81	161.59
95th-Percentile Queue Length [veh]	10.98	14.15	5.39	12.12
95th-Percentile Queue Length [ft]	274.38	353.75	134.68	302.91

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	12.81	12.81	12.81	14.98	14.98	14.98	14.10	14.10	14.10	20.35	20.35	20.35
Movement LOS	B	B	B	B	B	B	B	B	B	C	C	C
d_A, Approach Delay [s/veh]	12.81			14.98			14.10			20.35		
Approach LOS	B			B			B			C		
d_I, Intersection Delay [s/veh]	15.64											
Intersection LOS	B											
Intersection V/C	0.751											

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Intersection Level Of Service Report

#29: El Camino Real (SR 82)/Encinal Ave-Menlo College Entrance

Control Type:	Signalized	Delay (sec / veh):	18.1
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.646

Intersection Setup

Name	Menlo College			Encinal Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	↵↵			↵↵			↵↵↵			↵↵↵		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	1	0	1	1	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	140.00	100.00	150.00	135.00	100.00	100.00
Speed [mph]	15.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Menlo College			Encinal Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	8	57	9	156	14	149	16	840	113	169	1838	71
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	3.20	0.00	2.70	0.00	3.60	0.90	0.60	3.30	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	124	0	0	0
Total Hourly Volume [veh/h]	8	57	9	156	14	149	16	840	0	169	1838	71
Peak Hour Factor	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	2	15	2	42	4	40	4	228	0	46	499	19
Total Analysis Volume [veh/h]	9	62	10	170	15	162	17	913	0	184	1998	77
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	2			2			0			11		
Bicycle Volume [bicycles/h]	0			4			2			4		

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	8	0	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	0	5	0	0	5	0	5	5	0	5	5	0
Maximum Green [s]	0	30	0	0	30	0	30	30	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	0	35	0	0	35	0	9	75	0	26	92	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
Walk [s]	0	1	0	0	1	0	0	1	0	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	no		no	no	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	50.0	0.0	0.0	0.0	0.0	50.0	50.0	0.0	50.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	C	R	L	C	R	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	31	31	31	31	5	71	71	22	88
g / C, Green / Cycle	0.23	0.23	0.23	0.23	0.04	0.52	0.52	0.16	0.65
(v / s)_i Volume / Saturation Flow Rate	0.01	0.04	0.14	0.10	0.01	0.26	0.00	0.10	0.42
Total Saturation Flow Adjustment	0.46	0.98	0.68	0.83	0.95	0.92	0.84	0.94	0.87
s, saturation flow rate [veh/h]	868	1860	1284	1573	1805	3492	1601	1794	4982
c, Capacity [veh/h]	198	424	293	358	66	1823	836	290	3224
d1, Uniform Delay [s]	40.96	42.16	47.36	45.19	63.69	21.03	15.53	53.24	14.52
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.43	0.87	9.97	4.07	9.09	0.99	0.00	10.12	1.00
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.33	1.33	1.33	1.33	1.33
PF, progression factor	1.00	1.00	1.00	1.00	1.00	0.73	0.73	1.00	0.45

Lane Group Results

X, volume / capacity	0.05	0.17	0.63	0.45	0.26	0.50	0.00	0.63	0.64
d, Delay for Lane Group [s/veh]	41.39	43.03	57.33	49.26	72.79	16.38	11.37	63.36	7.50
Lane Group LOS	D	D	E	D	E	B	B	E	A
Critical Lane Group	no	no	yes	no	yes	no	no	no	yes
50th-Percentile Queue Length [veh]	0.29	2.35	7.33	5.87	0.70	9.90	0.00	7.35	11.83
50th-Percentile Queue Length [ft]	7.22	58.87	183.36	146.66	17.40	247.62	0.00	183.66	295.71
95th-Percentile Queue Length [veh]	0.73	5.24	13.43	11.20	1.72	17.21	0.00	13.44	20.04
95th-Percentile Queue Length [ft]	18.35	130.96	335.67	280.02	42.99	430.35	0.00	336.12	500.90

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	41.39	43.03	43.03	57.33	57.33	49.26	72.79	16.38	11.37	63.36	7.50	7.50
Movement LOS	D	D	D	E	E	D	E	B	B	E	A	A
d_A, Approach Delay [s/veh]	42.85			53.56			17.41			12.05		
Approach LOS	D			D			B			B		
d_I, Intersection Delay [s/veh]	18.10											
Intersection LOS	B											
Intersection V/C	0.646											

Sequence

Ring 1	1	2	4	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	8	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Intersection Level Of Service Report

#30: El Camino Real (SR 82)/Glenwood Ave-Valparaiso Ave

Control Type:	Signalized	Delay (sec / veh):	35.4
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.823

Intersection Setup

Name	Valparaiso Avenue			Glenwood Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	⇌⇌⇌			⇌			⇌⇌⇌			⇌⇌⇌		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	1	1	0	0	1	0	1	1	0	0
Pocket Length [ft]	205.00	100.00	130.00	120.00	100.00	100.00	190.00	100.00	105.00	180.00	100.00	100.00
Speed [mph]	30.00			25.00			35.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Valparaiso Avenue			Glenwood Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	255	126	124	84	177	16	101	700	35	54	1349	547
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.40	2.40	1.60	0.00	2.80	0.00	0.00	3.90	0.00	1.90	3.90	3.30
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	74	0	0	0	0	0	29	0	0	285
Total Hourly Volume [veh/h]	255	126	50	84	177	16	101	700	6	54	1349	262
Peak Hour Factor	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	70	35	14	23	49	4	28	192	2	15	371	72
Total Analysis Volume [veh/h]	280	138	55	92	195	18	111	769	7	59	1482	288
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	4			12			10			5		
Bicycle Volume [bicycles/h]	18			15			4			9		

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	121.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Split	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	8	0	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	0	5	0	0	5	0	5	5	0	5	5	0
Maximum Green [s]	0	30	0	0	30	0	30	30	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	0	24	0	0	23	0	15	77	0	12	74	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
Walk [s]	0	1	0	0	1	0	0	1	0	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	no		no	no	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	50.0	0.0	0.0	50.0	0.0	50.0	50.0	0.0	50.0	50.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	L	C	L	C	R	L	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	20	20	19	19	11	73	73	8	70	70
g / C, Green / Cycle	0.15	0.15	0.14	0.14	0.08	0.54	0.54	0.06	0.51	0.51
(v / s)_i Volume / Saturation Flow Rate	0.12	0.03	0.05	0.12	0.06	0.22	0.00	0.03	0.43	0.18
Total Saturation Flow Adjustment	0.90	0.84	0.95	0.96	0.95	0.92	0.85	0.93	0.92	0.82
s, saturation flow rate [veh/h]	3418	1590	1805	1825	1805	3482	1615	1771	3482	1563
c, Capacity [veh/h]	503	234	252	255	146	1869	867	104	1792	805
d1, Uniform Delay [s]	56.36	51.24	53.03	56.98	61.21	18.73	14.66	62.31	27.88	19.63
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	14.79	2.35	4.04	26.43	30.49	0.67	0.02	20.42	4.54	1.24
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.33	1.33	1.33	1.33	1.33	1.33
PF, progression factor	1.00	1.00	1.00	1.00	1.00	0.71	0.71	1.00	0.74	0.74

Lane Group Results

X, volume / capacity	0.83	0.24	0.36	0.84	0.76	0.41	0.01	0.57	0.83	0.36
d, Delay for Lane Group [s/veh]	71.15	53.60	57.07	83.40	91.70	13.90	10.37	82.73	25.28	15.84
Lane Group LOS	E	D	E	F	F	B	B	F	C	B
Critical Lane Group	yes	no	no	yes	yes	no	no	no	yes	no
50th-Percentile Queue Length [veh]	10.20	2.00	3.48	9.96	5.10	7.19	0.09	2.53	26.83	5.25
50th-Percentile Queue Length [ft]	254.98	50.10	86.98	248.90	127.38	179.74	2.17	63.31	670.87	131.22
95th-Percentile Queue Length [veh]	17.64	4.55	7.30	17.29	9.99	13.21	0.22	5.58	43.06	10.24
95th-Percentile Queue Length [ft]	441.12	113.72	182.54	432.23	249.79	330.25	5.61	139.45	1076.52	255.88

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	71.15	71.15	53.60	57.07	83.40	83.40	91.70	13.90	10.37	82.73	25.28	15.84
Movement LOS	E	E	D	E	F	F	F	B	B	F	C	B
d_A, Approach Delay [s/veh]	69.11			75.46			23.61			25.64		
Approach LOS	E			E			C			C		
d_I, Intersection Delay [s/veh]	35.36											
Intersection LOS	D											
Intersection V/C	0.823											

Sequence

Ring 1	1	2	4	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#31: El Camino Real (SR 82)/Oak Grove Ave**

Control Type:	Signalized	Delay (sec / veh):	30.6
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.728

Intersection Setup

Name	Oak Grove Avenue			Oak Grove Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	⇌⇌⇌			⇌⇌⇌			⇌⇌⇌			⇌⇌⇌		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	1	1	0	1	1	0	1	1	0	1
Pocket Length [ft]	90.00	100.00	110.00	115.00	100.00	110.00	280.00	100.00	110.00	200.00	100.00	140.00
Speed [mph]	25.00			25.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Oak Grove Avenue			Oak Grove Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	57	196	53	117	254	63	89	731	73	125	1365	85
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	8.80	1.00	1.90	0.90	2.80	3.20	4.50	5.20	4.10	1.60	4.30	9.40
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	58	0	0	50	0	0	69	0	0	69
Total Hourly Volume [veh/h]	57	196	0	117	254	13	89	731	4	125	1365	16
Peak Hour Factor	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	15	51	0	30	66	3	23	190	1	33	355	4
Total Analysis Volume [veh/h]	59	204	0	122	265	14	93	761	4	130	1422	17
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	8			4			25			11		
Bicycle Volume [bicycles/h]	19			38			0			2		

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	98.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	7	4	0	3	8	0	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	5	5	0	5	5	0	5	5	0	5	5	0
Maximum Green [s]	30	30	0	30	30	0	30	30	0	30	30	0
Amber [s]	3.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
All red [s]	1.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	11	25	0	17	31	0	14	74	0	20	80	0
Vehicle Extension [s]	3.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
Walk [s]	0	1	0	0	1	0	0	1	0	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	2.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
Minimum Recall	no	no		no	no		no	no		no	no	
Maximum Recall	no	no		no	no		no	no		no	no	
Pedestrian Recall	no	no		no	no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	50.0	50.0	0.0	50.0	50.0	0.0	50.0	50.0	0.0	50.0	50.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	R	L	C	R	L	C	R	L	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	7	21	21	13	27	27	10	70	70	16	76	76
g / C, Green / Cycle	0.05	0.15	0.15	0.10	0.20	0.20	0.07	0.51	0.51	0.12	0.56	0.56
(v / s)_i Volume / Saturation Flow Rate	0.04	0.11	0.00	0.07	0.14	0.01	0.05	0.22	0.00	0.07	0.41	0.01
Total Saturation Flow Adjustment	0.87	0.99	0.83	0.94	0.97	0.82	0.91	0.90	0.82	0.94	0.91	0.78
s, saturation flow rate [veh/h]	1659	1881	1585	1789	1848	1565	1727	3439	1551	1777	3468	1476
c, Capacity [veh/h]	85	290	245	171	367	311	127	1770	799	209	1938	825
d1, Uniform Delay [s]	63.44	54.54	48.62	59.69	50.99	44.07	61.69	20.57	16.06	57.12	22.43	13.39
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	37.19	13.30	0.00	22.34	11.66	0.27	30.86	0.76	0.01	13.15	2.51	0.05
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.67	1.67	1.67	1.33	1.33	1.33
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.29	0.29	1.00	0.66	0.66

Lane Group Results

X, volume / capacity	0.69	0.70	0.00	0.71	0.72	0.05	0.73	0.43	0.01	0.62	0.73	0.02
d, Delay for Lane Group [s/veh]	100.62	67.83	48.62	82.03	62.66	44.35	89.28	6.78	4.71	70.27	17.42	8.95
Lane Group LOS	F	E	D	F	E	D	F	A	A	E	B	A
Critical Lane Group	yes	no	no	no	yes	no	yes	no	no	no	yes	no
50th-Percentile Queue Length [veh]	2.72	8.66	0.00	5.43	11.10	0.46	4.21	4.47	0.03	5.37	19.69	0.19
50th-Percentile Queue Length [ft]	67.94	216.39	0.00	135.69	277.42	11.49	105.27	111.81	0.70	134.15	492.31	4.86
95th-Percentile Queue Length [veh]	5.93	15.38	0.00	10.52	18.96	1.15	8.55	8.98	0.07	10.42	31.89	0.50
95th-Percentile Queue Length [ft]	148.15	384.55	0.00	262.93	474.02	28.85	213.77	224.61	1.82	260.51	797.29	12.45

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	100.62	67.83	48.62	82.03	62.66	44.35	89.28	6.78	4.71	70.27	17.42	8.95
Movement LOS	F	E	D	F	E	D	F	A	A	E	B	A
d_A, Approach Delay [s/veh]	75.19			67.91			15.71			21.71		
Approach LOS	E			E			B			C		
d_I, Intersection Delay [s/veh]	30.59											
Intersection LOS	C											
Intersection V/C	0.728											

Sequence

Ring 1	1	2	3	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	7	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#32: El Camino Real (SR 82)/Santa Cruz Ave**

Control Type:	Signalized	Delay (sec / veh):	11.3
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.658

Intersection Setup

Name	Santa Cruz Avenue			Santa Cruz Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	↵↵↵			↵↵			↵↵			↵↵		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	1	1	0	0	0	0	1	0	0	1
Pocket Length [ft]	140.00	100.00	140.00	50.00	100.00	100.00	100.00	100.00	280.00	100.00	100.00	105.00
Speed [mph]	30.00			25.00			35.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Santa Cruz Avenue			Santa Cruz Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	78	54	76	41	44	30	0	818	39	0	1420	16
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	3.80	18.50	7.90	2.40	20.50	3.30	0.00	3.80	5.10	0.00	3.50	8.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	53	0	0	84
Total Hourly Volume [veh/h]	78	54	76	41	44	30	0	818	0	0	1420	0
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	1.0000	0.9700	0.9700	1.0000	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	20	14	20	11	11	8	0	211	0	0	366	0
Total Analysis Volume [veh/h]	80	56	78	42	45	31	0	843	0	0	1464	0
Presence of On-Street Parking	no		no	no		no			no			no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	19			15			43			20		
Bicycle Volume [bicycles/h]	14			36			2			4		

Intersection Settings

Located in CBD	yes
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	92.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Split	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	0	4	0	0	8	0	0	6	0	0	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	-	-	-	-	-	-
Minimum Green [s]	0	5	0	0	5	0	0	5	0	0	5	0
Maximum Green [s]	0	30	0	0	30	0	0	30	0	0	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	0.0	1.0	0.0	0.0	1.0	0.0
Split [s]	0	17	0	0	17	0	0	102	0	0	102	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	1	0	0	1	0	0	1	0	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Minimum Recall		no			no			no			no	
Maximum Recall		no			no			no			no	
Pedestrian Recall		no			no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	R	L	C	C	R	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	13	13	13	13	13	98	98	98	98
g / C, Green / Cycle	0.10	0.10	0.10	0.10	0.10	0.72	0.72	0.72	0.72
(v / s)_i Volume / Saturation Flow Rate	0.05	0.04	0.06	0.03	0.06	0.27	0.00	0.47	0.00
Total Saturation Flow Adjustment	0.82	0.76	0.71	0.83	0.70	0.83	0.73	0.83	0.71
s, saturation flow rate [veh/h]	1565	1443	1347	1586	1332	3137	1383	3146	1346
c, Capacity [veh/h]	150	138	129	152	127	2260	997	2267	970
d1, Uniform Delay [s]	58.62	57.87	59.04	57.13	58.99	7.26	5.31	9.93	5.31
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	13.02	8.64	19.36	4.49	18.94	0.47	0.00	1.44	0.00
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.33	1.33	1.33	1.33
PF, progression factor	1.00	1.00	1.00	1.00	1.00	0.15	0.15	0.15	0.15

Lane Group Results

X, volume / capacity	0.53	0.41	0.61	0.28	0.60	0.37	0.00	0.65	0.00
d, Delay for Lane Group [s/veh]	71.64	66.51	78.40	61.62	77.93	1.60	0.82	2.97	0.82
Lane Group LOS	E	E	E	E	E	A	A	A	A
Critical Lane Group	no	no	yes	no	yes	no	no	yes	no
50th-Percentile Queue Length [veh]	3.32	2.24	3.34	1.63	3.25	2.14	0.00	6.26	0.00
50th-Percentile Queue Length [ft]	83.05	56.12	83.62	40.71	81.22	53.40	0.00	156.53	0.00
95th-Percentile Queue Length [veh]	7.02	5.02	7.07	3.78	6.89	4.81	0.00	11.81	0.00
95th-Percentile Queue Length [ft]	175.61	125.61	176.63	94.52	172.37	120.28	0.00	295.19	0.00

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	71.64	66.51	78.40	61.62	77.93	77.93	0.00	1.60	0.82	0.00	2.97	0.82
Movement LOS	E	E	E	E	E	E		A	A		A	A
d_A, Approach Delay [s/veh]	72.76			72.12			1.60			2.97		
Approach LOS	E			E			A			A		
d_I, Intersection Delay [s/veh]	11.28											
Intersection LOS	B											
Intersection V/C	0.658											

Sequence

Ring 1	-	2	-	-	4	-	8	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Intersection Level Of Service Report

#33: El Camino Real (SR 82)/Ravenswood Ave-Menlo Ave

Control Type:	Signalized	Delay (sec / veh):	37.8
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.824

Intersection Setup

Name	Menlo Avenue			Ravenswood Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	1	0	1	1	0	0	1	0	1
Pocket Length [ft]	100.00	100.00	100.00	240.00	100.00	135.00	320.00	100.00	100.00	240.00	100.00	65.00
Speed [mph]	30.00			30.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Menlo Avenue			Ravenswood Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	24	345	60	417	214	44	88	779	396	161	1420	16
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	4.20	1.20	1.70	2.20	1.40	6.80	10.20	3.10	2.50	3.10	3.50	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	58	0	0	377	0	0	14
Total Hourly Volume [veh/h]	24	345	60	417	214	0	88	779	19	161	1420	2
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	6	89	15	107	55	0	23	201	5	41	366	1
Total Analysis Volume [veh/h]	25	356	62	430	221	0	91	803	20	166	1464	2
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	23			3			1			20		
Bicycle Volume [bicycles/h]	41			11			1			4		

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	109.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Split	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	8	8	8	4	4	4	1	6	6	5	2	2
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	16	16	16	16	4	16	16	4	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	25	25	25	25	25	25	14	64	64	22	72	72
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	1	1	1	1	0	1	1	0	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	no		no	no	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	6.0	6.0	6.0	20.0	20.0	20.0	20.0	6.0	6.0	20.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	L	C	R	L	C	R	L	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	21	21	21	21	10	60	60	18	68	68
g / C, Green / Cycle	0.15	0.15	0.15	0.15	0.07	0.44	0.44	0.13	0.50	0.50
(v / s)_i Volume / Saturation Flow Rate	0.13	0.13	0.12	0.00	0.06	0.23	0.01	0.09	0.42	0.00
Total Saturation Flow Adjustment	0.92	0.90	0.99	0.80	0.86	0.92	0.83	0.92	0.92	0.85
s, saturation flow rate [veh/h]	3490	3430	1874	1512	1638	3509	1576	1751	3495	1615
c, Capacity [veh/h]	539	530	289	233	120	1548	695	232	1748	807
d1, Uniform Delay [s]	55.69	55.59	55.12	48.62	61.80	27.54	21.51	56.55	29.25	17.02
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	13.24	12.75	17.30	0.00	35.04	1.25	0.08	17.28	4.98	0.01
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.33	1.33	1.33	1.67	1.67	1.67
PF, progression factor	1.00	1.00	1.00	1.00	1.00	0.85	0.85	0.90	0.33	0.33

Lane Group Results

X, volume / capacity	0.82	0.81	0.76	0.00	0.76	0.52	0.03	0.72	0.84	0.00
d, Delay for Lane Group [s/veh]	68.94	68.34	72.43	48.62	96.84	24.59	18.31	68.08	14.72	5.67
Lane Group LOS	E	E	E	D	F	C	B	E	B	A
Critical Lane Group	yes	yes	no	no	yes	no	no	no	yes	no
50th-Percentile Queue Length [veh]	10.68	10.10	9.73	0.00	4.22	10.83	0.35	7.01	21.71	0.02
50th-Percentile Queue Length [ft]	266.99	252.59	243.16	0.00	105.40	270.66	8.79	175.30	542.67	0.40
95th-Percentile Queue Length [veh]	18.35	17.51	16.95	0.00	8.56	18.56	0.89	12.94	35.01	0.04
95th-Percentile Queue Length [ft]	458.73	437.63	423.81	0.00	213.99	464.11	22.26	323.60	875.34	1.03

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	68.94	68.94	68.94	68.34	72.43	48.62	96.84	24.59	18.31	68.08	14.72	5.67
Movement LOS	E	E	E	E	E	D	F	C	B	E	B	A
d_A, Approach Delay [s/veh]	68.94			69.73			31.64			20.14		
Approach LOS	E			E			C			C		
d_I, Intersection Delay [s/veh]	37.83											
Intersection LOS	D											
Intersection V/C	0.824											

Sequence

Ring 1	1	2	-	4	-	8	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#34: El Camino Real (SR 82)/Roble Ave**

Control Type:	Signalized	Delay (sec / veh):	8.7
Analysis Method:	HCM2000	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.569

Intersection Setup

Name	Roble Avenue			Roble Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	+			+			+			+		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	1	1	0	0	1	0	1
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	200.00	100.00	100.00	115.00	100.00	210.00
Speed [mph]	25.00			15.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Roble Avenue			Roble Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	65	8	68	6	3	10	43	1188	25	43	1755	43
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.50	0.00	0.00	0.00	0.00	10.00	0.00	2.90	0.00	2.30	2.60	2.30
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	9	0	0	0	0	0	0
Total Hourly Volume [veh/h]	65	8	68	6	3	1	43	1188	25	43	1755	43
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	17	2	18	2	1	0	11	313	7	11	462	11
Total Analysis Volume [veh/h]	68	8	72	6	3	1	45	1251	26	45	1847	45
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	11			8			0			2		
Bicycle Volume [bicycles/h]	16			0			3			5		

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	65.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	8	8	8	4	4	4	1	6	6	5	2	2
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	16	16	16	16	4	16	16	4	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	29	29	29	29	29	29	11	96	96	11	96	96
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	1	1	1	1	0	1	1	0	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	no		no	no	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	6.0	6.0	6.0	6.0	6.0	6.0	20.0	6.0	6.0	20.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	R	L	C	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	25	25	25	7	92	7	92
g / C, Green / Cycle	0.18	0.18	0.18	0.05	0.68	0.05	0.68
(v / s)_i Volume / Saturation Flow Rate	0.10	0.01	0.00	0.02	0.25	0.03	0.38
Total Saturation Flow Adjustment	0.77	0.87	0.77	0.95	0.88	0.93	0.88
s, saturation flow rate [veh/h]	1463	1655	1468	1805	5014	1764	5026
c, Capacity [veh/h]	269	304	270	93	3392	91	3400
d1, Uniform Delay [s]	50.39	45.55	45.33	62.74	9.55	62.78	11.41
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	7.88	0.18	0.02	16.96	0.32	18.03	0.66
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.33	1.33	1.33	1.33
PF, progression factor	1.00	1.00	1.00	1.00	0.35	1.00	0.35

Lane Group Results

X, volume / capacity	0.55	0.03	0.00	0.48	0.38	0.50	0.56
d, Delay for Lane Group [s/veh]	58.27	45.73	45.35	79.70	3.66	80.82	4.65
Lane Group LOS	E	D	D	E	A	F	A
Critical Lane Group	yes	no	no	yes	no	no	yes
50th-Percentile Queue Length [veh]	5.79	0.30	0.03	1.90	3.70	1.91	7.34
50th-Percentile Queue Length [ft]	144.68	7.48	0.83	47.45	92.41	47.65	183.53
95th-Percentile Queue Length [veh]	11.08	0.76	0.09	4.34	7.68	4.35	13.44
95th-Percentile Queue Length [ft]	276.96	19.01	2.15	108.38	191.98	108.79	335.92

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	58.27	58.27	58.27	45.73	45.73	45.35	79.70	3.66	3.66	80.82	4.65	4.65
Movement LOS	E	E	E	D	D	D	E	A	A	F	A	A
d_A, Approach Delay [s/veh]	58.27			45.69			6.24			6.42		
Approach LOS	E			D			A			A		
d_I, Intersection Delay [s/veh]	8.71											
Intersection LOS	A											
Intersection V/C	0.569											

Sequence

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#35: El Camino Real (SR 82)/Middle Ave**

Control Type:	Signalized	Delay (sec / veh):	16.8
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.685

Intersection Setup

Name	Middle Avenue		El Camino Real (SR 82)		El Camino Real (SR 82)	
Approach	Northeastbound		Northwestbound		Southeastbound	
Lane Configuration						
Turning Movement	Left	Right	Left	Thru	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	1	1	0	0	0
Pocket Length [ft]	100.00	55.00	275.00	100.00	100.00	100.00
Speed [mph]	30.00		30.00		35.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		no		yes	

Volumes

Name	Middle Avenue		El Camino Real (SR 82)		El Camino Real (SR 82)	
Base Volume Input [veh/h]	185	285	182	1040	1753	68
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.20	0.70	2.20	4.80	3.60	2.90
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	183	0	0	0	0
Total Hourly Volume [veh/h]	185	102	182	1040	1753	68
Peak Hour Factor	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	50	28	49	283	476	18
Total Analysis Volume [veh/h]	201	111	198	1130	1905	74
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	13		0		8	
Bicycle Volume [bicycles/h]	5		3		4	

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	76.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	12.00

Phasing & Timing

Control Type	Protected	Permissive	Protected	Permissive	Permissive	Permissive
Signal Group	4	0	1	6	2	0
Auxiliary Signal Groups						
Lead / Lag	Lead	-	Lead	-	-	-
Minimum Green [s]	5	0	5	5	5	0
Maximum Green [s]	30	0	30	30	30	0
Amber [s]	3.0	0.0	3.0	3.0	3.0	0.0
All red [s]	1.0	0.0	1.0	1.0	1.0	0.0
Split [s]	27	0	26	109	83	0
Vehicle Extension [s]	3.0	0.0	3.0	3.0	3.0	0.0
Walk [s]	1	0	0	1	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	0.0	2.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	2.0	0.0	2.0	2.0	2.0	0.0
Minimum Recall	no		no	no	no	
Maximum Recall	no		no	no	no	
Pedestrian Recall	no		no	no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	R	L	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	23	23	22	105	79
g / C, Green / Cycle	0.17	0.17	0.16	0.77	0.58
(v / s)_i Volume / Saturation Flow Rate	0.11	0.07	0.11	0.23	0.40
Total Saturation Flow Adjustment	0.93	0.84	0.93	0.87	0.87
s, saturation flow rate [veh/h]	1766	1604	1766	4939	4968
c, Capacity [veh/h]	299	271	286	3813	2886
d1, Uniform Delay [s]	52.97	50.44	53.81	4.58	19.85
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	11.50	4.52	13.00	0.20	1.35
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.33	1.24	1.33
PF, progression factor	1.00	1.00	1.00	0.23	0.62

Lane Group Results

X, volume / capacity	0.67	0.41	0.69	0.30	0.69
d, Delay for Lane Group [s/veh]	64.47	54.96	66.81	1.27	13.64
Lane Group LOS	E	D	E	A	B
Critical Lane Group	yes	no	yes	no	yes
50th-Percentile Queue Length [veh]	8.34	4.16	8.18	1.82	16.42
50th-Percentile Queue Length [ft]	208.58	103.96	204.51	45.49	410.61
95th-Percentile Queue Length [veh]	14.92	8.46	14.68	4.18	26.89
95th-Percentile Queue Length [ft]	373.05	211.59	367.04	104.39	672.36

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	64.47	54.96	66.81	1.27	13.64	13.64
Movement LOS	E	D	E	A	B	B
d_A, Approach Delay [s/veh]	61.09		11.04		13.64	
Approach LOS	E		B		B	
d_I, Intersection Delay [s/veh]	16.78					
Intersection LOS	B					
Intersection V/C	0.685					

Sequence

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#36: El Camino Real (SR 82)/Cambridge Ave**

Control Type:	Signalized	Delay (sec / veh):	4.3
Analysis Method:	HCM2000	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.591

Intersection Setup

Name	Cambridge Avenue						El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	↔↔			+			↔↔↔			↔↔↔		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	1	0	0	0	1	0	0	1	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	370.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	25.00			15.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Cambridge Avenue						El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	28	0	50	1	0	0	119	1204	1	30	2068	15
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	3.60	0.00	0.00	0.00	0.00	0.00	0.80	3.70	0.00	6.70	2.60	6.70
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	54	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	28	0	0	1	0	0	119	1204	1	30	2068	15
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	7	0	0	0	0	0	31	317	0	8	544	4
Total Analysis Volume [veh/h]	29	0	0	1	0	0	125	1267	1	32	2177	16
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	6			0			0			6		
Bicycle Volume [bicycles/h]	0			0			5			5		

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	111.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	8	0	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	0	5	0	0	5	0	5	5	0	5	5	0
Maximum Green [s]	0	30	0	0	30	0	30	30	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	0	9	0	0	9	0	21	108	0	19	106	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
Walk [s]	0	1	0	0	1	0	0	1	0	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	no		no	no	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	50.0	0.0	0.0	50.0	0.0	50.0	50.0	0.0	50.0	50.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	R	L	L	C	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	2.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	5	5	5	17	104	15	102
g / C, Green / Cycle	0.04	0.04	0.04	0.13	0.76	0.11	0.75
(v / s)_i Volume / Saturation Flow Rate	0.02	0.00	0.00	0.07	0.25	0.02	0.44
Total Saturation Flow Adjustment	0.92	0.85	0.80	0.94	0.88	0.89	0.88
s, saturation flow rate [veh/h]	1742	1615	1520	1791	4990	1692	5039
c, Capacity [veh/h]	64	59	56	224	3816	187	3779
d1, Uniform Delay [s]	64.16	63.09	63.13	55.97	5.05	54.87	7.52
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	21.39	0.00	0.59	9.70	0.23	1.99	0.66
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.33	1.25	1.33	1.33
PF, progression factor	1.00	1.00	1.00	1.00	0.22	1.00	0.00

Lane Group Results

X, volume / capacity	0.45	0.00	0.02	0.56	0.33	0.17	0.58
d, Delay for Lane Group [s/veh]	85.55	63.09	63.72	65.67	1.37	56.85	0.66
Lane Group LOS	F	E	E	E	A	E	A
Critical Lane Group	yes	no	no	yes	no	no	yes
50th-Percentile Queue Length [veh]	1.25	0.00	0.04	4.98	2.13	1.15	2.58
50th-Percentile Queue Length [ft]	31.26	0.00	1.00	124.55	53.36	28.87	64.48
95th-Percentile Queue Length [veh]	2.97	0.00	0.10	9.81	4.81	2.76	5.67
95th-Percentile Queue Length [ft]	74.35	0.00	2.60	245.27	120.20	69.11	141.67

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	85.55	85.55	63.09	63.72	63.72	63.72	65.67	1.37	1.37	56.85	0.66	0.66
Movement LOS	F	F	E	E	E	E	E	A	A	E	A	A
d_A, Approach Delay [s/veh]	85.55			63.72			7.14			1.47		
Approach LOS	F			E			A			A		
d_I, Intersection Delay [s/veh]	4.32											
Intersection LOS	A											
Intersection V/C	0.591											

Sequence

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#38: Santa Cruz Ave/University Dr (S)**

Control Type:	Signalized	Delay (sec / veh):	16.9
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.601

Intersection Setup

Name	Santa Cruz Avenue		Santa Cruz Avenue		University Drive	
Approach	Northeastbound		Southwestbound		Northwestbound	
Lane Configuration	← →		← →		← →	
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	1	1	0	1	0
Pocket Length [ft]	100.00	100.00	60.00	100.00	100.00	100.00
Speed [mph]	30.00		25.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Santa Cruz Avenue		Santa Cruz Avenue		University Drive	
Base Volume Input [veh/h]	397	386	59	340	302	67
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.93	1.92	2.00	1.94	1.90	1.74
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	397	386	59	340	302	67
Peak Hour Factor	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	107	104	16	91	81	18
Total Analysis Volume [veh/h]	427	415	63	366	325	72
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		21		23	
Bicycle Volume [bicycles/h]	11		91		16	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	60
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	12.00

Phasing & Timing

Control Type	Permissive	Permissive	Protected	Permissive	Split	Split
Signal Group	2	2	1	6	8	8
Auxiliary Signal Groups						
Lead / Lag	-	-	Lead	-	Lead	-
Minimum Green [s]	4	4	5	4	4	4
Maximum Green [s]	16	16	30	16	16	16
Amber [s]	3.5	3.5	3.0	3.5	3.5	3.5
All red [s]	0.5	0.5	1.0	0.5	0.5	0.5
Split [s]	29	29	9	38	22	22
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	0	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no		no	no	no	
Maximum Recall	yes		no	yes	no	
Pedestrian Recall	yes		no	yes	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	6.0	6.0	20.0	20.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	L	C	L	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	25	25	5	34	18	18
g / C, Green / Cycle	0.42	0.42	0.08	0.57	0.30	0.30
(v / s)_i Volume / Saturation Flow Rate	0.23	0.26	0.04	0.20	0.18	0.05
Total Saturation Flow Adjustment	0.98	0.83	0.93	0.98	0.93	0.84
s, saturation flow rate [veh/h]	1864	1585	1770	1864	1771	1587
c, Capacity [veh/h]	777	660	147	1056	531	476
d1, Uniform Delay [s]	13.24	13.83	26.14	7.01	18.00	15.40
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	2.79	4.49	8.80	0.90	5.18	0.67
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

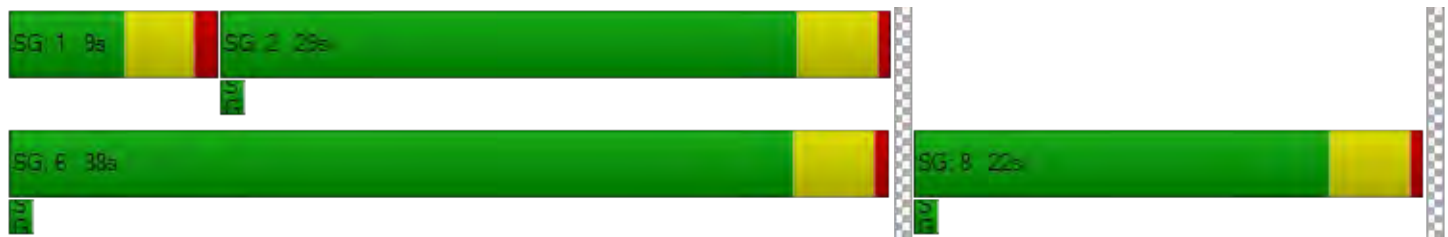
X, volume / capacity	0.55	0.63	0.43	0.35	0.61	0.15
d, Delay for Lane Group [s/veh]	16.03	18.32	34.94	7.91	23.18	16.07
Lane Group LOS	B	B	C	A	C	B
Critical Lane Group	no	yes	yes	no	yes	no
50th-Percentile Queue Length [veh]	6.25	6.52	1.16	3.76	5.49	0.97
50th-Percentile Queue Length [ft]	156.20	162.97	29.09	94.01	137.15	24.27
95th-Percentile Queue Length [veh]	11.79	12.20	2.78	7.79	10.61	2.35
95th-Percentile Queue Length [ft]	294.68	304.99	69.58	194.73	265.22	58.82

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	16.03	18.32	34.94	7.91	23.18	16.07
Movement LOS	B	B	C	A	C	B
d_A, Approach Delay [s/veh]	17.16		11.88		21.89	
Approach LOS	B		B		C	
d_I, Intersection Delay [s/veh]	16.93					
Intersection LOS	B					
Intersection V/C	0.601					

Sequence

Ring 1	1	2	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#39: Santa Cruz Ave/Sand Hill Rd**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 52.6
 Level Of Service: D
 Volume to Capacity (v/c): 0.782

Intersection Setup

Name	Sand Hill Road			Sand Hill Road			Santa Cruz Avenue			Santa Cruz Avenue		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	三			三			三			三		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	2	0	1	2	0	1	2	0	1	2	0	1
Pocket Length [ft]	210.00	100.00	240.00	135.00	100.00	140.00	100.00	100.00	100.00	150.00	100.00	180.00
Speed [mph]	35.00			35.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Sand Hill Road			Sand Hill Road			Santa Cruz Avenue			Santa Cruz Avenue		
Base Volume Input [veh/h]	231	1143	261	331	559	51	94	615	389	223	669	236
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	3.50	1.40	4.20	1.50	3.40	3.90	8.50	3.90	1.50	0.90	3.40	4.70
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	144	0	0	38	0	0	288	0	0	281
Total Hourly Volume [veh/h]	231	1143	117	331	559	13	94	615	101	223	669	0
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	61	304	31	88	149	3	25	164	27	59	178	0
Total Analysis Volume [veh/h]	246	1216	124	352	595	14	100	654	107	237	712	0
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	3			7			5			7		
Bicycle Volume [bicycles/h]	33			33			29			49		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	150
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Protecte
Signal Group	5	2	2	1	6	6	3	8	8	7	4	4
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	5	4	4	5	4	4	5	4	4	5	4	4
Maximum Green [s]	30	16	16	30	16	16	30	16	16	30	16	16
Amber [s]	3.0	3.5	3.5	3.0	3.5	3.5	3.0	3.5	3.5	3.0	3.5	3.5
All red [s]	1.0	0.5	0.5	1.0	0.5	0.5	1.0	0.5	0.5	1.0	0.5	0.5
Split [s]	35	72	72	22	59	59	13	39	39	17	43	43
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	0	1	1	0	1	1	0	1	1	0	1	1
Pedestrian Clearance [s]	0	1	1	0	1	1	0	1	1	0	1	1
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no	no		no	no		no	no		no	no	no
Maximum Recall	no	yes		no	yes		no	no		no	no	no
Pedestrian Recall	no	yes		no	yes		no	no		no	no	no
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	R	L	C	R	L	C	R	L	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	31	68	68	18	55	55	9	35	35	13	39	39
g / C, Green / Cycle	0.21	0.45	0.45	0.12	0.37	0.37	0.06	0.23	0.23	0.09	0.26	0.26
(v / s)_i Volume / Saturation Flow Rate	0.07	0.34	0.08	0.10	0.17	0.01	0.03	0.19	0.07	0.07	0.20	0.00
Total Saturation Flow Adjustment	0.89	0.94	0.82	0.91	0.92	0.82	0.85	0.92	0.84	0.91	0.92	0.81
s, saturation flow rate [veh/h]	3387	3568	1550	3454	3499	1554	3231	3482	1591	3474	3499	1543
c, Capacity [veh/h]	700	1617	703	414	1283	570	194	812	371	301	910	401
d1, Uniform Delay [s]	50.90	34.00	24.36	64.67	36.25	30.36	68.39	54.28	47.26	67.14	51.56	41.07
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	1.39	3.28	0.55	19.11	1.21	0.08	9.48	8.35	1.95	18.53	6.67	0.00
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.35	0.75	0.18	0.85	0.46	0.02	0.52	0.81	0.29	0.79	0.78	0.00
d, Delay for Lane Group [s/veh]	52.29	37.28	24.91	83.78	37.46	30.44	77.87	62.63	49.21	85.67	58.24	41.07
Lane Group LOS	D	D	C	F	D	C	E	E	D	F	E	D
Critical Lane Group	no	yes	no	yes	no	no	no	yes	no	yes	no	no
50th-Percentile Queue Length [veh]	4.94	25.92	3.34	9.46	10.98	0.40	2.41	16.41	3.99	6.24	17.35	0.00
50th-Percentile Queue Length [ft]	123.56	648.01	83.56	236.60	274.59	10.01	60.25	410.35	99.79	156.00	433.71	0.00
95th-Percentile Queue Length [veh]	9.75	41.62	7.06	16.57	18.79	1.01	5.34	26.88	8.18	11.78	28.30	0.00
95th-Percentile Queue Length [ft]	243.67	1040.45	176.52	414.21	469.87	25.26	133.61	671.95	204.58	294.39	707.43	0.00

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	52.29	37.28	24.91	83.78	37.46	30.44	77.87	62.63	49.21	85.67	58.24	41.07
Movement LOS	D	D	C	F	D	C	E	E	D	F	E	D
d_A, Approach Delay [s/veh]	38.64			54.32			62.73			65.09		
Approach LOS	D			D			E			E		
d_I, Intersection Delay [s/veh]	52.62											
Intersection LOS	D											
Intersection V/C	0.782											

Sequence

Ring 1	1	2	3	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	7	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#58: University Avenue and Adams Drive**

Control Type:	Two-way stop	Delay (sec / veh):	146.8
Analysis Method:	HCM2000	Level Of Service:	F
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.196

Intersection Setup

Name	University Avenue			University Avenue			Adams Drive			Adams Drive		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	270.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	no			no			no			no		

Volumes

Name	University Avenue			University Avenue			Adams Drive			Adams Drive		
Base Volume Input [veh/h]	42	508	0	0	1449	140	6	0	1	0	0	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	14.30	4.70	2.00	2.00	2.60	4.30	5.00	0.00	0.00	2.00	2.00	2.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	42	508	0	0	1449	140	6	0	1	0	0	0
Peak Hour Factor	0.9300	0.9300	1.0000	1.0000	0.9300	0.9300	0.9300	0.9300	0.9300	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	11	137	0	0	390	38	2	0	0	0	0	0
Total Analysis Volume [veh/h]	45	546	0	0	1558	151	6	0	1	0	0	0
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings

Priority Scheme	Free	Free	Stop	Stop
Flared Lane			no	
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance			no	
Number of Storage Spaces in Median	0	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.14	0.01	0.00	0.00	0.02	0.00	0.20	0.00	0.00	0.00	0.00	0.00
d_M, Delay for Movement [s/veh]	18.21	0.00	0.00	0.00	0.00	0.00	146.82	131.34	40.78	0.00	0.00	0.00
Movement LOS	C	A			A	A	F	F	E			
95th-Percentile Queue Length [veh]	0.49	0.00	0.00	0.00	0.00	0.00	0.63	0.63	0.63	0.00	0.00	0.00
95th-Percentile Queue Length [ft]	12.21	0.00	0.00	0.00	0.00	0.00	15.84	15.84	15.84	0.00	0.00	0.00
d_A, Approach Delay [s/veh]	1.39			0.00			131.67			0.00		
Approach LOS	A			A			F			A		
d_I, Intersection Delay [s/veh]	0.75											
Intersection LOS	F											

**Intersection Level Of Service Report
#74: University Ave/O'Brien Dr**

Control Type:	Signalized	Delay (sec / veh):	3.7
Analysis Method:	HCM2000	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.569

Intersection Setup

Name	University Avenue		University Avenue		O'Brien Drive	
Approach	Northbound		Southbound		Eastbound	
Lane Configuration	⇐		⇐		⇐⇐	
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	0	1	0
Pocket Length [ft]	90.00	100.00	100.00	100.00	45.00	100.00
Speed [mph]	25.00		25.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	University Avenue		University Avenue		O'Brien Drive	
Base Volume Input [veh/h]	67	532	1284	142	16	25
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.50	5.10	2.90	3.50	18.80	4.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	67	532	1284	142	16	25
Peak Hour Factor	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	17	139	334	37	4	7
Total Analysis Volume [veh/h]	70	554	1338	148	17	26
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		1		0	
Bicycle Volume [bicycles/h]	7		5		1	

Intersection Settings

Located in CBD	yes
Signal Coordination Group	-
Cycle Length [s]	85
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	11.00

Phasing & Timing

Control Type	Protected	Permissive	Permissive	Permissive	Split	Split
Signal Group	0	2	6	0	3	0
Auxiliary Signal Groups						
Lead / Lag	-	-	-	-	Lead	-
Minimum Green [s]	0	5	5	0	5	0
Maximum Green [s]	0	30	30	0	30	0
Amber [s]	0.0	3.5	3.5	0.0	3.5	0.0
All red [s]	0.0	0.5	1.0	0.0	1.0	0.0
Split [s]	0	75	75	0	10	0
Vehicle Extension [s]	0.0	3.0	3.0	0.0	3.0	0.0
Walk [s]	0	1	1	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	2.5	0.0	2.5	0.0
Minimum Recall		no	no		no	
Maximum Recall		yes	yes		no	
Pedestrian Recall		no	no		no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	20.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	L	R
L, Total Lost Time per Cycle [s]	4.00	4.50	4.50	4.50
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.50	2.50	2.50
g_i, Effective Green Time [s]	71	71	6	6
g / C, Green / Cycle	0.84	0.83	0.06	0.06
(v / s)_i Volume / Saturation Flow Rate	0.18	0.48	0.01	0.02
Total Saturation Flow Adjustment	0.82	0.82	0.72	0.74
s, saturation flow rate [veh/h]	3098	3117	1367	1398
c, Capacity [veh/h]	2588	2585	88	90
d1, Uniform Delay [s]	1.40	2.36	37.65	37.88
k, delay calibration	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.19	0.94	4.78	7.84
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.21	0.57	0.19	0.29
d, Delay for Lane Group [s/veh]	1.59	3.30	42.42	45.72
Lane Group LOS	A	A	D	D
Critical Lane Group	no	yes	no	yes
50th-Percentile Queue Length [veh]	1.75	7.80	0.43	0.67
50th-Percentile Queue Length [ft]	43.69	195.03	10.69	16.66
95th-Percentile Queue Length [veh]	4.03	14.12	1.08	1.65
95th-Percentile Queue Length [ft]	100.70	353.02	26.92	41.25

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	0.00	1.59	3.30	3.30	42.42	45.72
Movement LOS		A	A	A	D	D
d_A, Approach Delay [s/veh]	1.59		3.30		44.42	
Approach LOS	A		A		D	
d_I, Intersection Delay [s/veh]	3.70					
Intersection LOS	A					
Intersection V/C	0.569					

Sequence

Ring 1	2	3	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#88: Valparaiso Ave/ University Dr**

Control Type:	Signalized	Delay (sec / veh):	19.8
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.675

Intersection Setup

Name	Valparaiso Ave			Valparaiso Ave			University Drive (North)			University Drive		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	⇌			⇌			⊕			⇌		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	0	0	0	1	0	0
Pocket Length [ft]	100.00	100.00	100.00	75.00	100.00	100.00	100.00	100.00	100.00	35.00	100.00	100.00
Speed [mph]	35.00			35.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Valparaiso Ave			Valparaiso Ave			University Drive (North)			University Drive		
Base Volume Input [veh/h]	152	429	122	42	550	59	56	85	33	71	51	62
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	1.90	0.80	2.40	4.20	0.00	0.00	8.70	0.00	0.00	13.70	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	152	429	122	42	550	59	56	85	33	71	51	62
Peak Hour Factor	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	45	128	36	13	164	18	17	25	10	21	15	18
Total Analysis Volume [veh/h]	181	511	145	50	655	70	67	101	39	85	61	74
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	54			1			12			21		
Bicycle Volume [bicycles/h]	26			2			7			1		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	85
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	4.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	5	2	0	1	6	0	0	8	0	0	4	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	-	-	-	-	-	-
Minimum Green [s]	5	5	0	5	5	0	0	5	0	0	5	0
Maximum Green [s]	30	30	0	30	30	0	0	30	0	0	30	0
Amber [s]	3.0	3.0	0.0	3.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
All red [s]	1.0	1.0	0.0	1.0	1.0	0.0	0.0	1.0	0.0	0.0	1.0	0.0
Split [s]	9	53	0	9	53	0	0	23	0	0	23	0
Vehicle Extension [s]	3.0	3.0	0.0	3.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	1	0	0	1	0	0	1	0	0	1	0
Pedestrian Clearance [s]	0	1	0	0	1	0	0	1	0	0	1	0
I1, Start-Up Lost Time [s]	2.0	2.0	0.0	2.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	2.0	2.0	0.0	2.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	yes		no	yes			no			no	
Pedestrian Recall	no	yes		no	yes			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	2.00	2.00	0.00
l2, Clearance Lost Time [s]	0.00	2.00	0.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	58	49	58	49	19	19	19
g / C, Green / Cycle	0.68	0.58	0.68	0.58	0.22	0.22	0.22
(v / s)_i Volume / Saturation Flow Rate	0.30	0.36	0.07	0.40	0.16	0.09	0.09
Total Saturation Flow Adjustment	0.32	0.95	0.35	0.95	0.69	0.50	0.81
s, saturation flow rate [veh/h]	610	1803	670	1797	1312	943	1534
c, Capacity [veh/h]	416	1039	457	1036	293	211	343
d1, Uniform Delay [s]	7.97	11.98	6.16	12.78	30.43	28.16	28.10
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	3.28	2.91	0.48	3.94	13.39	5.65	3.37
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

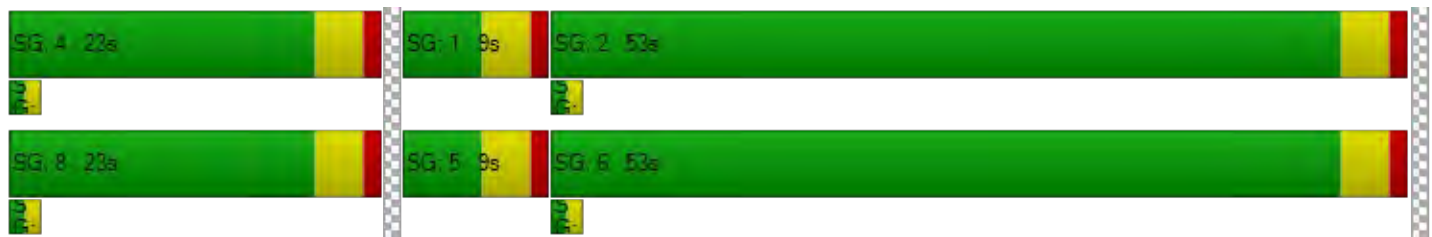
X, volume / capacity	0.43	0.63	0.11	0.70	0.71	0.40	0.39
d, Delay for Lane Group [s/veh]	11.25	14.90	6.64	16.72	43.81	33.82	31.47
Lane Group LOS	B	B	A	B	D	C	C
Critical Lane Group	no	no	no	yes	yes	no	no
50th-Percentile Queue Length [veh]	2.38	12.17	0.48	14.62	5.53	1.96	3.05
50th-Percentile Queue Length [ft]	59.51	304.26	12.08	365.49	138.15	48.96	76.16
95th-Percentile Queue Length [veh]	5.29	20.54	1.21	24.18	10.67	4.46	6.53
95th-Percentile Queue Length [ft]	132.18	513.49	30.29	604.42	266.79	111.42	163.28

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	11.25	14.90	14.90	6.64	16.72	16.72	43.81	43.81	43.81	33.82	31.47	31.47
Movement LOS	B	B	B	A	B	B	D	D	D	C	C	C
d_A, Approach Delay [s/veh]	14.11			16.07			43.81			32.37		
Approach LOS	B			B			D			C		
d_I, Intersection Delay [s/veh]	19.84											
Intersection LOS	B											
Intersection V/C	0.675											

Sequence

Ring 1	4	1	2	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	8	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#103: Addison Wesley/Sand Hill Rd**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 155.5
 Level Of Service: F
 Volume to Capacity (v/c): 0.668

Intersection Setup

Name	Addison-Wesley			Addison-Wesley			Sand Hill Rd			Sand Hill Rd		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	⇐⇐⇐			⇐⇐			⇐⇐⇐			⇐⇐		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	1	0	1	1	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	230.00	100.00	230.00	240.00	100.00	100.00
Speed [mph]	25.00			25.00			40.00			40.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Addison-Wesley			Addison-Wesley			Sand Hill Rd			Sand Hill Rd		
Base Volume Input [veh/h]	23	2	41	2	2	7	103	1888	233	113	852	39
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	23	2	41	2	2	7	103	1888	233	113	852	39
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	6	1	11	1	1	2	27	502	62	30	227	10
Total Analysis Volume [veh/h]	24	2	44	2	2	7	110	2009	248	120	906	41
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	4			2			1			6		
Bicycle Volume [bicycles/h]	0			0			27			22		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	100
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	9.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Split	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	8	0	0	4	0	5	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lag	-	-	Lead	-	-
Minimum Green [s]	0	6	0	0	6	0	4	10	0	4	10	0
Maximum Green [s]	0	0	0	0	0	0	0	0	0	0	0	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	4.0	0.0	3.0	4.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	0	13	0	0	10	0	11	41	0	8	38	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	6.0	0.0	3.0	6.0	0.0
Walk [s]	0	7	0	0	0	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	30	0	0	0	0	0	24	0	0	19	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no			no		no	yes		no	yes	
Maximum Recall		no			no		no	no		no	no	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	0.0	0.0	20.0	0.0	0.0	20.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	L	C	L	C	R	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	11	11	8	8	9	39	39	6	36
g / C, Green / Cycle	0.11	0.11	0.08	0.08	0.09	0.39	0.39	0.06	0.36
(v / s)_i Volume / Saturation Flow Rate	0.01	0.03	0.00	0.01	0.06	0.57	0.16	0.07	0.27
Total Saturation Flow Adjustment	0.89	0.83	0.93	0.87	0.93	0.93	0.80	0.93	0.93
s, saturation flow rate [veh/h]	3390	1577	1770	1645	1770	3547	1522	1770	3519
c, Capacity [veh/h]	373	173	142	132	159	1383	594	106	1267
d1, Uniform Delay [s]	39.91	40.74	42.37	42.55	44.15	30.50	22.23	47.00	28.02
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.36	3.49	0.18	1.00	21.82	207.69	2.16	126.66	4.06
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

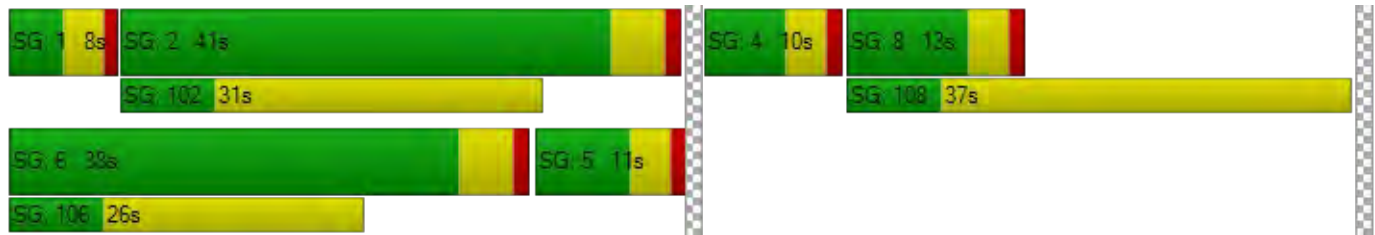
X, volume / capacity	0.07	0.25	0.01	0.07	0.69	1.45	0.42	1.13	0.75
d, Delay for Lane Group [s/veh]	40.27	44.23	42.55	43.55	65.97	238.19	24.38	173.66	32.08
Lane Group LOS	D	D	D	D	E	F	C	F	C
Critical Lane Group	no	yes	no	yes	no	yes	no	yes	no
50th-Percentile Queue Length [veh]	0.37	1.24	0.06	0.25	3.65	73.34	5.62	6.34	14.55
50th-Percentile Queue Length [ft]	9.24	31.02	1.39	6.33	91.20	1833.48	140.60	158.46	363.73
95th-Percentile Queue Length [veh]	0.93	2.95	0.14	0.65	7.60	117.34	10.82	11.93	24.07
95th-Percentile Queue Length [ft]	23.37	73.83	3.60	16.14	189.88	2933.57	270.61	298.14	601.79

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	40.27	40.27	44.23	42.55	43.55	43.55	65.97	238.19	24.38	173.66	32.08	32.08
Movement LOS	D	D	D	D	D	D	E	F	C	F	C	C
d_A, Approach Delay [s/veh]	42.76			43.37			207.78			48.01		
Approach LOS	D			D			F			D		
d_I, Intersection Delay [s/veh]	155.48											
Intersection LOS	F											
Intersection V/C	0.668											

Sequence

Ring 1	1	2	4	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	5	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Intersection Level Of Service Report
#107: Alpine Rd/Santa Cruz Ave&Junipero Serra Blvd

Control Type:	Signalized	Delay (sec / veh):	46.4
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.784

Intersection Setup

Name	Santa Cruz Avenue		Junipero Serra Blvd		Alpine Road	
Approach	Southbound		Westbound		Northeastbound	
Lane Configuration	TTT		TT		TT	
Turning Movement	Left	Thru	Left	Right	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	1	0	0	0
Pocket Length [ft]	210.00	100.00	300.00	100.00	100.00	100.00
Speed [mph]	40.00		35.00		40.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Santa Cruz Avenue		Junipero Serra Blvd		Alpine Road	
Base Volume Input [veh/h]	663	556	65	172	852	517
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.10	4.00	3.10	1.70	2.70	0.20
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	663	556	65	172	852	517
Peak Hour Factor	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	178	149	17	46	229	139
Total Analysis Volume [veh/h]	713	598	70	185	916	556
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		1		1	
Bicycle Volume [bicycles/h]	43		21		15	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	150
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	10.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Split
Signal Group	0	7	1	0	3	0
Auxiliary Signal Groups						
Lead / Lag	-	-	Lead	-	-	-
Minimum Green [s]	0	5	5	0	5	0
Maximum Green [s]	0	30	30	0	30	0
Amber [s]	0.0	3.0	3.0	0.0	3.0	0.0
All red [s]	0.0	1.0	1.0	0.0	1.0	0.0
Split [s]	0	55	26	0	69	0
Vehicle Extension [s]	0.0	3.0	3.0	0.0	3.0	0.0
Walk [s]	0	1	1	0	1	0
Pedestrian Clearance [s]	0	1	1	0	1	0
I1, Start-Up Lost Time [s]	0.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	2.0	0.0	2.0	0.0
Minimum Recall		no	no		no	
Maximum Recall		no	no		no	
Pedestrian Recall		no	no		no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	20.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	L	R	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	51	22	22	65	65
g / C, Green / Cycle	0.34	0.15	0.15	0.43	0.43
(v / s)_i Volume / Saturation Flow Rate	0.27	0.04	0.12	0.26	0.34
Total Saturation Flow Adjustment	0.85	0.92	0.84	0.93	0.85
s, saturation flow rate [veh/h]	4845	1751	1588	3522	1612
c, Capacity [veh/h]	1647	257	233	1526	698
d1, Uniform Delay [s]	44.79	56.89	61.81	32.55	36.77
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	4.08	2.61	23.75	1.75	9.15
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.80	0.27	0.79	0.60	0.80
d, Delay for Lane Group [s/veh]	48.87	59.49	85.56	34.30	45.91
Lane Group LOS	D	E	F	C	D
Critical Lane Group	yes	no	yes	no	yes
50th-Percentile Queue Length [veh]	21.77	2.83	9.20	17.35	24.08
50th-Percentile Queue Length [ft]	544.28	70.66	230.08	433.77	602.07
95th-Percentile Queue Length [veh]	35.11	6.13	16.19	28.30	38.73
95th-Percentile Queue Length [ft]	877.84	153.21	404.65	707.53	968.18

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	48.87	48.87	59.49	85.56	34.30	45.91
Movement LOS	D	D	E	F	C	D
d_A, Approach Delay [s/veh]	48.87		78.41		38.69	
Approach LOS	D		E		D	
d_I, Intersection Delay [s/veh]	46.42					
Intersection LOS	D					
Intersection V/C	0.784					

Sequence

Ring 1	1	3	7	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#110: Marsh Road and US 101 NB Ramps**

Control Type:	Signalized	Delay (sec / veh):	17.1
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.681

Intersection Setup

Name	Marsh Road		Marsh Road		Northwestbound	
Approach	Northbound		Southbound			
Lane Configuration	↑↑		↑↑		11↑	
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	1	1
Pocket Length [ft]	100.00	100.00	100.00	100.00	500.00	360.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	no		yes		yes	

Volumes

Name	Marsh Road		Marsh Road		Northwestbound	
Base Volume Input [veh/h]	1097	0	0	1261	848	349
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	4.50	0.00	0.00	5.20	1.90	4.30
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	1097	0	0	1261	848	349
Peak Hour Factor	0.9000	1.0000	1.0000	0.9000	0.9000	0.9000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	305	0	0	350	236	97
Total Analysis Volume [veh/h]	1219	0	0	1401	942	388
Presence of On-Street Parking	no			no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		0		0	
Bicycle Volume [bicycles/h]	0		0		0	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	80
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	35.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permissive	Permissive	Permissive	Permissive	Protected	Overlap
Signal Group	2	0	0	6	8	1
Auxiliary Signal Groups						1,6,8
Lead / Lag	-	-	-	-	Lag	-
Minimum Green [s]	8	0	0	8	6	4
Maximum Green [s]	16	0	0	16	16	30
Amber [s]	3.6	0.0	0.0	3.6	3.2	3.2
All red [s]	0.5	0.0	0.0	0.5	1.0	0.0
Split [s]	41	0	0	50	30	9
Vehicle Extension [s]	3.0	0.0	0.0	3.0	3.0	2.0
Walk [s]	7	0	0	0	7	0
Pedestrian Clearance [s]	18	0	0	0	20	0
I1, Start-Up Lost Time [s]	2.0	0.0	0.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	yes			yes	no	no
Maximum Recall	no			no	no	no
Pedestrian Recall	no			no	no	no
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	0.0	0.0	20.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	L	R
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	39	48	28	78
g / C, Green / Cycle	0.49	0.60	0.35	0.98
(v / s)_i Volume / Saturation Flow Rate	0.35	0.41	0.27	0.25
Total Saturation Flow Adjustment	0.91	0.90	0.91	0.81
s, saturation flow rate [veh/h]	3462	3439	3440	1548
c, Capacity [veh/h]	1688	2063	1204	1510
d1, Uniform Delay [s]	16.22	10.80	23.27	0.03
k, delay calibration	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	2.72	1.82	5.11	0.41
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00

Lane Group Results

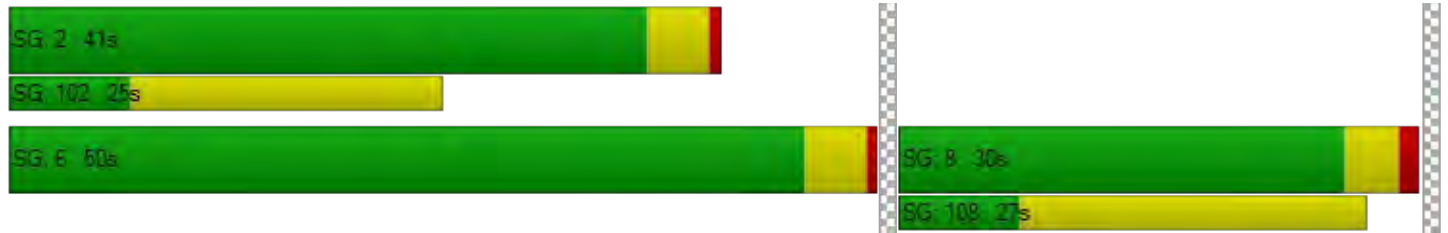
X, volume / capacity	0.72	0.68	0.78	0.26
d, Delay for Lane Group [s/veh]	18.93	12.62	28.38	0.45
Lane Group LOS	B	B	C	A
Critical Lane Group	no	yes	yes	no
50th-Percentile Queue Length [veh]	13.59	13.28	12.02	0.77
50th-Percentile Queue Length [ft]	339.79	331.90	300.53	19.28
95th-Percentile Queue Length [veh]	22.64	22.17	20.32	1.90
95th-Percentile Queue Length [ft]	566.08	554.36	508.00	47.38

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	18.93	0.00	0.00	12.62	28.38	0.45
Movement LOS	B			B	C	A
d_A, Approach Delay [s/veh]	18.93		12.62		20.23	
Approach LOS	B		B		C	
d_I, Intersection Delay [s/veh]	17.13					
Intersection LOS	B					
Intersection V/C	0.681					

Sequence

Ring 1	-	2	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#132: Oak Ave/Sand Hill Rd**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 18.2
 Level Of Service: B
 Volume to Capacity (v/c): 0.789

Intersection Setup

Name	Sand Hill Road		Sand Hill Road		Oak Avenue	
Approach	Northeastbound		Southwestbound		Southeastbound	
Lane Configuration			H		TH	
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	1	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	95.00	100.00
Speed [mph]	35.00		35.00		25.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Sand Hill Road		Sand Hill Road		Oak Avenue	
Base Volume Input [veh/h]	0	1794	740	38	77	189
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	1.20	1.90	2.60	0.00	0.50
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	1794	740	38	77	189
Peak Hour Factor	1.0000	0.8900	0.8900	0.8900	0.8900	0.8900
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	504	208	11	22	53
Total Analysis Volume [veh/h]	0	2016	831	43	87	212
Presence of On-Street Parking		no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	27		0		5	
Bicycle Volume [bicycles/h]	31		28		9	

Intersection Settings

Located in CBD	yes
Signal Coordination Group	-
Cycle Length [s]	150
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fixed time
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	3.00

Phasing & Timing

Control Type	Permissive	Permissive	Permissive	Permissive	Split	Split
Signal Group	0	2	6	0	3	0
Auxiliary Signal Groups						
Lead / Lag	-	-	-	-	Lead	-
Minimum Green [s]	0	5	5	0	5	0
Maximum Green [s]	0	30	30	0	30	0
Amber [s]	0.0	3.0	3.0	0.0	3.0	0.0
All red [s]	0.0	1.0	1.0	0.0	1.0	0.0
Split [s]	0	117	117	0	33	0
Vehicle Extension [s]	0.0	3.0	3.0	0.0	3.0	0.0
Walk [s]	0	1	1	0	1	0
Pedestrian Clearance [s]	0	1	1	0	1	0
I1, Start-Up Lost Time [s]	0.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	2.0	0.0	2.0	0.0
Minimum Recall		no	no		no	
Maximum Recall		yes	yes		no	
Pedestrian Recall		yes	yes		no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	20.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	L	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	113	113	29	29
g / C, Green / Cycle	0.75	0.75	0.19	0.19
(v / s)_i Volume / Saturation Flow Rate	0.63	0.28	0.05	0.15
Total Saturation Flow Adjustment	0.85	0.83	0.86	0.76
s, saturation flow rate [veh/h]	3217	3172	1625	1446
c, Capacity [veh/h]	2424	2389	314	280
d1, Uniform Delay [s]	12.22	6.30	51.56	57.19
k, delay calibration	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	3.51	0.43	2.18	17.40
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.83	0.37	0.28	0.76
d, Delay for Lane Group [s/veh]	15.73	6.73	53.75	74.59
Lane Group LOS	B	A	D	E
Critical Lane Group	yes	no	no	yes
50th-Percentile Queue Length [veh]	36.62	7.60	3.37	10.09
50th-Percentile Queue Length [ft]	915.53	190.12	84.13	252.24
95th-Percentile Queue Length [veh]	58.62	13.83	7.10	17.48
95th-Percentile Queue Length [ft]	1465.45	345.73	177.52	437.12

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	0.00	15.73	6.73	6.73	53.75	74.59
Movement LOS		B	A	A	D	E
d_A, Approach Delay [s/veh]	15.73		6.73		68.52	
Approach LOS	B		A		E	
d_I, Intersection Delay [s/veh]	18.22					
Intersection LOS	B					
Intersection V/C	0.789					

Sequence

Ring 1	2	3	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#156: Saga Ln/Sand Hill Rd**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 54.3
 Level Of Service: D
 Volume to Capacity (v/c): 0.605

Intersection Setup

Name	Saga Ln			Saga Ln			Sand Hill Road			Sand Hill Road		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	↕↔			↕↔			↔↕↔			↔↕↔		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	1	0	0	0	1	0	0	1	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	200.00	100.00	100.00	230.00	100.00	100.00
Speed [mph]	25.00			25.00			40.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Saga Ln			Saga Ln			Sand Hill Road			Sand Hill Road		
Base Volume Input [veh/h]	10	0	33	14	1	9	211	1594	117	73	760	50
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	10	0	33	14	1	9	211	1594	117	73	760	50
Peak Hour Factor	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	3	0	9	4	0	2	58	438	32	20	209	14
Total Analysis Volume [veh/h]	11	0	36	15	1	10	232	1752	129	80	835	55
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	2			10			7			0		
Bicycle Volume [bicycles/h]	1			7			23			14		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	100
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	62.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	4	0	5	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	0	12	0	0	12	0	4	10	0	6	10	0
Maximum Green [s]	0	30	0	0	30	0	30	30	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	4.0	0.0	3.0	4.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	0	28	0	0	28	0	25	55	0	17	47	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	5.0	0.0	3.0	5.0	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	27	0	0	27	0	0	20	0	0	15	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	yes		no	yes	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	0.0	0.0	20.0	0.0	0.0	20.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	R	C	R	L	C	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	26	26	26	26	23	53	15	45
g / C, Green / Cycle	0.25	0.25	0.25	0.25	0.22	0.50	0.14	0.42
(v / s)_i Volume / Saturation Flow Rate	0.01	0.02	0.01	0.01	0.13	0.54	0.05	0.25
Total Saturation Flow Adjustment	0.73	0.82	0.86	0.81	0.93	0.92	0.93	0.92
s, saturation flow rate [veh/h]	1382	1564	1630	1547	1770	3506	1770	3508
c, Capacity [veh/h]	339	384	400	380	384	1753	250	1489
d1, Uniform Delay [s]	30.43	30.90	30.49	30.39	37.40	26.50	40.91	23.52
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.18	0.49	0.19	0.13	6.89	44.15	3.34	1.78
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

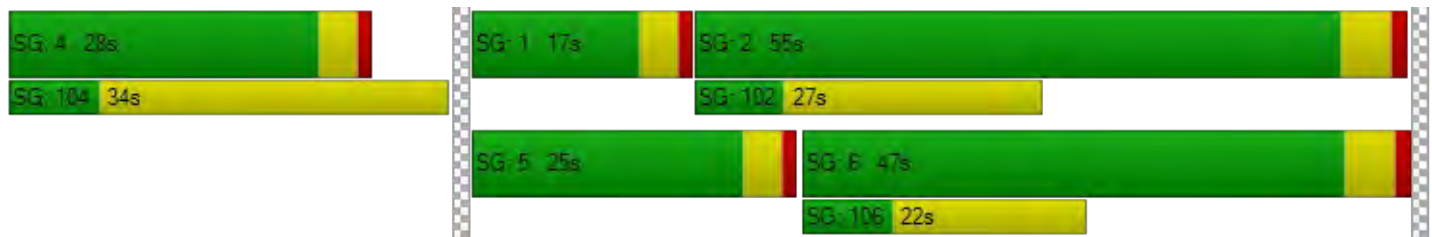
X, volume / capacity	0.03	0.09	0.04	0.03	0.60	1.07	0.32	0.60
d, Delay for Lane Group [s/veh]	30.61	31.38	30.68	30.51	44.29	70.65	44.25	25.29
Lane Group LOS	C	C	C	C	D	E	D	C
Critical Lane Group	no	yes	no	no	no	yes	yes	no
50th-Percentile Queue Length [veh]	0.27	0.89	0.39	0.24	7.11	46.22	2.34	12.15
50th-Percentile Queue Length [ft]	6.66	22.16	9.68	6.03	177.72	1155.50	58.59	303.84
95th-Percentile Queue Length [veh]	0.68	2.16	0.98	0.62	13.09	73.96	5.22	20.51
95th-Percentile Queue Length [ft]	16.98	54.02	24.44	15.40	327.23	1848.91	130.41	512.87

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	30.61	30.61	31.38	30.68	30.68	30.51	44.29	70.65	70.65	44.25	25.29	25.29
Movement LOS	C	C	C	C	C	C	D	E	E	D	C	C
d_A, Approach Delay [s/veh]	31.20			30.61			67.76			26.86		
Approach LOS	C			C			E			C		
d_I, Intersection Delay [s/veh]	54.34											
Intersection LOS	D											
Intersection V/C	0.605											

Sequence

Ring 1	4	2	1	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	5	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#157: Branner Dr/Sand Hill Rd**

Control Type:	Signalized	Delay (sec / veh):	44.3
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.575

Intersection Setup

Name	Branner Drive			Branner Drive			Sand Hill Road			Sand Hill Road		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	+			+			T T T			T T T		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	1	0	0	1	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	160.00	100.00	100.00	155.00	100.00	100.00
Speed [mph]	25.00			25.00			40.00			40.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Branner Drive			Branner Drive			Sand Hill Road			Sand Hill Road		
Base Volume Input [veh/h]	7	1	29	12	1	8	37	1666	12	17	867	21
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	7	1	29	12	1	8	37	1666	12	17	867	21
Peak Hour Factor	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	2	0	8	3	0	2	11	473	3	5	246	6
Total Analysis Volume [veh/h]	8	1	33	14	1	9	42	1893	14	19	985	24
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	1			6			0			11		
Bicycle Volume [bicycles/h]	2			0			32			53		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	100
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	92.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	4	0	5	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lag	-	-
Minimum Green [s]	0	6	0	0	6	0	4	10	0	4	10	0
Maximum Green [s]	0	30	0	0	30	0	30	30	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	4.0	0.0	3.0	4.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	0	34	0	0	34	0	13	54	0	12	53	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	2.0	5.0	0.0	2.0	5.0	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	28	0	0	28	0	0	20	0	0	20	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	yes		no	yes	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	0.0	0.0	20.0	0.0	0.0	20.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	L	C	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	32	32	11	52	10	51
g / C, Green / Cycle	0.32	0.32	0.11	0.51	0.10	0.50
(v / s)_i Volume / Saturation Flow Rate	0.03	0.01	0.02	0.54	0.01	0.29
Total Saturation Flow Adjustment	0.84	0.84	0.93	0.93	0.93	0.93
s, saturation flow rate [veh/h]	1596	1601	1770	3542	1770	3531
c, Capacity [veh/h]	506	507	193	1824	175	1783
d1, Uniform Delay [s]	24.21	23.93	41.07	24.50	41.44	17.33
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.32	0.18	2.58	34.15	1.25	1.31
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.08	0.05	0.22	1.05	0.11	0.57
d, Delay for Lane Group [s/veh]	24.53	24.10	43.66	58.65	42.69	18.63
Lane Group LOS	C	C	D	E	D	B
Critical Lane Group	yes	no	no	yes	yes	no
50th-Percentile Queue Length [veh]	0.90	0.51	1.18	43.40	0.53	11.80
50th-Percentile Queue Length [ft]	22.41	12.63	29.60	1084.98	13.25	295.05
95th-Percentile Queue Length [veh]	2.18	1.27	2.83	69.45	1.32	20.00
95th-Percentile Queue Length [ft]	54.58	31.63	70.71	1736.16	33.11	499.93

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	24.53	24.53	24.53	24.10	24.10	24.10	43.66	58.65	58.65	42.69	18.63	18.63
Movement LOS	C	C	C	C	C	C	D	E	E	D	B	B
d_A, Approach Delay [s/veh]	24.53			24.10			58.33			19.08		
Approach LOS	C			C			E			B		
d_I, Intersection Delay [s/veh]	44.33											
Intersection LOS	D											
Intersection V/C	0.575											

Sequence

Ring 1	4	2	1	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#162: Sharon Park Dr/ Sand Hill Rd**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 29.4
 Level Of Service: C
 Volume to Capacity (v/c): 0.650

Intersection Setup

Name	Sand Hill Road			Sand Hill Road			Sharon Park Drive			Sharon Park Drive		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	↵↵↵			↵↵↵			⊕			↵↵		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	0	0	0	0	0	1
Pocket Length [ft]	250.00	100.00	100.00	150.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	130.00
Speed [mph]	40.00			35.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Sand Hill Road			Sand Hill Road			Sharon Park Drive			Sharon Park Drive		
Base Volume Input [veh/h]	145	1496	17	27	732	136	2	0	9	215	3	174
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	145	1496	17	27	732	136	2	0	9	215	3	174
Peak Hour Factor	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	39	402	5	7	197	37	1	0	2	58	1	47
Total Analysis Volume [veh/h]	156	1609	18	29	787	146	2	0	10	231	3	187
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	15			0			4			3		
Bicycle Volume [bicycles/h]	0			32			20			61		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	100
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	99.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	5	2	0	1	6	0	0	4	0	0	4	0
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lead	-	-	-	-	-	-	-	-
Minimum Green [s]	4	10	0	5	10	0	0	6	0	0	6	0
Maximum Green [s]	30	30	0	30	30	0	0	30	0	0	30	0
Amber [s]	3.0	4.0	0.0	3.0	4.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
All red [s]	1.0	1.0	0.0	1.0	1.0	0.0	0.0	1.0	0.0	0.0	1.0	0.0
Split [s]	22	53	0	12	43	0	0	35	0	0	35	0
Vehicle Extension [s]	3.0	3.0	0.0	3.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	7	0	0	7	0	0	4	0	0	4	0
Pedestrian Clearance [s]	0	19	0	0	19	0	0	27	0	0	27	0
I1, Start-Up Lost Time [s]	2.0	2.0	0.0	2.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	yes		no	yes			no			no	
Pedestrian Recall	no	no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	0.0	0.0	20.0	0.0	0.0	20.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	C	R
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	2.00	2.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	20	51	10	41	33	33	33
g / C, Green / Cycle	0.20	0.51	0.10	0.41	0.33	0.33	0.33
(v / s)_i Volume / Saturation Flow Rate	0.09	0.46	0.02	0.27	0.01	0.17	0.13
Total Saturation Flow Adjustment	0.93	0.93	0.93	0.91	0.83	0.71	0.78
s, saturation flow rate [veh/h]	1770	3541	1770	3446	1584	1343	1491
c, Capacity [veh/h]	354	1806	177	1413	523	443	492
d1, Uniform Delay [s]	35.09	22.21	41.17	23.87	22.62	27.18	25.66
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	3.95	7.73	1.98	2.44	0.08	4.45	2.22
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

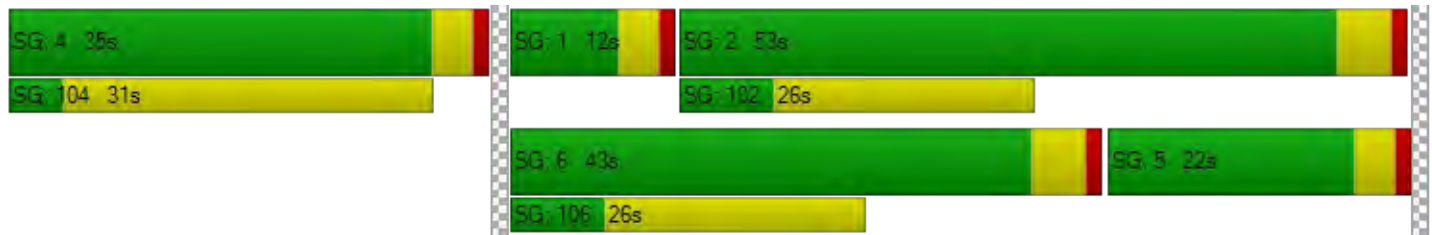
X, volume / capacity	0.44	0.90	0.16	0.66	0.02	0.53	0.38
d, Delay for Lane Group [s/veh]	39.04	29.94	43.16	26.30	22.70	31.63	27.89
Lane Group LOS	D	C	D	C	C	C	C
Critical Lane Group	no	yes	yes	no	no	yes	no
50th-Percentile Queue Length [veh]	4.26	28.34	0.81	12.85	0.24	6.03	4.43
50th-Percentile Queue Length [ft]	106.54	708.51	20.21	321.14	6.08	150.74	110.82
95th-Percentile Queue Length [veh]	8.64	45.44	1.98	21.54	0.62	11.45	8.92
95th-Percentile Queue Length [ft]	215.90	1136.07	49.54	538.43	15.53	286.32	222.98

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	39.04	29.94	29.94	43.16	26.30	26.30	22.70	22.70	22.70	31.63	31.63	27.89
Movement LOS	D	C	C	D	C	C	C	C	C	C	C	C
d_A, Approach Delay [s/veh]	30.74			26.81			22.70			29.97		
Approach LOS	C			C			C			C		
d_I, Intersection Delay [s/veh]	29.42											
Intersection LOS	C											
Intersection V/C	0.650											

Sequence

Ring 1	4	1	2	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	5	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#163: Bayfront Expy/Marsh Rd**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 140.9
 Level Of Service: F
 Volume to Capacity (v/c): 1.173

Intersection Setup

Name	Northbound			Haven Avenue Eastbound			Bayfront Expressway Westbound			Marsh Road Southwestbound		
Approach	Northbound			Eastbound			Westbound			Southwestbound		
Lane Configuration	T T T			T T			T T T T			T		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Northbound			Haven Avenue Eastbound			Bayfront Expressway Westbound			Marsh Road Southwestbound		
Base Volume Input [veh/h]	143	35	1141	5	66	209	2636	407	12	6	35	8
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	7.00	0.00	4.60	0.00	18.20	9.10	4.70	4.90	0.00	0.00	0.00	16.70
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	143	35	1141	5	66	209	2636	407	12	6	35	8
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	38	9	300	1	17	55	694	107	3	2	9	2
Total Analysis Volume [veh/h]	151	37	1201	5	69	220	2775	428	13	6	37	8
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	1			2			0			9		
Bicycle Volume [bicycles/h]	1			14			0			0		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	160
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	125.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Permiss	Overlap	Split	Split	Split	Split	Split	Split	Permiss	Permiss	Permiss
Signal Group	2	3	2	4	1	4	8	2	8	6	4	6
Auxiliary Signal Groups			2,3									
Lead / Lag	Lag	-	-	Lag	-	-	-	-	-	-	-	-
Minimum Green [s]	10	6	10	4	12	4	0	10	0	0	4	0
Maximum Green [s]	50	26	50	30	30	30	0	50	0	0	30	0
Amber [s]	4.7	3.6	4.7	3.6	3.6	3.6	0.0	4.7	0.0	0.0	3.6	0.0
All red [s]	1.0	1.0	1.0	0.5	0.0	0.5	0.0	1.0	0.0	0.0	0.5	0.0
Split [s]	71	20	71	34	35	34	0	71	0	0	34	0
Vehicle Extension [s]	4.5	2.5	4.5	2.8	2.5	2.8	0.0	4.5	0.0	0.0	2.8	0.0
Walk [s]	7	5	7	7	5	7	0	7	0	0	7	0
Pedestrian Clearance [s]	16	10	16	22	26	22	0	16	0	0	22	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	0.0	2.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	3.7	2.6	3.7	2.1	1.6	2.1	0.0	3.7	0.0	0.0	2.1	0.0
Minimum Recall		no	no		no			no			no	
Maximum Recall		no	yes		no			yes			no	
Pedestrian Recall		no	no		no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	0.0	20.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	C	R	L	C	C
L, Total Lost Time per Cycle [s]	4.60	4.60	3.60	3.60	5.70	5.70	4.10
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.60	2.60	1.60	1.60	3.70	3.70	2.10
g_i, Effective Green Time [s]	15	86	31	31	65	65	30
g / C, Green / Cycle	0.10	0.54	0.20	0.20	0.41	0.41	0.19
(v / s)_i Volume / Saturation Flow Rate	0.10	0.44	0.02	0.15	0.55	0.24	0.03
Total Saturation Flow Adjustment	0.96	0.72	0.80	0.76	0.88	0.95	0.97
s, saturation flow rate [veh/h]	1827	2728	3050	1440	5022	1803	1847
c, Capacity [veh/h]	176	1473	599	283	2050	736	345
d1, Uniform Delay [s]	72.30	30.24	52.97	61.00	47.35	37.10	54.40
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	87.49	5.08	0.42	18.82	162.56	3.59	0.90
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	1.07	0.82	0.12	0.78	1.35	0.60	0.15
d, Delay for Lane Group [s/veh]	159.79	35.33	53.39	79.82	209.91	40.69	55.30
Lane Group LOS	F	D	D	E	F	D	E
Critical Lane Group	no	yes	no	yes	yes	no	yes
50th-Percentile Queue Length [veh]	12.65	30.00	1.53	11.25	77.94	17.32	2.04
50th-Percentile Queue Length [ft]	316.22	750.05	38.24	281.35	1948.41	432.90	50.89
95th-Percentile Queue Length [veh]	21.25	48.08	3.57	19.19	124.70	28.25	4.61
95th-Percentile Queue Length [ft]	531.15	1201.94	89.35	479.79	3117.46	706.20	115.29

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	159.79	159.79	35.33	53.39	53.39	79.82	209.91	40.69	40.69	55.30	55.30	55.30
Movement LOS	F	F	D	D	D	E	F	D	D	E	E	E
d_A, Approach Delay [s/veh]	52.18			73.16			186.71			55.30		
Approach LOS	D			E			F			E		
d_I, Intersection Delay [s/veh]	140.86											
Intersection LOS	F											
Intersection V/C	1.173											

Sequence

Ring 1	1	2	3	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#181: Santa Cruz Ave/Elder Ave**

Control Type:	Signalized	Delay (sec / veh):	15.4
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.545

Intersection Setup

Name	Santa Cruz Avenue		Santa Cruz Avenue		Elder Ave	
Approach	Northeastbound		Southwestbound		Southeastbound	
Lane Configuration						
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	0	0	1
Pocket Length [ft]	60.00	100.00	100.00	100.00	100.00	105.00
Speed [mph]	25.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Santa Cruz Avenue		Santa Cruz Avenue		Elder Ave	
Base Volume Input [veh/h]	64	636	403	112	100	74
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	1.90	3.20	5.40	1.00	1.40
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	64	636	403	112	100	74
Peak Hour Factor	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	18	181	114	32	28	21
Total Analysis Volume [veh/h]	73	723	458	127	114	84
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		20		20	
Bicycle Volume [bicycles/h]	24		13		11	

Intersection Settings

Located in CBD	yes
Signal Coordination Group	-
Cycle Length [s]	100
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	8.00

Phasing & Timing

Control Type	Protected	Permissive	Permissive	Permissive	Split	Split
Signal Group	5	2	6	0	3	0
Auxiliary Signal Groups						
Lead / Lag	Lead	-	-	-	Lead	-
Minimum Green [s]	5	5	5	0	5	0
Maximum Green [s]	30	30	30	0	30	0
Amber [s]	3.0	3.0	3.0	0.0	3.0	0.0
All red [s]	1.0	1.0	1.0	0.0	1.0	0.0
Split [s]	13	83	70	0	17	0
Vehicle Extension [s]	3.0	3.0	3.0	0.0	3.0	0.0
Walk [s]	0	0	1	0	1	0
Pedestrian Clearance [s]	0	0	1	0	1	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	0.0	2.0	0.0
Minimum Recall	no	no	no		no	
Maximum Recall	no	yes	yes		no	
Pedestrian Recall	no	no	no		no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	C	L	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	9	79	66	13	13
g / C, Green / Cycle	0.09	0.79	0.66	0.13	0.13
(v / s)_i Volume / Saturation Flow Rate	0.04	0.43	0.36	0.07	0.06
Total Saturation Flow Adjustment	0.86	0.88	0.85	0.85	0.75
s, saturation flow rate [veh/h]	1625	1678	1608	1608	1433
c, Capacity [veh/h]	146	1326	1062	209	186
d1, Uniform Delay [s]	43.35	3.87	9.08	40.73	40.20
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	11.67	1.62	2.06	9.85	7.69
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.50	0.55	0.55	0.55	0.45
d, Delay for Lane Group [s/veh]	55.03	5.49	11.14	50.58	47.89
Lane Group LOS	E	A	B	D	D
Critical Lane Group	no	yes	no	yes	no
50th-Percentile Queue Length [veh]	2.24	9.17	10.22	3.44	2.46
50th-Percentile Queue Length [ft]	56.02	229.15	255.44	85.98	61.51
95th-Percentile Queue Length [veh]	5.02	16.13	17.67	7.23	5.44
95th-Percentile Queue Length [ft]	125.42	403.28	441.80	180.79	136.02

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	55.03	5.49	11.14	11.14	50.58	47.89
Movement LOS	E	A	B	B	D	D
d_A, Approach Delay [s/veh]	10.03		11.14		49.44	
Approach LOS	B		B		D	
d_I, Intersection Delay [s/veh]	15.39					
Intersection LOS	B					
Intersection V/C	0.545					

Sequence

Ring 1	-	2	3	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#195: Bayfront Expy/Chilco St**

Control Type:	Signalized	Delay (sec / veh):	18.9
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.843

Intersection Setup

Name	Chilco Street		Bayfront Expy		Bayfront Expy	
Approach	Northbound		Westbound		Southeastbound	
Lane Configuration	⇐ ⇨		⇨ ⇨ ⇨ ⇨		⇨ ⇨ ⇨	
Turning Movement	Left	Right	Left	Thru	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	1	0	0	1
Pocket Length [ft]	80.00	100.00	520.00	100.00	100.00	660.00
Speed [mph]	30.00		45.00		50.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Chilco Street		Bayfront Expy		Bayfront Expy	
Base Volume Input [veh/h]	205	26	176	2726	933	169
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	3.40	23.10	6.30	3.80	5.10	5.30
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	205	26	176	2726	933	169
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	55	7	47	725	248	45
Total Analysis Volume [veh/h]	218	28	187	2900	993	180
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		2		2	
Bicycle Volume [bicycles/h]	0		0		0	

Intersection Settings

Located in CBD	yes
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	9.00

Phasing & Timing

Control Type	Split	Split	Protected	Permissive	Permissive	Permissive
Signal Group	5	0	7	4	8	0
Auxiliary Signal Groups						
Lead / Lag	Lead	-	Lead	-	-	-
Minimum Green [s]	5	0	5	5	5	0
Maximum Green [s]	30	0	30	30	30	0
Amber [s]	3.0	0.0	3.0	3.0	3.0	0.0
All red [s]	1.0	0.0	1.0	1.0	1.0	0.0
Split [s]	26	0	26	104	78	0
Vehicle Extension [s]	3.0	0.0	3.0	3.0	3.0	0.0
Walk [s]	1	0	0	1	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	0.0	2.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	2.0	0.0	2.0	2.0	2.0	0.0
Minimum Recall	no		no	no	no	
Maximum Recall	no		no	yes	yes	
Pedestrian Recall	no		no	no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	0.0	0.0	20.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	R	L	C	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	22	22	22	100	74	74
g / C, Green / Cycle	0.17	0.17	0.17	0.77	0.57	0.57
(v / s)_i Volume / Saturation Flow Rate	0.14	0.02	0.12	0.65	0.22	0.13
Total Saturation Flow Adjustment	0.83	0.62	0.80	0.79	0.78	0.73
s, saturation flow rate [veh/h]	1571	1181	1528	4488	4432	1380
c, Capacity [veh/h]	266	200	259	3452	2523	786
d1, Uniform Delay [s]	52.09	45.95	51.12	9.78	15.54	13.87
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	23.82	1.46	16.09	2.64	0.46	0.68
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.82	0.14	0.72	0.84	0.39	0.23
d, Delay for Lane Group [s/veh]	75.91	47.41	67.21	12.43	16.01	14.55
Lane Group LOS	E	D	E	B	B	B
Critical Lane Group	yes	no	no	yes	no	no
50th-Percentile Queue Length [veh]	9.59	0.94	7.70	32.22	8.21	3.59
50th-Percentile Queue Length [ft]	239.74	23.45	192.46	805.62	205.13	89.72
95th-Percentile Queue Length [veh]	16.75	2.28	13.97	51.61	14.72	7.49
95th-Percentile Queue Length [ft]	418.81	56.97	349.21	1290.27	367.96	187.33

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	75.91	47.41	67.21	12.43	16.01	14.55
Movement LOS	E	D	E	B	B	B
d_A, Approach Delay [s/veh]	72.67		15.75		15.78	
Approach LOS	E		B		B	
d_I, Intersection Delay [s/veh]	18.86					
Intersection LOS	B					
Intersection V/C	0.843					

Sequence



Ring 1	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	7	8	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#196: Bayfront Expy/Chrysler Drive**

Control Type:	Signalized	Delay (sec / veh):	15.1
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.883

Intersection Setup

Name	Chrysler Drive		Bayfront Expy		Bayfront Expy	
Approach	Northbound		Eastbound		Westbound	
Lane Configuration						
Turning Movement	Left	Right	Thru	Right	Left	Thru
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	1	0
Pocket Length [ft]	140.00	100.00	100.00	290.00	345.00	100.00
Speed [mph]	25.00		45.00		45.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Chrysler Drive		Bayfront Expy		Bayfront Expy	
Base Volume Input [veh/h]	179	12	1106	56	22	2914
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	12.30	8.30	5.30	7.10	0.00	3.70
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	179	12	1106	56	22	2914
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	48	3	294	15	6	775
Total Analysis Volume [veh/h]	190	13	1177	60	23	3100
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		0		4	
Bicycle Volume [bicycles/h]	1		1		1	

Intersection Settings

Located in CBD	yes
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	9.00

Phasing & Timing

Control Type	Split	Split	Permissive	Permissive	Protected	Permissive
Signal Group	5	0	8	0	7	4
Auxiliary Signal Groups						
Lead / Lag	Lead	-	-	-	Lead	-
Minimum Green [s]	5	0	5	0	5	5
Maximum Green [s]	30	0	30	0	30	30
Amber [s]	3.0	0.0	3.0	0.0	3.0	3.0
All red [s]	1.0	0.0	1.0	0.0	1.0	1.0
Split [s]	24	0	97	0	9	106
Vehicle Extension [s]	3.0	0.0	3.0	0.0	3.0	3.0
Walk [s]	1	0	1	0	0	1
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	0.0	2.0	0.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	0.0	2.0	0.0	2.0	2.0
Minimum Recall	no		no		no	no
Maximum Recall	no		yes		no	yes
Pedestrian Recall	no		no		no	no
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	0.0	20.0	0.0	0.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	R	C	R	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	20	20	93	93	5	102
g / C, Green / Cycle	0.15	0.15	0.72	0.72	0.04	0.78
(v / s)_i Volume / Saturation Flow Rate	0.13	0.01	0.27	0.04	0.01	0.69
Total Saturation Flow Adjustment	0.76	0.71	0.78	0.71	0.86	0.79
s, saturation flow rate [veh/h]	1447	1342	4424	1357	1625	4492
c, Capacity [veh/h]	223	206	3165	971	62	3524
d1, Uniform Delay [s]	53.58	46.99	7.17	5.51	60.96	9.73
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	31.82	0.58	0.34	0.12	15.89	3.50
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.85	0.06	0.37	0.06	0.37	0.88
d, Delay for Lane Group [s/veh]	85.40	47.58	7.51	5.63	76.85	13.24
Lane Group LOS	F	D	A	A	E	B
Critical Lane Group	yes	no	no	no	no	yes
50th-Percentile Queue Length [veh]	8.70	0.43	7.01	0.74	0.93	37.44
50th-Percentile Queue Length [ft]	217.46	10.85	175.31	18.51	23.27	936.10
95th-Percentile Queue Length [veh]	15.44	1.09	12.94	1.82	2.26	59.93
95th-Percentile Queue Length [ft]	386.11	27.31	323.62	45.58	56.54	1498.28

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	85.40	47.58	7.51	5.63	76.85	13.24
Movement LOS	F	D	A	A	E	B
d_A, Approach Delay [s/veh]	82.98		7.42		13.70	
Approach LOS	F		A		B	
d_I, Intersection Delay [s/veh]	15.08					
Intersection LOS	B					
Intersection V/C	0.883					

Sequence

Ring 1	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	7	8	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#207: Chilco St/Constitution Dr**

Control Type: All-way stop
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 11.6
 Level Of Service: B

Intersection Setup

Name	Chilco Street			Chilco Street			Constitution Drive			Constitution Drive		
Approach	Northbound			Southbound			Eastbound			Northwestbound		
Lane Configuration	⤵			⤵			⤵			⤵		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Chilco Street			Chilco Street			Constitution Drive			Constitution Drive		
Base Volume Input [veh/h]	114	148	74	133	204	8	5	5	17	14	8	28
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	4.40	10.10	6.80	2.30	3.40	0.00	40.00	40.00	29.40	14.30	37.50	7.10
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	114	148	74	133	204	8	5	5	17	14	8	28
Peak Hour Factor	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	31	40	20	36	55	2	1	1	5	4	2	8
Total Analysis Volume [veh/h]	124	161	80	145	222	9	5	5	18	15	9	30
Pedestrian Volume [ped/h]	0			1			0			1		

Intersection Settings

Lanes

Movement, Approach, & Intersection Results

95th-Percentile Queue Length [veh]	2.54	2.71	0.14	0.28
95th-Percentile Queue Length [ft]	63.46	67.76	3.62	6.88
Approach Delay [s/veh]	11.69	12.04	9.22	9.14
Approach LOS	B	B	A	A
Intersection Delay [s/veh]	11.60			
Intersection LOS	B			

**Intersection Level Of Service Report
#209: Jefferson Dr/Constitution Dr**

Control Type: Two-way stop
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 9.6
 Level Of Service: A
 Volume to Capacity (v/c): 0.000

Intersection Setup

Name	Southbound			Eastbound			Westbound			Northeastbound		
Approach	Southbound			Eastbound			Westbound			Northeastbound		
Lane Configuration	↑			↑			↑			Y		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	25.00			30.00			30.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Southbound			Eastbound			Westbound			Northeastbound		
Base Volume Input [veh/h]	1	0	0	1	53	0	2	17	1	1	0	11
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	1.90	0.00	0.00	5.90	0.00	0.00	0.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	1	0	0	1	53	0	2	17	1	1	0	11
Peak Hour Factor	0.7500	0.7500	0.7500	0.7500	0.7500	0.7500	0.7500	0.7500	0.7500	0.7500	0.7500	0.7500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	0	0	0	18	0	1	6	0	0	0	4
Total Analysis Volume [veh/h]	1	0	0	1	71	0	3	23	1	1	0	15
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings

Priority Scheme	Stop	Free	Free	Stop
Flared Lane	no			no
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance	no			no
Number of Storage Spaces in Median	0	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.02
d_M, Delay for Movement [s/veh]	9.20	9.57	8.40	7.25	0.00	0.00	7.34	0.00	0.00	9.15	9.62	8.67
Movement LOS	A	A	A	A	A	A	A	A	A	A	A	A
95th-Percentile Queue Length [veh]	0.00	0.00	0.00	0.14	0.14	0.14	0.05	0.05	0.05	0.05	0.05	0.05
95th-Percentile Queue Length [ft]	0.09	0.09	0.09	3.52	3.52	3.52	1.34	1.34	1.34	1.23	1.23	1.23
d_A, Approach Delay [s/veh]	9.20			0.10			0.82			8.70		
Approach LOS	A			A			A			A		
d_I, Intersection Delay [s/veh]	1.53											
Intersection LOS	A											

**Intersection Level Of Service Report
#213: Chrysler Dr/Independence Dr**

Control Type: Two-way stop
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 10.1
 Level Of Service: B
 Volume to Capacity (v/c): 0.016

Intersection Setup

Name	Chrysler Drive			Chrysler Drive			Independence Drive					
Approach	Northbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	Y			T			T			T		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Chrysler Drive			Chrysler Drive			Independence Drive					
Base Volume Input [veh/h]	8	33	5	1	6	0	29	11	91	0	0	1
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	9.10	20.00	100.00	33.30	0.00	10.30	0.00	0.00	0.00	0.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	8	33	5	1	6	0	29	11	91	0	0	1
Peak Hour Factor	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400	0.8400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	2	10	1	0	2	0	9	3	27	0	0	0
Total Analysis Volume [veh/h]	10	39	6	1	7	0	35	13	108	0	0	1
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings

Priority Scheme	Free	Free	Stop	Stop
Flared Lane			no	no
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance			no	no
Number of Storage Spaces in Median	0	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.01	0.00	0.00	0.00	0.00	0.00	0.04	0.02	0.10	0.00	0.00	0.00
d_M, Delay for Movement [s/veh]	7.23	0.00	0.00	8.24	0.00	0.00	9.72	10.10	9.18	9.84	9.42	8.33
Movement LOS	A	A	A	A	A	A	A	B	A	A	A	A
95th-Percentile Queue Length [veh]	0.10	0.10	0.10	0.02	0.02	0.02	0.57	0.57	0.57	0.00	0.00	0.00
95th-Percentile Queue Length [ft]	2.62	2.62	2.62	0.54	0.54	0.54	14.15	14.15	14.15	0.07	0.07	0.07
d_A, Approach Delay [s/veh]	1.31			1.03			9.38			8.33		
Approach LOS	A			A			A			A		
d_I, Intersection Delay [s/veh]	7.05											
Intersection LOS	B											

**Intersection Level Of Service Report
#214: Chrysler Dr/Jefferson Dr**

Control Type: Two-way stop
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 12.2
 Level Of Service: B
 Volume to Capacity (v/c): 0.000

Intersection Setup

Name	Chrysler Drive			Chrysler Drive			Jefferson Drive					
Approach	Southbound			Northeastbound			Northwestbound			Southeastbound		
Lane Configuration	Y			T			+					
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			25.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Chrysler Drive			Chrysler Drive			Jefferson Drive					
Base Volume Input [veh/h]	111	39	0	0	23	14	7	0	11	0	0	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.70	10.30	2.00	2.00	26.10	7.10	0.00	0.00	18.20	2.00	2.00	2.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	111	39	0	0	23	14	7	0	11	0	0	0
Peak Hour Factor	0.7800	0.7800	1.0000	1.0000	0.7800	0.7800	0.7800	0.7800	0.7800	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	36	13	0	0	7	4	2	0	4	0	0	0
Total Analysis Volume [veh/h]	142	50	0	0	29	18	9	0	14	0	0	0
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings

Priority Scheme	Free	Free	Stop	Stop
Flared Lane			no	
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance			no	
Number of Storage Spaces in Median	0	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.09	0.00	0.00	0.00	0.00	0.00	0.02	0.00	0.01	0.00	0.00	0.00
d_M, Delay for Movement [s/veh]	7.55	0.00	0.00	0.00	0.00	0.00	11.74	12.23	8.79	0.00	0.00	0.00
Movement LOS	A	A			A	A	B	B	A			
95th-Percentile Queue Length [veh]	0.42	0.42	0.00	0.00	0.00	0.00	0.09	0.09	0.09	0.00	0.00	0.00
95th-Percentile Queue Length [ft]	10.53	10.53	0.00	0.00	0.00	0.00	2.37	2.37	2.37	0.00	0.00	0.00
d_A, Approach Delay [s/veh]	5.58			0.00			9.95			0.00		
Approach LOS	A			A			A			A		
d_I, Intersection Delay [s/veh]	4.96											
Intersection LOS	B											

**Intersection Level Of Service Report
#215: Chrysler Dr/Constitution Dr**

Control Type: All-way stop
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 8.8
 Level Of Service: A

Intersection Setup

Name	Chrysler Drive						Constitution Drive					
Approach	Southbound			Eastbound			Westbound			Northeastbound		
Lane Configuration	↕			↕			↕			↕		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Chrysler Drive						Constitution Drive					
Base Volume Input [veh/h]	23	47	12	19	127	97	19	5	102	1	28	2
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	13.00	8.50	8.30	21.10	0.80	3.10	5.30	40.00	9.80	0.00	17.90	100.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	23	47	12	19	127	97	19	5	102	1	28	2
Peak Hour Factor	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	6	13	3	5	35	27	5	1	28	0	8	1
Total Analysis Volume [veh/h]	26	52	13	21	141	108	21	6	113	1	31	2
Pedestrian Volume [ped/h]	0			1			1			1		

Intersection Settings

Lanes

Movement, Approach, & Intersection Results

95th-Percentile Queue Length [veh]	0.43	1.36	0.59	0.16
95th-Percentile Queue Length [ft]	10.76	34.03	14.72	3.93
Approach Delay [s/veh]	8.70	9.15	8.08	8.56
Approach LOS	A	A	A	A
Intersection Delay [s/veh]	8.76			
Intersection LOS	A			

**Intersection Level Of Service Report
#233: Sand Hill Road and Sand Hill Circle**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 63.7
 Level Of Service: E
 Volume to Capacity (v/c): 0.435

Intersection Setup

Name	Hwy 280 NB Off-Ramp			Sand Hill Road			Sand Hill Road			Westbound		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	↵			↵↶						↵↶		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Hwy 280 NB Off-Ramp			Sand Hill Road			Sand Hill Road			Westbound		
Base Volume Input [veh/h]	4	293	0	0	51	40	0	0	0	0	795	32
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	26.60	2.00	2.00	5.90	7.50	2.00	2.00	2.00	3.00	3.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	4	293	0	0	51	40	0	0	0	0	795	32
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	1	73	0	0	13	10	0	0	0	0	199	8
Total Analysis Volume [veh/h]	4	293	0	0	51	40	0	0	0	0	795	32
Presence of On-Street Parking	no					no				no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	1			0			0			0		
Bicycle Volume [bicycles/h]	0			1			0			1		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fixed time
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Protecte	Permiss	Permiss	Permiss	Permiss
Signal Group	0	8	0	0	8	0	0	0	0	0	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	-	-	-	-	-	-
Minimum Green [s]	0	6	0	0	6	0	0	0	0	0	8	0
Maximum Green [s]	0	13	0	0	13	0	0	0	0	0	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	0.0	0.0	0.0	3.0	0.0
All red [s]	0.0	0.5	0.0	0.0	0.5	0.0	0.0	0.0	0.0	0.0	0.5	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	2.5	0.0	0.0	2.5	0.0	0.0	0.0	0.0	0.0	3.0	0.0
Walk [s]	0	10	0	0	10	0	0	0	0	0	7	0
Pedestrian Clearance [s]	0	10	0	0	10	0	0	0	0	0	14	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	0.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no			no						yes	
Maximum Recall		no			no						no	
Pedestrian Recall		no			no						no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	0.0	0.0	20.0	0.0	0.0	0.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	R		C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00		2.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	0.00		0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00		0.00
g_i, Effective Green Time [s]	15	15	15		32
g / C, Green / Cycle	0.16	0.16	0.16		0.35
(v / s)_i Volume / Saturation Flow Rate	0.20	0.03	0.03		0.24
Total Saturation Flow Adjustment	0.79	0.94	0.78		0.92
s, saturation flow rate [veh/h]	1498	1794	1485		3490
c, Capacity [veh/h]	241	289	239		1222
d1, Uniform Delay [s]	37.75	32.59	32.54		24.92
k, delay calibration	0.50	0.50	0.50		0.50
l, Upstream Filtering Factor	1.00	1.00	1.00		1.00
d2, Incremental Delay [s]	134.53	1.33	1.50		3.03
d3, Initial Queue Delay [s]	0.00	0.00	0.00		0.00
Rp, platoon ratio	1.00	1.00	1.00		1.00
PF, progression factor	1.00	1.00	1.00		1.00

Lane Group Results

X, volume / capacity	1.23	0.18	0.17		0.68
d, Delay for Lane Group [s/veh]	172.28	33.92	34.05		27.94
Lane Group LOS	F	C	C		C
Critical Lane Group	yes	no	no		yes
50th-Percentile Queue Length [veh]	16.18	1.20	0.95		10.90
50th-Percentile Queue Length [ft]	404.41	30.08	23.65		272.49
95th-Percentile Queue Length [veh]	26.52	2.87	2.30		18.67
95th-Percentile Queue Length [ft]	662.96	71.77	57.42		466.79

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	172.28	172.28	0.00	0.00	33.92	34.05	0.00	0.00	0.00	27.94	27.94	27.94
Movement LOS	F	F			C	C				C	C	C
d_A, Approach Delay [s/veh]	172.28			33.98			0.00			27.94		
Approach LOS	F			C			A			C		
d_I, Intersection Delay [s/veh]	63.68											
Intersection LOS	E											
Intersection V/C	0.435											

Sequence

Ring 1	2	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#234: Sand Hill Rd/Hwy 280 NB Off-Ramp**

Control Type:	Signalized	Delay (sec / veh):	233.2
Analysis Method:	HCM2000	Level Of Service:	F
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.731

Intersection Setup

Name	Hwy 280 NB Off-Ramp			Hwy 280 NB Off-Ramp			Sand Hill Road			Sand Hill Road		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	T T			L			T T T					
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	1	0	0	0	1	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	300.00	100.00	100.00	100.00	270.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	40.00			40.00			40.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Hwy 280 NB Off-Ramp			Hwy 280 NB Off-Ramp			Sand Hill Road			Sand Hill Road		
Base Volume Input [veh/h]	0	112	478	48	0	0	156	1696	0	0	0	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.70	3.60	4.20	2.00	2.00	2.60	1.50	2.00	2.00	2.00	2.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	112	478	48	0	0	156	1696	0	0	0	0
Peak Hour Factor	1.0000	0.9500	0.9500	0.9500	1.0000	1.0000	0.9500	0.9500	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	29	126	13	0	0	41	446	0	0	0	0
Total Analysis Volume [veh/h]	0	118	503	51	0	0	164	1785	0	0	0	0
Presence of On-Street Parking			no	no			no					
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			40			0		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fixed time
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Split	Split	Split	Protecte	Permiss	Protecte	Permiss	Permiss	Protecte	Protecte	Permiss
Signal Group	0	8	0	7	0	0	1	6	0	0	0	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	Lag	-	-	Lead	-	-	-	-	-
Minimum Green [s]	0	6	0	6	0	0	4	8	0	0	0	0
Maximum Green [s]	0	13	0	25	0	0	16	30	0	0	0	0
Amber [s]	0.0	3.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	0.0	0.0
All red [s]	0.0	1.0	0.0	1.0	0.0	0.0	1.0	1.0	0.0	0.0	0.0	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	3.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	0.0	0.0
Walk [s]	0	5	0	5	0	0	0	5	0	0	0	0
Pedestrian Clearance [s]	0	10	0	10	0	0	0	10	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	0.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	0.0	0.0
Minimum Recall		no		no			no	yes				
Maximum Recall		no		no			no	no				
Pedestrian Recall		no		no			no	no				
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	0.0	20.0	0.0	0.0	20.0	20.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	L	L	C	
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	
g_i, Effective Green Time [s]	13	25	30	30	
g / C, Green / Cycle	0.14	0.28	0.33	0.33	
(v / s)_i Volume / Saturation Flow Rate	0.20	0.03	0.09	0.50	
Total Saturation Flow Adjustment	0.81	0.91	0.93	0.94	
s, saturation flow rate [veh/h]	3095	1732	1759	3564	
c, Capacity [veh/h]	447	481	586	1188	
d1, Uniform Delay [s]	38.50	24.18	22.06	30.00	
k, delay calibration	0.50	0.50	0.50	0.50	
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	
d2, Incremental Delay [s]	188.54	0.44	1.19	230.55	
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	
Rp, platoon ratio	1.00	1.00	1.00	1.00	
PF, progression factor	1.00	1.00	1.00	1.00	

Lane Group Results

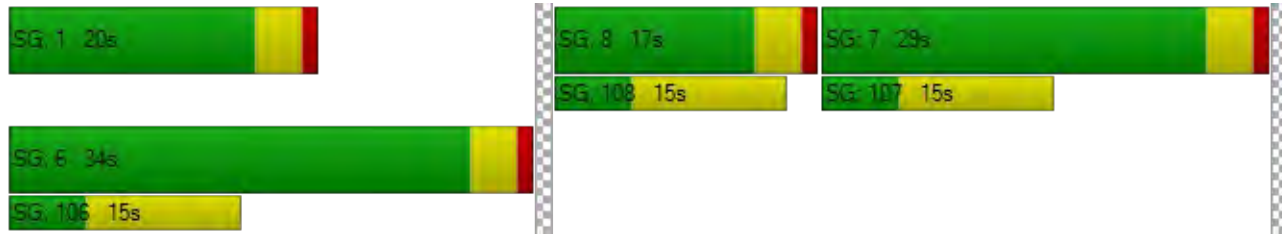
X, volume / capacity	1.39	0.11	0.28	1.50	
d, Delay for Lane Group [s/veh]	227.04	24.63	23.24	260.55	
Lane Group LOS	F	C	C	F	
Critical Lane Group	yes	yes	no	yes	
50th-Percentile Queue Length [veh]	20.90	1.03	3.32	64.95	
50th-Percentile Queue Length [ft]	522.57	25.74	82.94	1623.66	
95th-Percentile Queue Length [veh]	33.76	2.49	7.02	103.91	
95th-Percentile Queue Length [ft]	844.09	62.14	175.43	2597.85	

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	0.00	227.04	227.04	24.63	0.00	0.00	23.24	260.55	0.00	0.00	0.00	0.00
Movement LOS		F	F	C			C	F				
d_A, Approach Delay [s/veh]	227.04			24.63			240.58			0.00		
Approach LOS	F			C			F			A		
d_I, Intersection Delay [s/veh]	233.17											
Intersection LOS	F											
Intersection V/C	0.731											

Sequence

Ring 1	1	-	7	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Menlo Park GP Circulation Update

Vistro File: J:\...\Menlo Park AM_update.vistro
Report File: J:\...\Menlo Park_AM Results.pdf

Scenario 1: Existing AM
1/9/2015

Turning Movement Volume: Summary

ID	Intersection Name	Northeastbound		Southwestbound		Southeastbound		Total Volume
		Thru	Right	Left	Thru	Left	Right	
1	Marsh Rd (SR 84)/US 101 SB Offramp	784		1747		953	320	3804

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
2	Marsh Rd/Rolison Rd-Scott Dr	28	1184	14	439	1422	296	15	4	71	239	12	5	3729

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
3	Marsh Rd/Florence St-Bohannon Dr	99	767	90	27	1068	435	447	47	198	27	16	25	3246

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4	Marsh Rd/Bay Rd	1	803	82	268	867	31	114	78	7	45	19	173	2488

ID	Intersection Name	Northeastbound		Northwestbound		Southeastbound		Total Volume
		Left	Right	Left	Thru	Thru	Right	
9	Middlefield Rd/Ravenswood Ave	85	522	446	364	420	75	1912

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
10	Middlefield Rd/Ringswood Ave	5	3	12	136	44	251	55	559	84	216	653	62	2080

ID	Intersection Name	Northeastbound		Southwestbound		Northwestbound		Total Volume
		Thru	Right	Left	Thru	Left	Right	
15	Bayfront Expy (SR 84)/University Ave (SR 109)	873	85	1495	3915	185	359	6912

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
16	Bayfront Expy (SR 84)/Willow Rd (SR 114)	128	380	361	16	59	42	217	713	155	732	2780	24	5607

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
17	Willow Rd (SR 114)/Hamilton Ave	124	807	71	88	795	57	78	22	50	26	24	23	2165

ID	Intersection Name	Northbound		Southbound		Eastbound		Total Volume
		Left	Thru	Thru	Right	Left	Right	
18	Willow Rd (SR 114)/Ivy Dr	129	1141	842	9	17	147	2285

ID	Intersection Name	Northbound		Southbound		Westbound		Total Volume
		Thru	Right	Left	Thru	Left	Right	
19	Willow Rd (SR 114)/O'Brien Dr	1201	304	54	969	195	58	2781

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
20	Willow Rd (SR 114)/Newbridge St	151	1407	166	51	1132	10	37	169	281	273	120	42	3839

ID	Intersection Name	Northeastbound		Southwestbound		Southeastbound		Total Volume
		Left	Thru	Thru	Right	Left	Right	
21	Willow Rd/Bay Rd	67	1278	1161	445	363	60	3374

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
22	Willow Rd/Durham St-VA Med Entrance	26	1040	16	36	925	89	40	3	10	48	12	70	2315

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
23	Willow Rd/Coleman Ave	22	864	3	1	805	120	191	6	51	4	4	5	2076

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
24	Willow Rd/Gilbert Ave	3	745	40	27	826	4	37	70	17	78	112	100	2059

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
25	Middlefield Rd-Willow Rd	28	201	139	458	75	391	80	368	256	320	356	15	2687

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
26	Ravenswood Ave/Laurel St	15	618	67	20	471	11	138	113	13	136	188	65	1855

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
28	Oak Grove Ave/Laurel St	11	258	85	30	345	61	30	107	16	72	200	52	1267

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
29	El Camino Real (SR 82)/Encinal Ave-Menlo College Entrance	8	57	9	156	14	149	16	840	113	169	1838	71	3440

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
30	El Camino Real (SR 82)/Glenwood Ave-Valparaiso Ave	255	126	124	84	177	16	101	700	35	54	1349	547	3568

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
31	El Camino Real (SR 82)/Oak Grove Ave	57	196	53	117	254	63	89	731	73	125	1365	85	3208

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound		Southeastbound		Total Volume
		Left	Thru	Right	Left	Thru	Right	Thru	Right	Thru	Right	
32	El Camino Real (SR 82)/Santa Cruz Ave	78	54	76	41	44	30	818	39	1420	16	2616

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
33	El Camino Real (SR 82)/Ravenswood Ave-Menlo Ave	24	345	60	417	214	44	88	779	396	161	1420	16	3964

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
34	El Camino Real (SR 82)/Roble Ave	65	8	68	6	3	10	43	1188	25	43	1755	43	3257

ID	Intersection Name	Northeastbound		Northwestbound		Southeastbound		Total Volume
		Left	Right	Left	Thru	Thru	Right	
35	El Camino Real (SR 82)/Middle Ave	185	285	182	1040	1753	68	3513

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
36	El Camino Real (SR 82)/Cambridge Ave	28	0	50	1	0	0	119	1204	1	30	2068	15	3516

ID	Intersection Name	Northeastbound		Southwestbound		Northwestbound		Total Volume
		Thru	Right	Left	Thru	Left	Right	
38	Santa Cruz Ave/University Dr (S)	397	386	59	340	302	67	1551

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
39	Santa Cruz Ave/Sand Hill Rd	231	1143	261	331	559	51	94	615	389	223	669	236	4802

ID	Intersection Name	Northbound		Southbound		Eastbound			Total Volume
		Left	Thru	Thru	Right	Left	Thru	Right	
58	University Avenue and Adams Drive	42	508	1449	140	6	0	1	2146

ID	Intersection Name	Northbound		Southbound		Eastbound		Total Volume
		Left	Thru	Thru	Right	Left	Right	
74	University Ave/O'Brien Dr	67	532	1284	142	16	25	2066

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
88	Valparaiso Ave/ University Dr	152	429	122	42	550	59	56	85	33	71	51	62	1712

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
103	Addison Wesley/Sand Hill Rd	23	2	41	2	2	7	103	1888	233	113	852	39	3305

ID	Intersection Name	Southbound		Westbound		Northeastbound		Total Volume
		Left	Thru	Left	Right	Thru	Right	
107	Alpine Rd/Santa Cruz Ave&Junipero Serra Blvd	663	556	65	172	852	517	2825

ID	Intersection Name	Northbound		Southbound		Northwestbound		Total Volume
		Thru		Thru		Left	Right	
110	Marsh Road and US 101 NB Ramps	1097		1261		848	349	3555

ID	Intersection Name	Northeastbound		Southwestbound		Southeastbound		Total Volume
		Thru		Thru	Right	Left	Right	
132	Oak Ave/Sand Hill Rd	1794		740	38	77	189	2838

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
156	Saga Ln/Sand Hill Rd	10	0	33	14	1	9	211	1594	117	73	760	50	2872

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
157	Branner Dr/Sand Hill Rd	7	1	29	12	1	8	37	1666	12	17	867	21	2678

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
162	Sharon Park Dr/ Sand Hill Rd	145	1496	17	27	732	136	2	0	9	215	3	174	2956

ID	Intersection Name	Northbound			Eastbound			Westbound			Southwestbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
163	Bayfront Expy/Marsh Rd	143	35	1141	5	66	209	2636	407	12	6	35	8	4703

ID	Intersection Name	Northeastbound		Southwestbound		Southeastbound		Total Volume
		Left	Thru	Thru	Right	Left	Right	
181	Santa Cruz Ave/Elder Ave	64	636	403	112	100	74	1389

ID	Intersection Name	Northbound		Westbound		Southeastbound		Total Volume
		Left	Right	Left	Thru	Thru	Right	
195	Bayfront Expy/Chilco St	205	26	176	2726	933	169	4235

ID	Intersection Name	Northbound		Eastbound		Westbound		Total Volume
		Left	Right	Thru	Right	Left	Thru	
196	Bayfront Expy/Chrysler Drive	179	12	1106	56	22	2914	4289

ID	Intersection Name	Northbound			Southbound			Eastbound			Northwestbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
207	Chilco St/Constitution Dr	114	148	74	133	204	8	5	5	17	14	8	28	758

ID	Intersection Name	Southbound			Eastbound			Westbound			Northeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
209	Jefferson Dr/Constitution Dr	1	0	0	1	53	0	2	17	1	1	0	11	87

ID	Intersection Name	Northbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
213	Chrysler Dr/Independence Dr	8	33	5	1	6	0	29	11	91	0	0	1	185

ID	Intersection Name	Southbound		Northeastbound		Northwestbound			Total Volume
		Left	Thru	Thru	Right	Left	Thru	Right	
214	Chrysler Dr/Jefferson Dr	111	39	23	14	7	0	11	205

ID	Intersection Name	Southbound			Eastbound			Westbound			Northeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
215	Chrysler Dr/Constitution Dr	23	47	12	19	127	97	19	5	102	1	28	2	482

ID	Intersection Name	Northbound		Southbound		Westbound			Total Volume
		Left	Thru	Thru	Right	Left	Thru	Right	
233	Sand Hill Road and Sand Hill Circle	4	293	51	40	0	795	32	1215

ID	Intersection Name	Northbound		Southbound	Eastbound		Total Volume
		Thru	Right	Left	Left	Thru	
234	Sand Hill Rd/Hwy 280 NB Off-Ramp	112	478	48	156	1696	2490

Menlo Park GP Circulation Update

Vistro File: J:\...\Menlo Park AM_update.vistro
Report File: J:\...\Menlo Park_AM Results.pdf

Scenario 1: Existing AM
1/9/2015

Turning Movement Volume: Detail

ID	Intersection Name	Volume Type	Northeastbound		Southwestbound		Southeastbound		Total Volume
			Thru		Thru		Left	Right	
1	Marsh Rd (SR 84)/US 101 SB Offramp	Final Base	784		1747		953	320	3804
		Growth Rate	1.00		1.00		1.00	1.00	-
		In Process	0		0		0	0	0
		Net New Trips	0		0		0	0	0
		Other	0		0		0	0	0
		Future Total	784		1747		953	320	3804

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
2	Marsh Rd/Rolison Rd-Scott Dr	Final Base	28	1184	14	439	1422	296	15	4	71	239	12	5	3729	
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	28	1184	14	439	1422	296	15	4	71	239	12	5	3729	

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
3	Marsh Rd/Florence St-Bohannon Dr	Final Base	99	767	90	27	1068	435	447	47	198	27	16	25	3246	
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	99	767	90	27	1068	435	447	47	198	27	16	25	3246	

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
4	Marsh Rd/Bay Rd	Final Base	1	803	82	268	867	31	114	78	7	45	19	173	2488	
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	1	803	82	268	867	31	114	78	7	45	19	173	2488	

ID	Intersection Name	Volume Type	Northeastbound		Northwestbound		Southeastbound		Total Volume
			Left	Right	Left	Thru	Thru	Right	
9	Middlefield Rd/Ravenswood Ave	Final Base	85	522	446	364	420	75	1912
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	85	522	446	364	420	75	1912

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
10	Middlefield Rd/Ringswood Ave	Final Base	5	3	12	136	44	251	55	559	84	216	653	62	2080
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	5	3	12	136	44	251	55	559	84	216	653	62	2080

ID	Intersection Name	Volume Type	Northeastbound		Southwestbound		Northwestbound		Total Volume
			Thru	Right	Left	Thru	Left	Right	
15	Bayfront Expy (SR 84)/University Ave (SR 109)	Final Base	873	85	1495	3915	185	359	6912
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	873	85	1495	3915	185	359	6912

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
16	Bayfront Expy (SR 84)/Willow Rd (SR 114)	Final Base	128	380	361	16	59	42	217	713	155	732	2780	24	5607
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	128	380	361	16	59	42	217	713	155	732	2780	24	5607

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
17	Willow Rd (SR 114)/Hamilton Ave	Final Base	124	807	71	88	795	57	78	22	50	26	24	23	2165
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	124	807	71	88	795	57	78	22	50	26	24	23	2165

ID	Intersection Name	Volume Type	Northbound		Southbound		Eastbound		Total Volume
			Left	Thru	Thru	Right	Left	Right	
18	Willow Rd (SR 114)/Ivy Dr	Final Base	129	1141	842	9	17	147	2285
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	129	1141	842	9	17	147	2285

ID	Intersection Name	Volume Type	Northbound		Southbound		Westbound		Total Volume
			Thru	Right	Left	Thru	Left	Right	
19	Willow Rd (SR 114)/O'Brien Dr	Final Base	1201	304	54	969	195	58	2781
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	1201	304	54	969	195	58	2781

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
20	Willow Rd (SR 114)/Newbridge St	Final Base	151	1407	166	51	1132	10	37	169	281	273	120	42	3839
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	151	1407	166	51	1132	10	37	169	281	273	120	42	3839

ID	Intersection Name	Volume Type	Northeastbound		Southwestbound		Southeastbound		Total Volume
			Left	Thru	Thru	Right	Left	Right	
21	Willow Rd/Bay Rd	Final Base	67	1278	1161	445	363	60	3374
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	67	1278	1161	445	363	60	3374

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
22	Willow Rd/Durham St-VA Med Entrance	Final Base	26	1040	16	36	925	89	40	3	10	48	12	70	2315
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	26	1040	16	36	925	89	40	3	10	48	12	70	2315

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
23	Willow Rd/Coleman Ave	Final Base	22	864	3	1	805	120	191	6	51	4	4	5	2076
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	22	864	3	1	805	120	191	6	51	4	4	5	2076

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
24	Willow Rd/Gilbert Ave	Final Base	3	745	40	27	826	4	37	70	17	78	112	100	2059
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	3	745	40	27	826	4	37	70	17	78	112	100	2059

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
25	Middlefield Rd-Willow Rd	Final Base	28	201	139	458	75	391	80	368	256	320	356	15	2687
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	28	201	139	458	75	391	80	368	256	320	356	15	2687

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
26	Ravenswood Ave/Laurel St	Final Base	15	618	67	20	471	11	138	113	13	136	188	65	1855
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	15	618	67	20	471	11	138	113	13	136	188	65	1855

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
28	Oak Grove Ave/Laurel St	Final Base	11	258	85	30	345	61	30	107	16	72	200	52	1267
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	11	258	85	30	345	61	30	107	16	72	200	52	1267

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
29	El Camino Real (SR 82)/Encinal Ave-Menlo College Entrance	Final Base	8	57	9	156	14	149	16	840	113	169	1838	71	3440
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	8	57	9	156	14	149	16	840	113	169	1838	71	3440

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
30	El Camino Real (SR 82)/Glenwood Ave-Valparaiso Ave	Final Base	255	126	124	84	177	16	101	700	35	54	1349	547	3568	
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	255	126	124	84	177	16	101	700	35	54	1349	547	3568	

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
31	El Camino Real (SR 82)/Oak Grove Ave	Final Base	57	196	53	117	254	63	89	731	73	125	1365	85	3208
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	57	196	53	117	254	63	89	731	73	125	1365	85	3208

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound		Southeastbound		Total Volume
			Left	Thru	Right	Left	Thru	Right	Thru	Right	Thru	Right	
32	El Camino Real (SR 82)/Santa Cruz Ave	Final Base	78	54	76	41	44	30	818	39	1420	16	2616
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0
		Future Total	78	54	76	41	44	30	818	39	1420	16	2616

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
33	El Camino Real (SR 82)/Ravenswood Ave-Menlo Ave	Final Base	24	345	60	417	214	44	88	779	396	161	1420	16	3964
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	24	345	60	417	214	44	88	779	396	161	1420	16	3964

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
34	El Camino Real (SR 82)/Roble Ave	Final Base	65	8	68	6	3	10	43	1188	25	43	1755	43	3257
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	65	8	68	6	3	10	43	1188	25	43	1755	43	3257

ID	Intersection Name	Volume Type	Northeastbound		Northwestbound		Southeastbound		Total Volume
			Left	Right	Left	Thru	Thru	Right	
35	El Camino Real (SR 82)/Middle Ave	Final Base	185	285	182	1040	1753	68	3513
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	185	285	182	1040	1753	68	3513

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
36	El Camino Real (SR 82)/Cambridge Ave	Final Base	28	0	50	1	0	0	119	1204	1	30	2068	15	3516
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	28	0	50	1	0	0	119	1204	1	30	2068	15	3516

ID	Intersection Name	Volume Type	Northeastbound		Southwestbound		Northwestbound		Total Volume
			Thru	Right	Left	Thru	Left	Right	
38	Santa Cruz Ave/University Dr (S)	Final Base	397	386	59	340	302	67	1551
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	397	386	59	340	302	67	1551

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
39	Santa Cruz Ave/Sand Hill Rd	Final Base	231	1143	261	331	559	51	94	615	389	223	669	236	4802	
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	231	1143	261	331	559	51	94	615	389	223	669	236	4802	

ID	Intersection Name	Volume Type	Northbound		Southbound		Eastbound			Total Volume
			Left	Thru	Thru	Right	Left	Thru	Right	
58	University Avenue and Adams Drive	Final Base	42	508	1449	140	6	0	1	2146
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0
		Future Total	42	508	1449	140	6	0	1	2146

ID	Intersection Name	Volume Type	Northbound		Southbound		Eastbound		Total Volume
			Left	Thru	Thru	Right	Left	Right	
74	University Ave/O'Brien Dr	Final Base	67	532	1284	142	16	25	2066
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	67	532	1284	142	16	25	2066

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
88	Valparaiso Ave/University Dr	Final Base	152	429	122	42	550	59	56	85	33	71	51	62	1712	
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	152	429	122	42	550	59	56	85	33	71	51	62	1712	

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
103	Addison Wesley/Sand Hill Rd	Final Base	23	2	41	2	2	7	103	1888	233	113	852	39	3305
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	23	2	41	2	2	7	103	1888	233	113	852	39	3305

ID	Intersection Name	Volume Type	Southbound		Westbound		Northeastbound		Total Volume
			Left	Thru	Left	Right	Thru	Right	
107	Alpine Rd/Santa Cruz Ave&Junipero Serra Blvd	Final Base	663	556	65	172	852	517	2825
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	663	556	65	172	852	517	2825

ID	Intersection Name	Volume Type	Northbound	Southbound	Northwestbound		Total Volume
			Thru	Thru	Left	Right	
110	Marsh Road and US 101 NB Ramps	Final Base	1097	1261	848	349	3555
		Growth Rate	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0
		Net New Trips	0	0	0	0	0
		Other	0	0	0	0	0
		Future Total	1097	1261	848	349	3555

ID	Intersection Name	Volume Type	Northeastbound	Southwestbound		Southeastbound		Total Volume
			Thru	Thru	Right	Left	Right	
132	Oak Ave/Sand Hill Rd	Final Base	1794	740	38	77	189	2838
		Growth Rate	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0
		Other	0	0	0	0	0	0
		Future Total	1794	740	38	77	189	2838

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
156	Saga Ln/Sand Hill Rd	Final Base	10	0	33	14	1	9	211	1594	117	73	760	50	2872
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	10	0	33	14	1	9	211	1594	117	73	760	50	2872

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
157	Branner Dr/Sand Hill Rd	Final Base	7	1	29	12	1	8	37	1666	12	17	867	21	2678
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	7	1	29	12	1	8	37	1666	12	17	867	21	2678

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
162	Sharon Park Dr/ Sand Hill Rd	Final Base	145	1496	17	27	732	136	2	0	9	215	3	174	2956
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	145	1496	17	27	732	136	2	0	9	215	3	174	2956

ID	Intersection Name	Volume Type	Northbound			Eastbound			Westbound			Southwestbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
163	Bayfront Expy/Marsh Rd	Final Base	143	35	1141	5	66	209	2636	407	12	6	35	8	4703
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	143	35	1141	5	66	209	2636	407	12	6	35	8	4703

ID	Intersection Name	Volume Type	Northeastbound		Southwestbound		Southeastbound		Total Volume
			Left	Thru	Thru	Right	Left	Right	
181	Santa Cruz Ave/Elder Ave	Final Base	64	636	403	112	100	74	1389
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	64	636	403	112	100	74	1389

ID	Intersection Name	Volume Type	Northbound		Westbound		Southeastbound		Total Volume
			Left	Right	Left	Thru	Thru	Right	
195	Bayfront Expy/Chilco St	Final Base	205	26	176	2726	933	169	4235
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	205	26	176	2726	933	169	4235

ID	Intersection Name	Volume Type	Northbound		Eastbound		Westbound		Total Volume
			Left	Right	Thru	Right	Left	Thru	
196	Bayfront Expy/Chrysler Drive	Final Base	179	12	1106	56	22	2914	4289
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	179	12	1106	56	22	2914	4289

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Northwestbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
207	Chilco St/Constitution Dr	Final Base	114	148	74	133	204	8	5	5	17	14	8	28	758
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	114	148	74	133	204	8	5	5	17	14	8	28	758

ID	Intersection Name	Volume Type	Southbound			Eastbound			Westbound			Northeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
209	Jefferson Dr/Constitution Dr	Final Base	1	0	0	1	53	0	2	17	1	1	0	11	87
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	1	0	0	1	53	0	2	17	1	1	0	11	87

ID	Intersection Name	Volume Type	Northbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
213	Chrysler Dr/Independence Dr	Final Base	8	33	5	1	6	0	29	11	91	0	0	1	185
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	8	33	5	1	6	0	29	11	91	0	0	1	185

ID	Intersection Name	Volume Type	Southbound		Northeastbound		Northwestbound			Total Volume
			Left	Thru	Thru	Right	Left	Thru	Right	
214	Chrysler Dr/Jefferson Dr	Final Base	111	39	23	14	7	0	11	205
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0
		Future Total	111	39	23	14	7	0	11	205

ID	Intersection Name	Volume Type	Southbound			Eastbound			Westbound			Northeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
215	Chrysler Dr/Constitution Dr	Final Base	23	47	12	19	127	97	19	5	102	1	28	2	482
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	23	47	12	19	127	97	19	5	102	1	28	2	482

ID	Intersection Name	Volume Type	Northbound		Southbound		Westbound			Total Volume
			Left	Thru	Thru	Right	Left	Thru	Right	
233	Sand Hill Road and Sand Hill Circle	Final Base	4	293	51	40	0	795	32	1215
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0
		Future Total	4	293	51	40	0	795	32	1215

ID	Intersection Name	Volume Type	Northbound		Southbound	Eastbound		Total Volume
			Thru	Right	Left	Left	Thru	
234	Sand Hill Rd/Hwy 280 NB Off-Ramp	Final Base	112	478	48	156	1696	2490
		Growth Rate	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0
		Other	0	0	0	0	0	0
		Future Total	112	478	48	156	1696	2490

Signal Warrants Report For Intersection #58: University Avenue and Adams Drive

Warrants Summary

Warrant	Name	Met?
#1	Eight Hour Vehicular Volume	No
#2	Four Hour Vehicular Volume	No
#3	Peak Hour	No

Intersection Warrants Parameters

Major Approaches	S, N
Minor Approaches	W
Speed > 40mph	No
Population < 10,000	No
Warrant Factor	100%

Warrant Analysis Traffic Volumes

Hour	Major Streets		Minor Streets
	S	N	W
1	11	32	0
2	11	32	0
3	17	48	0
4	17	48	0
5	22	64	0
6	55	159	1
7	61	175	1
8	110	318	1
9	193	556	2
10	198	572	3
11	198	572	3
12	215	620	3
13	237	683	3
14	248	715	3
15	248	715	3
16	264	763	3
17	330	953	4
18	347	1001	4
19	374	1081	5
20	418	1208	5
21	440	1271	6
22	517	1494	7
23	528	1525	7
24	550	1589	7

Warrant Analysis by Hour

Hour	Major Lanes		Minor Lanes		Warrant 1 Condition A				Warrant 1 Condition B				Warrant 2	Warrant 3 Condition B
	Number	Volume	Number	Volume	100%	80%	70%	56%	100%	80%	70%	56%		
1	5	43	1	0	No	No	No	No	No	No	No	No	No	No
2	5	43	1	0	No	No	No	No	No	No	No	No	No	No
3	5	65	1	0	No	No	No	No	No	No	No	No	No	No
4	5	65	1	0	No	No	No	No	No	No	No	No	No	No
5	5	86	1	0	No	No	No	No	No	No	No	No	No	No
6	5	214	1	1	No	No	No	No	No	No	No	No	No	No
7	5	236	1	1	No	No	No	No	No	No	No	No	No	No
8	5	428	1	1	No	No	No	No	No	No	No	No	No	No
9	5	749	1	2	No	No	No	No	No	No	No	No	No	No
10	5	770	1	3	No	No	No	No	No	No	No	No	No	No
11	5	770	1	3	No	No	No	No	No	No	No	No	No	No
12	5	835	1	3	No	No	No	No	No	No	No	No	No	No
13	5	920	1	3	No	No	No	No	No	No	No	No	No	No
14	5	963	1	3	No	No	No	No	No	No	No	No	No	No
15	5	963	1	3	No	No	No	No	No	No	No	No	No	No
16	5	1027	1	3	No	No	No	No	No	No	No	No	No	No
17	5	1283	1	4	No	No	No	No	No	No	No	No	No	No
18	5	1348	1	4	No	No	No	No	No	No	No	No	No	No
19	5	1455	1	5	No	No	No	No	No	No	No	No	No	No
20	5	1626	1	5	No	No	No	No	No	No	No	No	No	No
21	5	1711	1	6	No	No	No	No	No	No	No	No	No	No
22	5	2011	1	7	No	No	No	No	No	No	No	No	No	No
23	5	2053	1	7	No	No	No	No	No	No	No	No	No	No
24	5	2139	1	7	No	No	No	No	No	No	No	No	No	No
Hours Met					0	0	0	0	0	0	0	0	0	0

Warrant 3 Condition A

Orientation	W
Total Stopped Delay Per Vehicle on Minor Approach (s)	131.7
Number of Lanes on Minor Street Approach	1
VehicleHours of Stopped Delay on Minor Approach ([h]h:mm)	0:15
Delay Condition Met	No
Volume on Minor Street Approach During Same Hour	7
High Minor Volume Condition Met	No
Total Entering Volume on All Approaches During Same Hour	2146
Number of Approaches on Intersection	3
Total Volume Condition Met	Yes
Warrant Met for Approach	No
Warrant Met for Intersection	No

Signal Warrants Report For Intersection #207: Chilco St/Constitution Dr

Warrants Summary

Warrant	Name	Met?
#1	Eight Hour Vehicular Volume	No
#2	Four Hour Vehicular Volume	No
#3	Peak Hour	No

Intersection Warrants Parameters

Major Approaches	SE, W
Minor Approaches	N, S
Speed > 40mph	No
Population < 10,000	No
Warrant Factor	100%

Warrant Analysis Traffic Volumes

Hour	Major Streets		Minor Streets	
	SE	W	N	S
1	1	1	7	7
2	1	1	7	7
3	2	1	10	10
4	2	1	10	10
5	2	1	14	13
6	5	3	35	34
7	6	3	38	37
8	10	5	69	67
9	18	9	121	118
10	18	10	124	121
11	18	10	124	121
12	20	11	135	131
13	22	12	148	144
14	23	12	155	151
15	23	12	155	151
16	24	13	166	161
17	30	16	207	202
18	32	17	217	212
19	34	18	235	228
20	38	21	262	255
21	40	22	276	269
22	47	25	324	316
23	48	26	331	323
24	50	27	345	336

Warrant Analysis by Hour

Hour	Major Lanes		Minor Lanes		Warrant 1 Condition A				Warrant 1 Condition B				Warrant 2	Warrant 3 Condition B
	Number	Volume	Number	Volume	100%	80%	70%	56%	100%	80%	70%	56%		
1	2	2	2	14	No	No	No	No	No	No	No	No	No	No
2	2	2	2	14	No	No	No	No	No	No	No	No	No	No
3	2	3	2	20	No	No	No	No	No	No	No	No	No	No
4	2	3	2	20	No	No	No	No	No	No	No	No	No	No
5	2	3	2	27	No	No	No	No	No	No	No	No	No	No
6	2	8	2	69	No	No	No	No	No	No	No	No	No	No
7	2	9	2	75	No	No	No	No	No	No	No	No	No	No
8	2	15	2	136	No	No	No	No	No	No	No	No	No	No
9	2	27	2	239	No	No	No	No	No	No	No	No	No	No
10	2	28	2	245	No	No	No	No	No	No	No	No	No	No
11	2	28	2	245	No	No	No	No	No	No	No	No	No	No
12	2	31	2	266	No	No	No	No	No	No	No	No	No	No
13	2	34	2	292	No	No	No	No	No	No	No	No	No	No
14	2	35	2	306	No	No	No	No	No	No	No	No	No	No
15	2	35	2	306	No	No	No	No	No	No	No	No	No	No
16	2	37	2	327	No	No	No	No	No	No	No	No	No	No
17	2	46	2	409	No	No	No	No	No	No	No	No	No	No
18	2	49	2	429	No	No	No	No	No	No	No	No	No	No
19	2	52	2	463	No	No	No	No	No	No	No	No	No	No
20	2	59	2	517	No	No	No	No	No	No	No	No	No	No
21	2	62	2	545	No	No	No	No	No	No	No	No	No	No
22	2	72	2	640	No	No	No	No	No	No	No	No	No	No
23	2	74	2	654	No	No	No	No	No	No	No	No	No	No
24	2	77	2	681	No	No	No	No	No	No	No	No	No	No
Hours Met					0	0	0	0	0	0	0	0	0	0

Warrant 3 Condition A

Orientation	N	S
Total Stopped Delay Per Vehicle on Minor Approach (s)	12	11.7
Number of Lanes on Minor Street Approach	1	1
VehicleHours of Stopped Delay on Minor Approach ([h]h:mm)	1:09	1:05
Delay Condition Met	No	No
Volume on Minor Street Approach During Same Hour	345	336
High Minor Volume Condition Met	Yes	Yes
Total Entering Volume on All Approaches During Same Hour	758	758
Number of Approaches on Intersection	4	4
Total Volume Condition Met	No	No
Warrant Met for Approach	No	No
Warrant Met for Intersection	No	

Signal Warrants Report For Intersection #209: Jefferson Dr/Constitution Dr

Warrants Summary

Warrant	Name	Met?
#1	Eight Hour Vehicular Volume	No
#2	Four Hour Vehicular Volume	No
#3	Peak Hour	No

Intersection Warrants Parameters

Major Approaches	E, W
Minor Approaches	N, SW
Speed > 40mph	No
Population < 10,000	No
Warrant Factor	100%

Warrant Analysis Traffic Volumes

Hour	Major Streets		Minor Streets	
	E	W	N	SW
1	0	1	0	0
2	0	1	0	0
3	1	2	0	0
4	1	2	0	0
5	1	2	0	0
6	2	5	0	1
7	2	6	0	1
8	4	11	0	2
9	7	19	0	4
10	7	19	0	4
11	7	19	0	4
12	8	21	0	5
13	9	23	0	5
14	9	24	0	5
15	9	24	0	5
16	10	26	0	6
17	12	32	1	7
18	13	34	1	8
19	14	37	1	8
20	15	41	1	9
21	16	43	1	10
22	19	51	1	11
23	19	52	1	12
24	20	54	1	12

Warrant Analysis by Hour

Hour	Major Lanes		Minor Lanes		Warrant 1 Condition A				Warrant 1 Condition B				Warrant 2	Warrant 3 Condition B
	Number	Volume	Number	Volume	100%	80%	70%	56%	100%	80%	70%	56%		
1	2	1	2	0	No	No	No	No	No	No	No	No	No	No
2	2	1	2	0	No	No	No	No	No	No	No	No	No	No
3	2	3	2	0	No	No	No	No	No	No	No	No	No	No
4	2	3	2	0	No	No	No	No	No	No	No	No	No	No
5	2	3	2	0	No	No	No	No	No	No	No	No	No	No
6	2	7	2	1	No	No	No	No	No	No	No	No	No	No
7	2	8	2	1	No	No	No	No	No	No	No	No	No	No
8	2	15	2	2	No	No	No	No	No	No	No	No	No	No
9	2	26	2	4	No	No	No	No	No	No	No	No	No	No
10	2	26	2	4	No	No	No	No	No	No	No	No	No	No
11	2	26	2	4	No	No	No	No	No	No	No	No	No	No
12	2	29	2	5	No	No	No	No	No	No	No	No	No	No
13	2	32	2	5	No	No	No	No	No	No	No	No	No	No
14	2	33	2	5	No	No	No	No	No	No	No	No	No	No
15	2	33	2	5	No	No	No	No	No	No	No	No	No	No
16	2	36	2	6	No	No	No	No	No	No	No	No	No	No
17	2	44	2	8	No	No	No	No	No	No	No	No	No	No
18	2	47	2	9	No	No	No	No	No	No	No	No	No	No
19	2	51	2	9	No	No	No	No	No	No	No	No	No	No
20	2	56	2	10	No	No	No	No	No	No	No	No	No	No
21	2	59	2	11	No	No	No	No	No	No	No	No	No	No
22	2	70	2	12	No	No	No	No	No	No	No	No	No	No
23	2	71	2	13	No	No	No	No	No	No	No	No	No	No
24	2	74	2	13	No	No	No	No	No	No	No	No	No	No
Hours Met					0	0	0	0	0	0	0	0	0	0

Warrant 3 Condition A

Orientation	N	SW
Total Stopped Delay Per Vehicle on Minor Approach (s)	9.2	8.7
Number of Lanes on Minor Street Approach	1	1
VehicleHours of Stopped Delay on Minor Approach ([h]h:mm)	0:00	0:01
Delay Condition Met	No	No
Volume on Minor Street Approach During Same Hour	1	12
High Minor Volume Condition Met	No	No
Total Entering Volume on All Approaches During Same Hour	87	87
Number of Approaches on Intersection	4	4
Total Volume Condition Met	No	No
Warrant Met for Approach	No	No
Warrant Met for Intersection	No	

Signal Warrants Report For Intersection #213: Chrysler Dr/Independence Dr

Warrants Summary

Warrant	Name	Met?
#1	Eight Hour Vehicular Volume	No
#2	Four Hour Vehicular Volume	No
#3	Peak Hour	No

Intersection Warrants Parameters

Major Approaches	NE, S
Minor Approaches	SE, NW
Speed > 40mph	No
Population < 10,000	No
Warrant Factor	100%

Warrant Analysis Traffic Volumes

Hour	Major Streets		Minor Streets	
	NE	S	SE	NW
1	0	1	3	0
2	0	1	3	0
3	0	1	4	0
4	0	1	4	0
5	0	2	5	0
6	1	5	13	0
7	1	5	14	0
8	1	9	26	0
9	2	16	46	0
10	3	17	47	0
11	3	17	47	0
12	3	18	51	0
13	3	20	56	0
14	3	21	59	0
15	3	21	59	0
16	3	22	63	0
17	4	28	79	1
18	4	29	83	1
19	5	31	89	1
20	5	35	100	1
21	6	37	105	1
22	7	43	123	1
23	7	44	126	1
24	7	46	131	1

Warrant Analysis by Hour

Hour	Major Lanes		Minor Lanes		Warrant 1 Condition A				Warrant 1 Condition B				Warrant 2	Warrant 3 Condition B
	Number	Volume	Number	Volume	100%	80%	70%	56%	100%	80%	70%	56%		
1	2	1	2	3	No	No	No	No	No	No	No	No	No	No
2	2	1	2	3	No	No	No	No	No	No	No	No	No	No
3	2	1	2	4	No	No	No	No	No	No	No	No	No	No
4	2	1	2	4	No	No	No	No	No	No	No	No	No	No
5	2	2	2	5	No	No	No	No	No	No	No	No	No	No
6	2	6	2	13	No	No	No	No	No	No	No	No	No	No
7	2	6	2	14	No	No	No	No	No	No	No	No	No	No
8	2	10	2	26	No	No	No	No	No	No	No	No	No	No
9	2	18	2	46	No	No	No	No	No	No	No	No	No	No
10	2	20	2	47	No	No	No	No	No	No	No	No	No	No
11	2	20	2	47	No	No	No	No	No	No	No	No	No	No
12	2	21	2	51	No	No	No	No	No	No	No	No	No	No
13	2	23	2	56	No	No	No	No	No	No	No	No	No	No
14	2	24	2	59	No	No	No	No	No	No	No	No	No	No
15	2	24	2	59	No	No	No	No	No	No	No	No	No	No
16	2	25	2	63	No	No	No	No	No	No	No	No	No	No
17	2	32	2	80	No	No	No	No	No	No	No	No	No	No
18	2	33	2	84	No	No	No	No	No	No	No	No	No	No
19	2	36	2	90	No	No	No	No	No	No	No	No	No	No
20	2	40	2	101	No	No	No	No	No	No	No	No	No	No
21	2	43	2	106	No	No	No	No	No	No	No	No	No	No
22	2	50	2	124	No	No	No	No	No	No	No	No	No	No
23	2	51	2	127	No	No	No	No	No	No	No	No	No	No
24	2	53	2	132	No	No	No	No	No	No	No	No	No	No
Hours Met					0	0	0	0	0	0	0	0	0	0

Warrant 3 Condition A

Orientation	SE	NW
Total Stopped Delay Per Vehicle on Minor Approach (s)	9.4	8.3
Number of Lanes on Minor Street Approach	1	1
VehicleHours of Stopped Delay on Minor Approach ([h]h:mm)	0:20	0:00
Delay Condition Met	No	No
Volume on Minor Street Approach During Same Hour	131	1
High Minor Volume Condition Met	Yes	No
Total Entering Volume on All Approaches During Same Hour	185	185
Number of Approaches on Intersection	4	4
Total Volume Condition Met	No	No
Warrant Met for Approach	No	No
Warrant Met for Intersection	No	

Signal Warrants Report For Intersection #214: Chrysler Dr/Jefferson Dr

Warrants Summary

Warrant	Name	Met?
#1	Eight Hour Vehicular Volume	No
#2	Four Hour Vehicular Volume	No
#3	Peak Hour	No

Intersection Warrants Parameters

Major Approaches	N, SW
Minor Approaches	SE
Speed > 40mph	No
Population < 10,000	No
Warrant Factor	100%

Warrant Analysis Traffic Volumes

Hour	Major Streets		Minor Streets
	N	SW	SE
1	3	1	0
2	3	1	0
3	5	1	1
4	5	1	1
5	6	1	1
6	15	4	2
7	17	4	2
8	30	7	4
9	53	13	6
10	54	13	6
11	54	13	6
12	59	14	7
13	65	16	8
14	68	17	8
15	68	17	8
16	72	18	9
17	90	22	11
18	95	23	11
19	102	25	12
20	114	28	14
21	120	30	14
22	141	35	17
23	144	36	17
24	150	37	18

Warrant Analysis by Hour

Hour	Major Lanes		Minor Lanes		Warrant 1 Condition A				Warrant 1 Condition B				Warrant 2	Warrant 3 Condition B
	Number	Volume	Number	Volume	100%	80%	70%	56%	100%	80%	70%	56%		
1	2	4	1	0	No	No	No	No	No	No	No	No	No	No
2	2	4	1	0	No	No	No	No	No	No	No	No	No	No
3	2	6	1	1	No	No	No	No	No	No	No	No	No	No
4	2	6	1	1	No	No	No	No	No	No	No	No	No	No
5	2	7	1	1	No	No	No	No	No	No	No	No	No	No
6	2	19	1	2	No	No	No	No	No	No	No	No	No	No
7	2	21	1	2	No	No	No	No	No	No	No	No	No	No
8	2	37	1	4	No	No	No	No	No	No	No	No	No	No
9	2	66	1	6	No	No	No	No	No	No	No	No	No	No
10	2	67	1	6	No	No	No	No	No	No	No	No	No	No
11	2	67	1	6	No	No	No	No	No	No	No	No	No	No
12	2	73	1	7	No	No	No	No	No	No	No	No	No	No
13	2	81	1	8	No	No	No	No	No	No	No	No	No	No
14	2	85	1	8	No	No	No	No	No	No	No	No	No	No
15	2	85	1	8	No	No	No	No	No	No	No	No	No	No
16	2	90	1	9	No	No	No	No	No	No	No	No	No	No
17	2	112	1	11	No	No	No	No	No	No	No	No	No	No
18	2	118	1	11	No	No	No	No	No	No	No	No	No	No
19	2	127	1	12	No	No	No	No	No	No	No	No	No	No
20	2	142	1	14	No	No	No	No	No	No	No	No	No	No
21	2	150	1	14	No	No	No	No	No	No	No	No	No	No
22	2	176	1	17	No	No	No	No	No	No	No	No	No	No
23	2	180	1	17	No	No	No	No	No	No	No	No	No	No
24	2	187	1	18	No	No	No	No	No	No	No	No	No	No
Hours Met					0	0	0	0	0	0	0	0	0	0

Warrant 3 Condition A

Orientation	SE
Total Stopped Delay Per Vehicle on Minor Approach (s)	9.9
Number of Lanes on Minor Street Approach	1
VehicleHours of Stopped Delay on Minor Approach ([h]h:mm)	0:02
Delay Condition Met	No
Volume on Minor Street Approach During Same Hour	18
High Minor Volume Condition Met	No
Total Entering Volume on All Approaches During Same Hour	205
Number of Approaches on Intersection	3
Total Volume Condition Met	No
Warrant Met for Approach	No
Warrant Met for Intersection	No

Signal Warrants Report For Intersection #215: Chrysler Dr/Constitution Dr

Warrants Summary

Warrant	Name	Met?
#1	Eight Hour Vehicular Volume	No
#2	Four Hour Vehicular Volume	No
#3	Peak Hour	No

Intersection Warrants Parameters

Major Approaches	E, W
Minor Approaches	N, SW
Speed > 40mph	No
Population < 10,000	No
Warrant Factor	100%

Warrant Analysis Traffic Volumes

Hour	Major Streets		Minor Streets	
	E	W	N	SW
1	3	5	2	1
2	3	5	2	1
3	4	7	2	1
4	4	7	2	1
5	5	10	3	1
6	13	24	8	3
7	14	27	9	3
8	25	49	16	6
9	44	85	29	11
10	45	87	30	11
11	45	87	30	11
12	49	95	32	12
13	54	104	35	13
14	57	109	37	14
15	57	109	37	14
16	60	117	39	15
17	76	146	49	19
18	79	153	52	20
19	86	165	56	21
20	96	185	62	24
21	101	194	66	25
22	118	228	77	29
23	121	233	79	30
24	126	243	82	31

Warrant Analysis by Hour

Hour	Major Lanes		Minor Lanes		Warrant 1 Condition A				Warrant 1 Condition B				Warrant 2	Warrant 3 Condition B
	Number	Volume	Number	Volume	100%	80%	70%	56%	100%	80%	70%	56%		
1	2	8	2	3	No	No	No	No	No	No	No	No	No	No
2	2	8	2	3	No	No	No	No	No	No	No	No	No	No
3	2	11	2	3	No	No	No	No	No	No	No	No	No	No
4	2	11	2	3	No	No	No	No	No	No	No	No	No	No
5	2	15	2	4	No	No	No	No	No	No	No	No	No	No
6	2	37	2	11	No	No	No	No	No	No	No	No	No	No
7	2	41	2	12	No	No	No	No	No	No	No	No	No	No
8	2	74	2	22	No	No	No	No	No	No	No	No	No	No
9	2	129	2	40	No	No	No	No	No	No	No	No	No	No
10	2	132	2	41	No	No	No	No	No	No	No	No	No	No
11	2	132	2	41	No	No	No	No	No	No	No	No	No	No
12	2	144	2	44	No	No	No	No	No	No	No	No	No	No
13	2	158	2	48	No	No	No	No	No	No	No	No	No	No
14	2	166	2	51	No	No	No	No	No	No	No	No	No	No
15	2	166	2	51	No	No	No	No	No	No	No	No	No	No
16	2	177	2	54	No	No	No	No	No	No	No	No	No	No
17	2	222	2	68	No	No	No	No	No	No	No	No	No	No
18	2	232	2	72	No	No	No	No	No	No	No	No	No	No
19	2	251	2	77	No	No	No	No	No	No	No	No	No	No
20	2	281	2	86	No	No	No	No	No	No	No	No	No	No
21	2	295	2	91	No	No	No	No	No	No	No	No	No	No
22	2	346	2	106	No	No	No	No	No	No	No	No	No	No
23	2	354	2	109	No	No	No	No	No	No	No	No	No	No
24	2	369	2	113	No	No	No	No	No	No	No	No	No	No
Hours Met					0	0	0	0	0	0	0	0	0	0

Warrant 3 Condition A

Orientation	N	SW
Total Stopped Delay Per Vehicle on Minor Approach (s)	8.7	8.6
Number of Lanes on Minor Street Approach	1	1
VehicleHours of Stopped Delay on Minor Approach ([h]h:mm)	0:11	0:04
Delay Condition Met	No	No
Volume on Minor Street Approach During Same Hour	82	31
High Minor Volume Condition Met	No	No
Total Entering Volume on All Approaches During Same Hour	482	482
Number of Approaches on Intersection	4	4
Total Volume Condition Met	No	No
Warrant Met for Approach	No	No
Warrant Met for Intersection	No	

Study Intersections

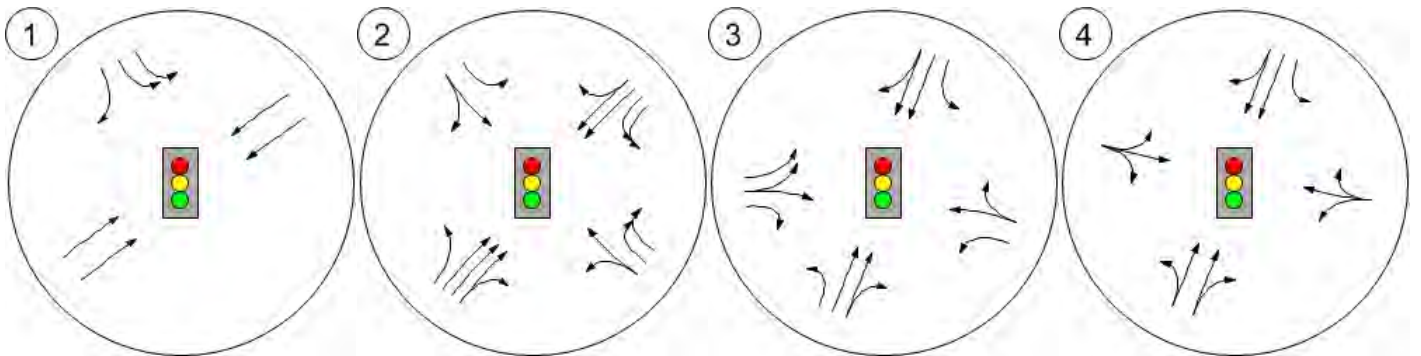


Lane Configuration and Traffic Control



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

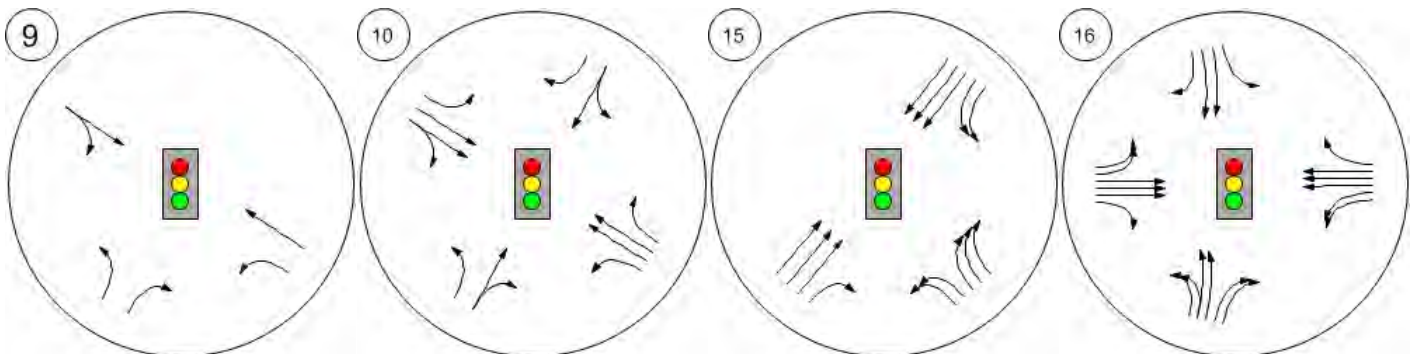


Middlefield Rd/Ravenswood

Middlefield Rd/Ringswood Av

Bayfront Expy (SR 84)/Univer

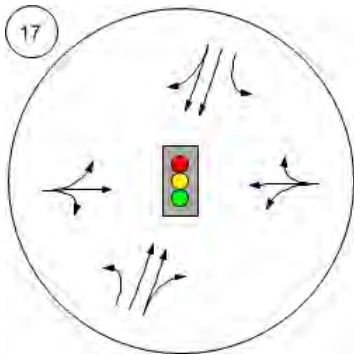
Bayfront Expy (SR 84)/Willow



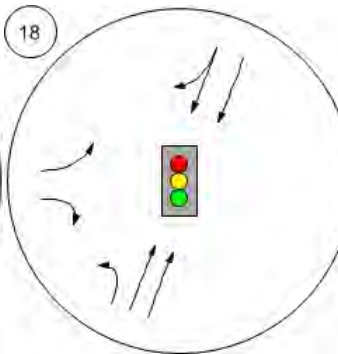
Lane Configuration and Traffic Control



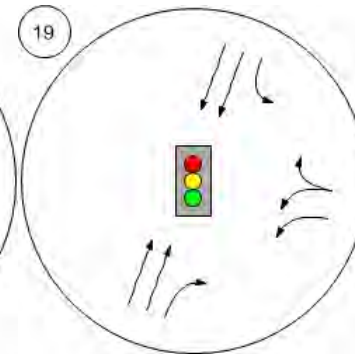
Willow Rd (SR 114)/Hamilton



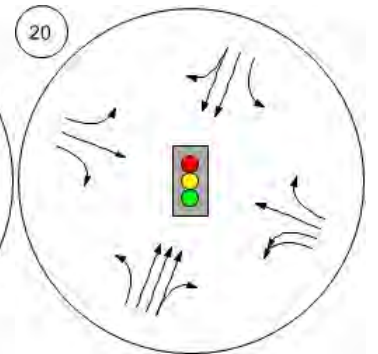
Willow Rd (SR 114)/Ivy Dr



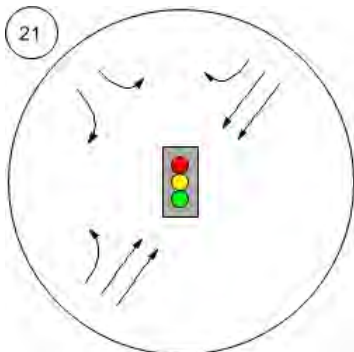
Willow Rd (SR 114)/O'Brien



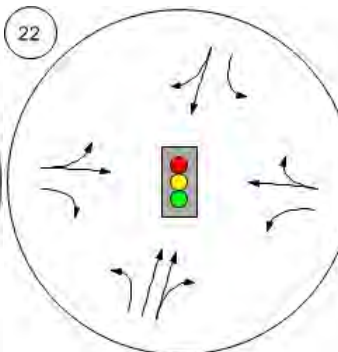
Willow Rd (SR 114)/Newbrid



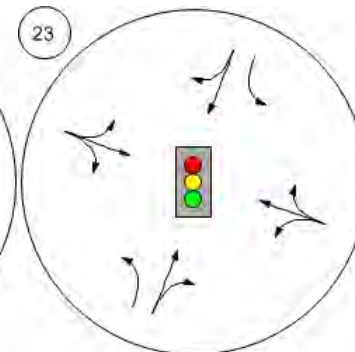
Willow Rd/Bay Rd



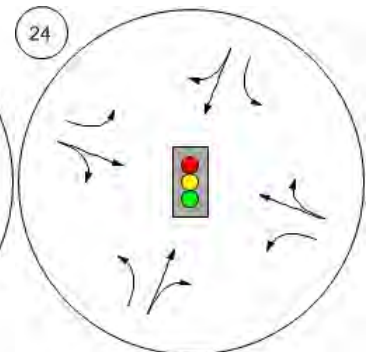
Willow Rd/Durham St-VA Me



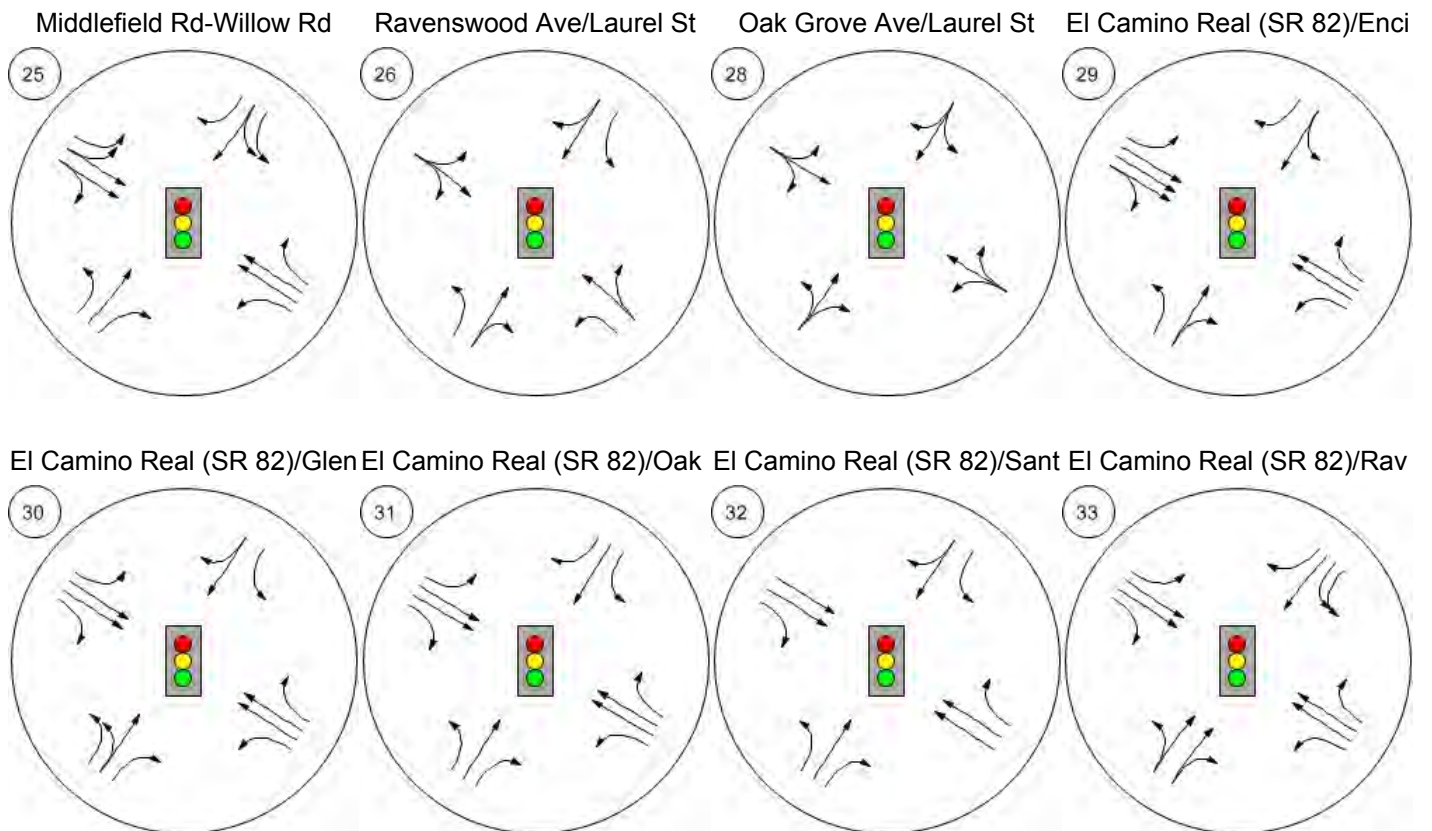
Willow Rd/Coleman Ave



Willow Rd/Gilbert Ave



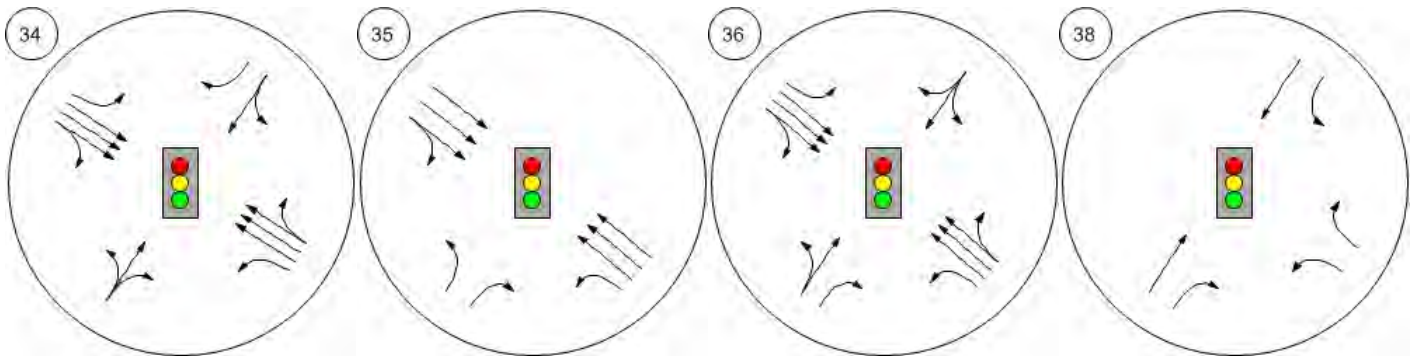
Lane Configuration and Traffic Control



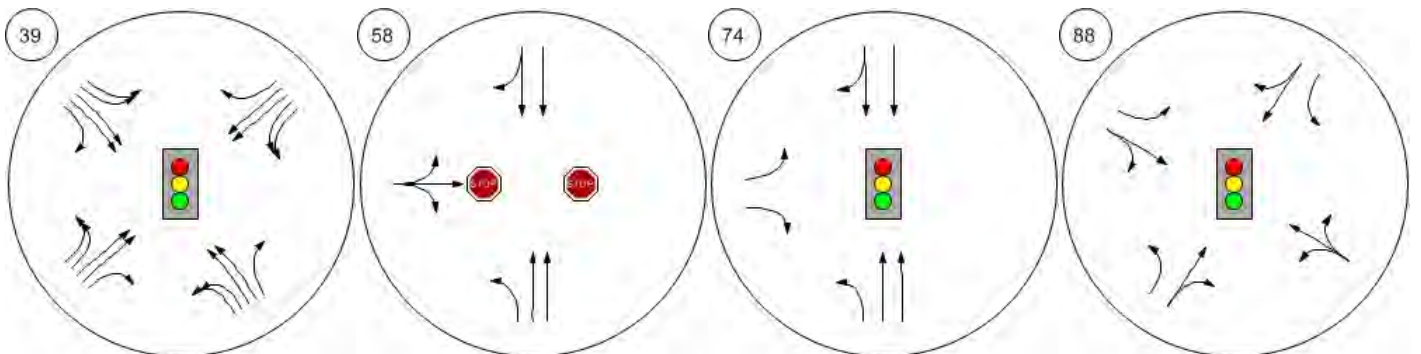
Lane Configuration and Traffic Control



El Camino Real (SR 82)/Robl El Camino Real (SR 82)/Middle El Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



Santa Cruz Ave/Sand Hill Rd University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr

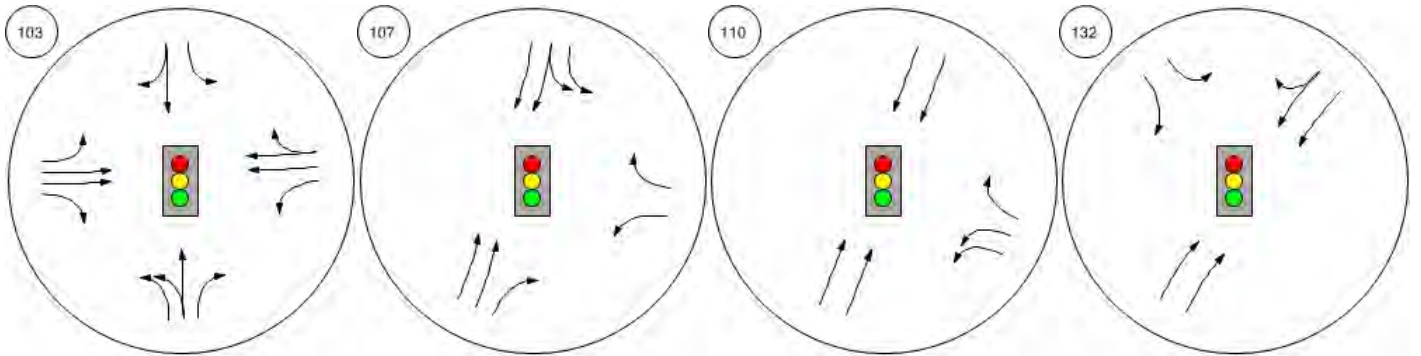


Lane Configuration and Traffic Control



Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road and US 101 NB

Oak Ave/Sand Hill Rd

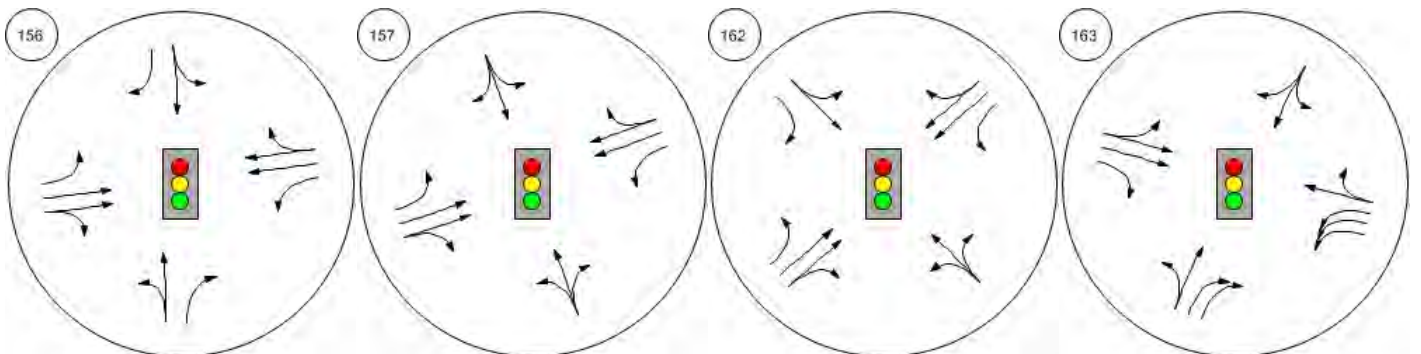


Saga Ln/Sand Hill Rd

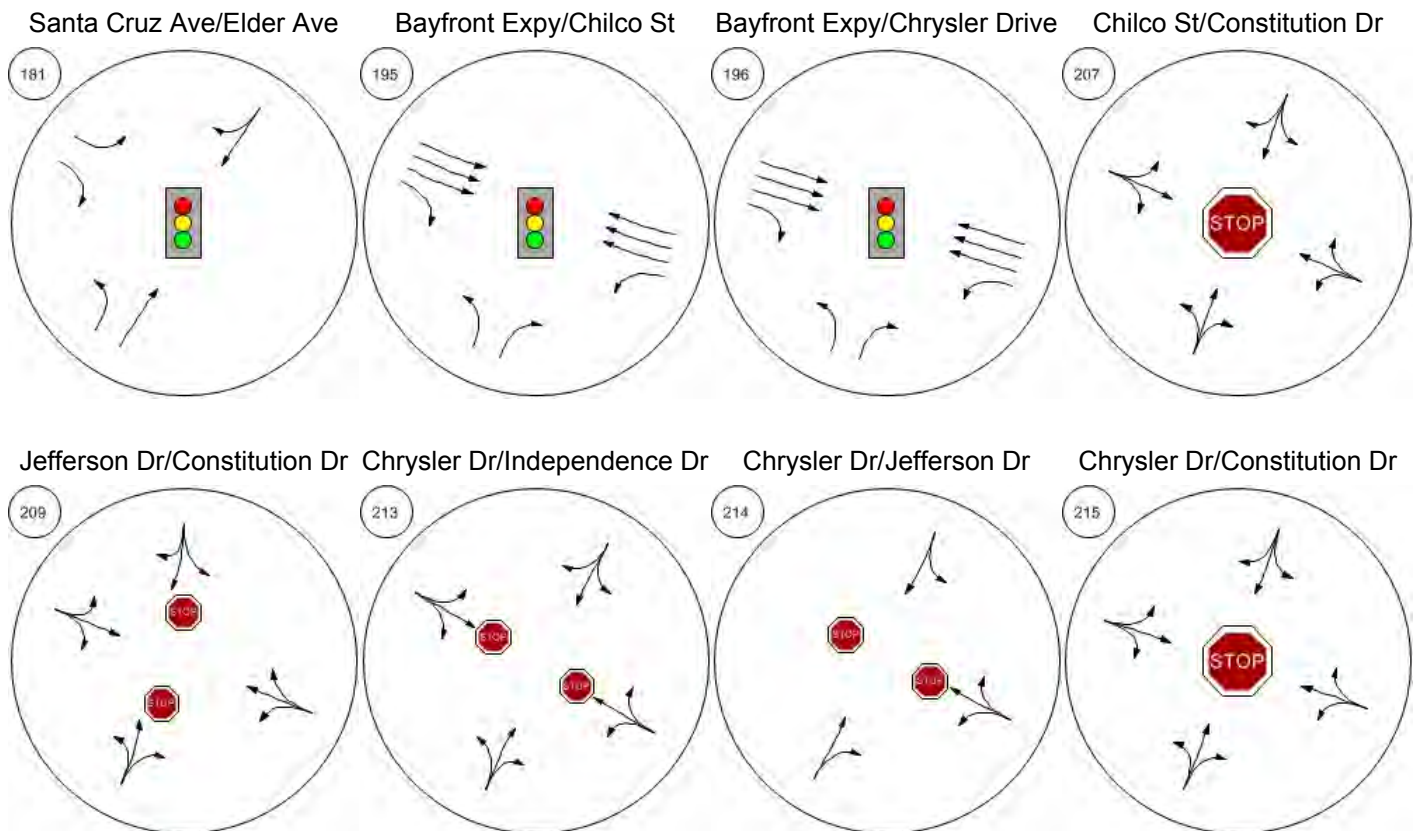
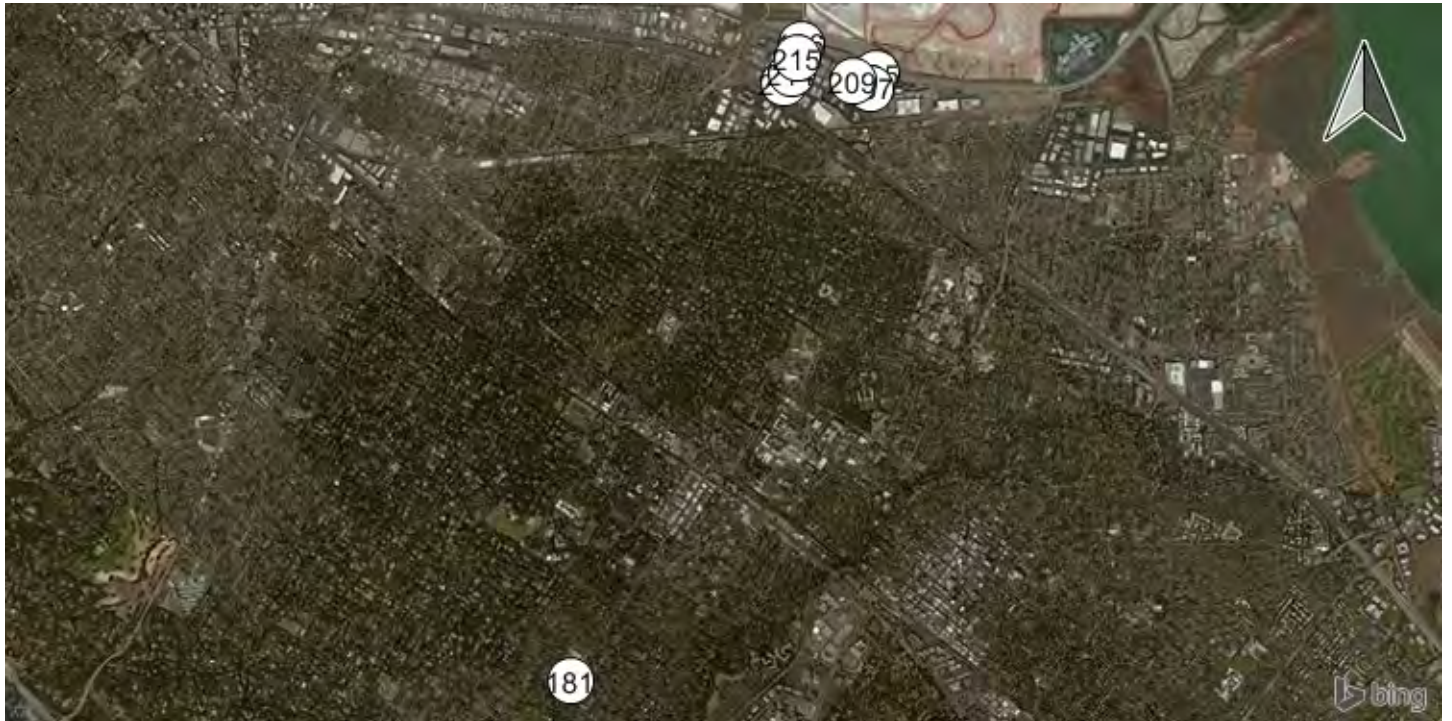
Branner Dr/Sand Hill Rd

Sharon Park Dr/ Sand Hill Rd

Bayfront Expy/Marsh Rd



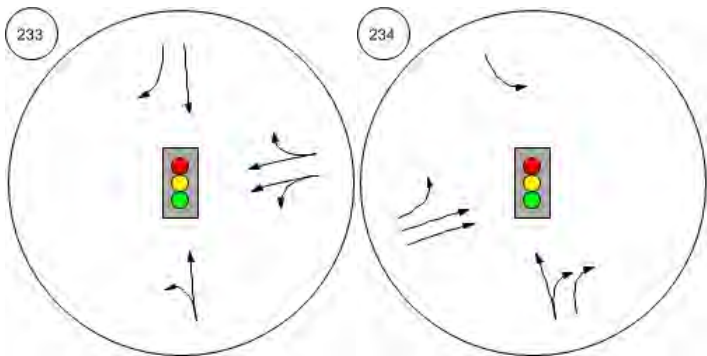
Lane Configuration and Traffic Control



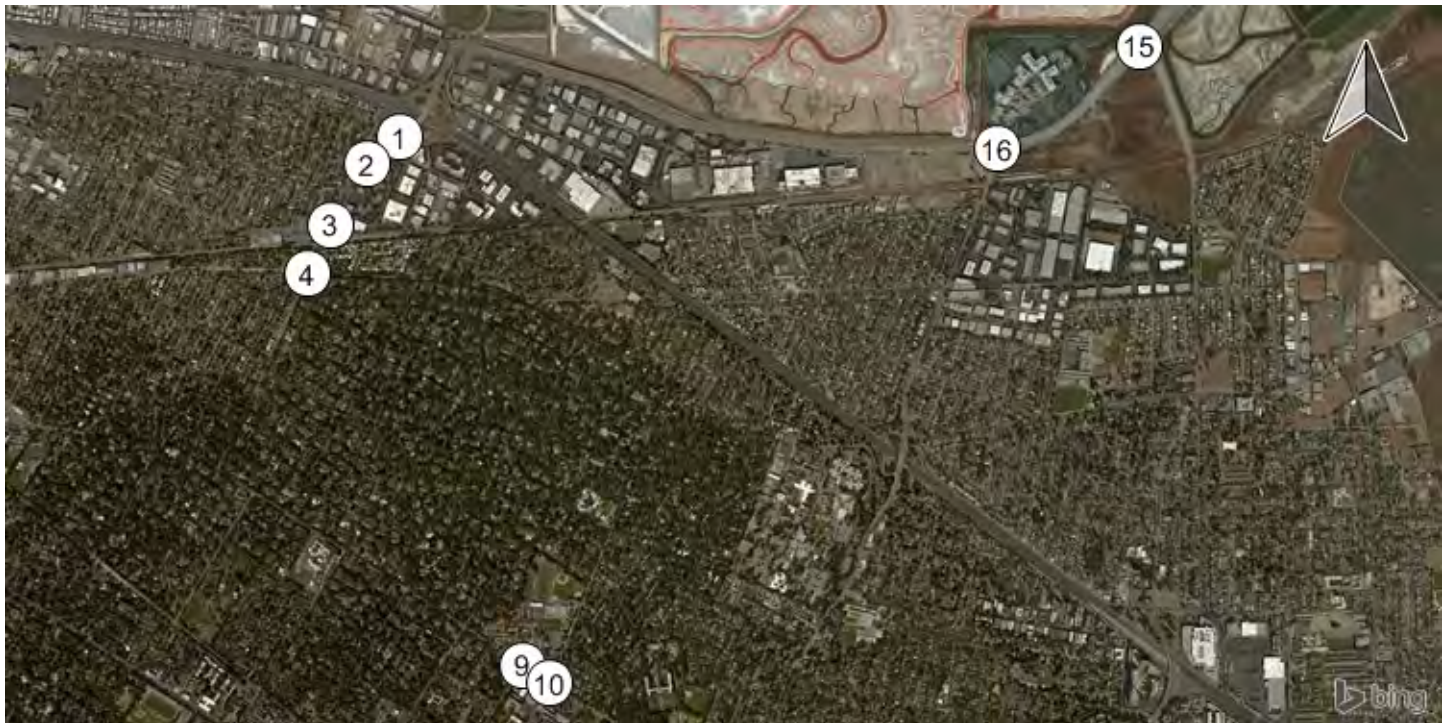
Lane Configuration and Traffic Control



Sand Hill Road and Sand Hill Sand Hill Rd/Hwy 280 NB Off

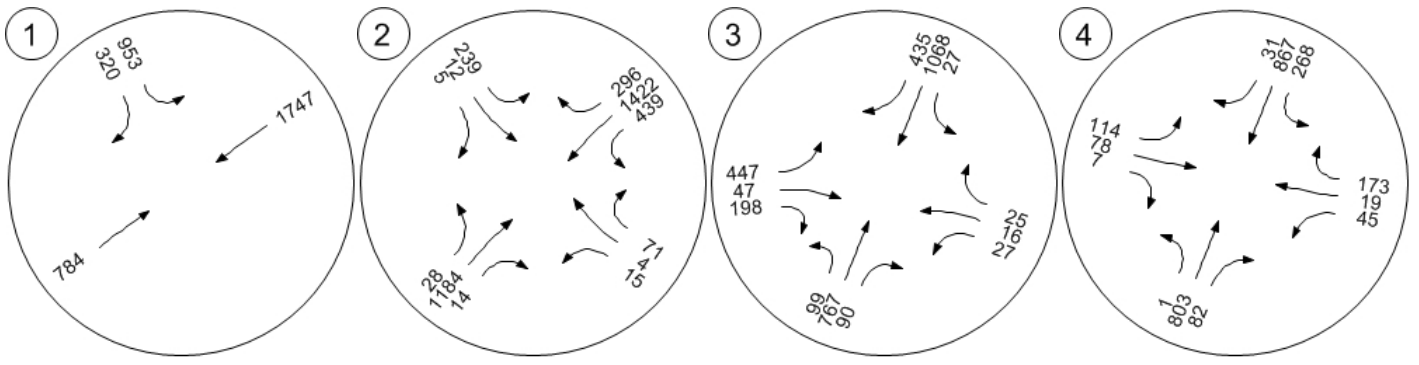


Traffic Volume - Base Volume



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

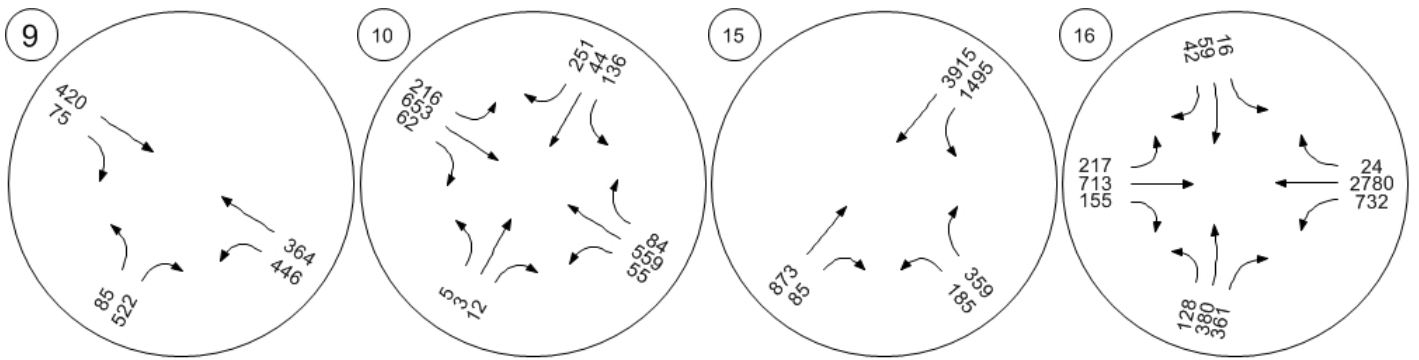


Middlefield Rd/Ravenswood

Middlefield Rd/Ringswood Av

Bayfront Expy (SR 84)/Univer

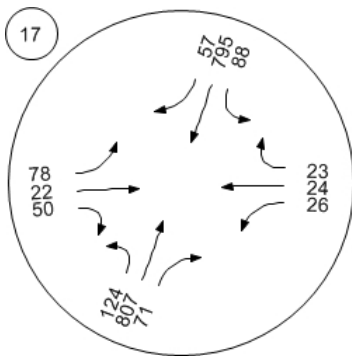
Bayfront Expy (SR 84)/Willow



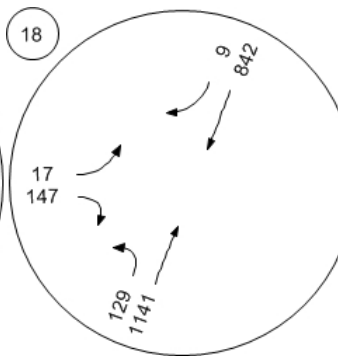
Traffic Volume - Base Volume



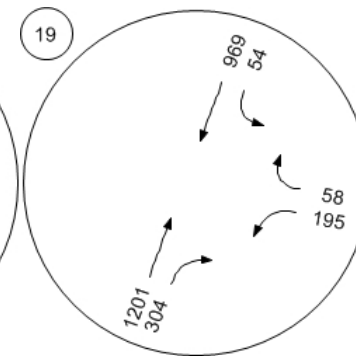
Willow Rd (SR 114)/Hamilton



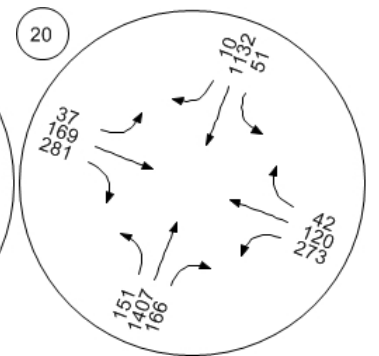
Willow Rd (SR 114)/Ivy Dr



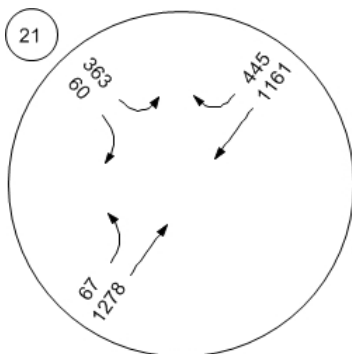
Willow Rd (SR 114)/O'Brien



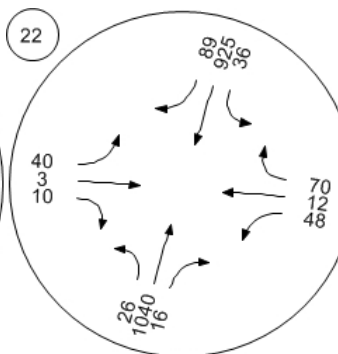
Willow Rd (SR 114)/Newbrid



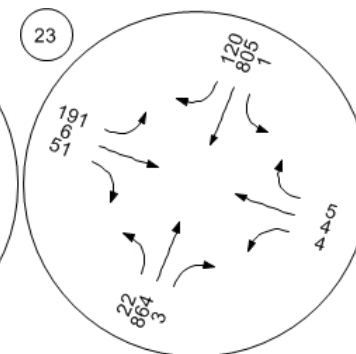
Willow Rd/Bay Rd



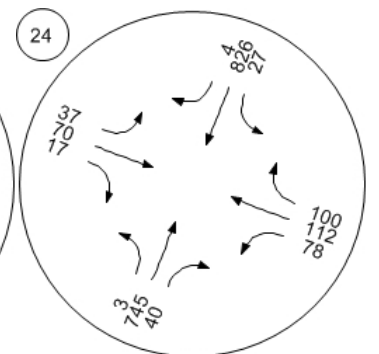
Willow Rd/Durham St-VA Me



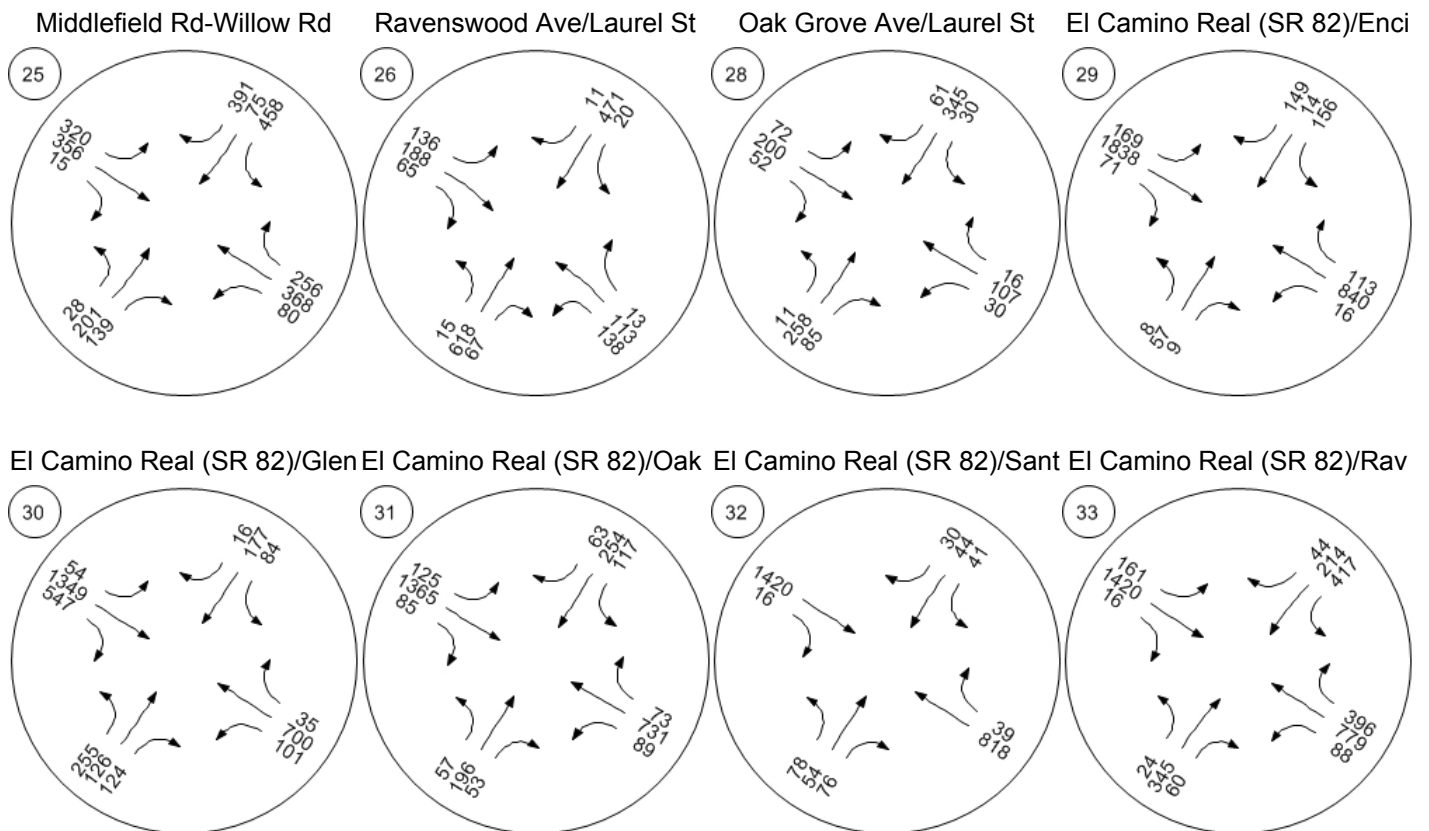
Willow Rd/Coleman Ave



Willow Rd/Gilbert Ave



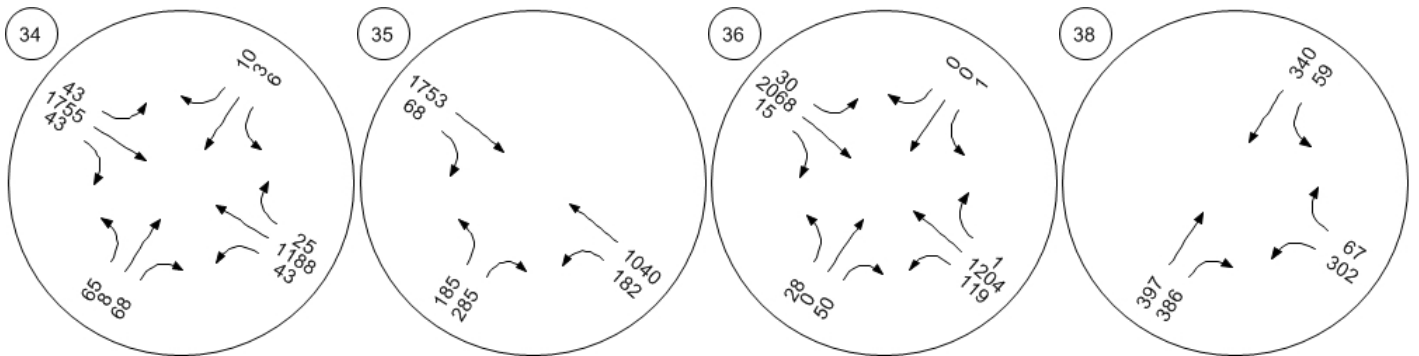
Traffic Volume - Base Volume



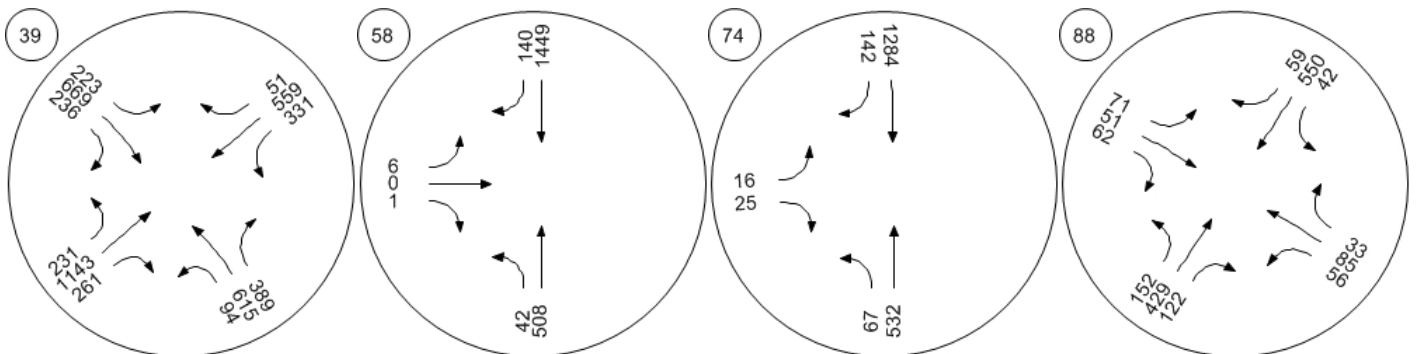
Traffic Volume - Base Volume



El Camino Real (SR 82)/Rob El Camino Real (SR 82)/Middle Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



Santa Cruz Ave/Sand Hill Rd University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr

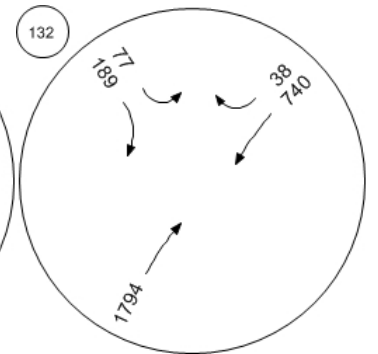
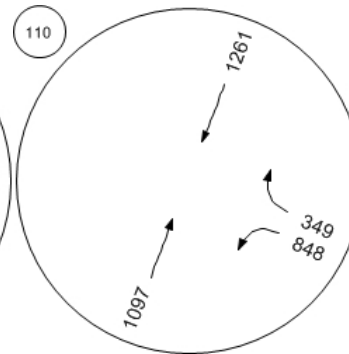
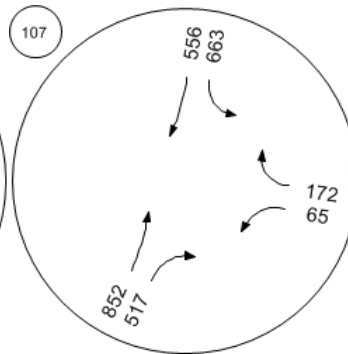
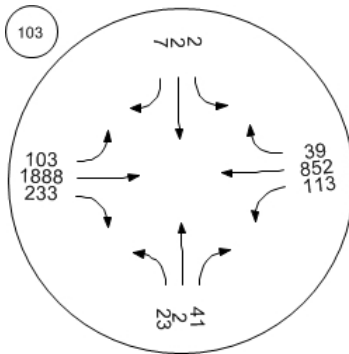


Traffic Volume - Base Volume



Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road and US 101 NB

Oak Ave/Sand Hill Rd

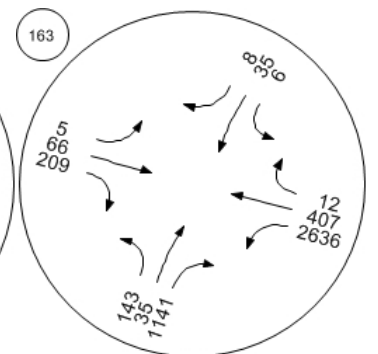
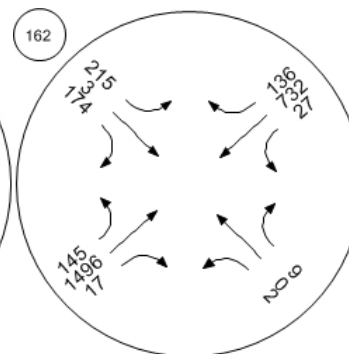
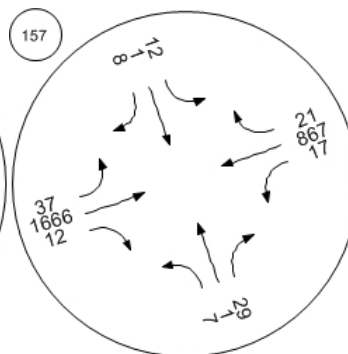
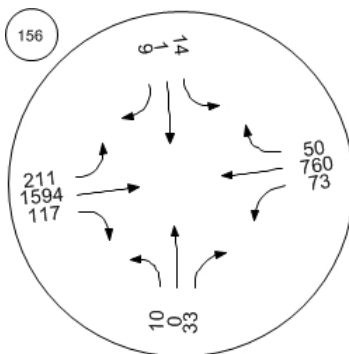


Saga Ln/Sand Hill Rd

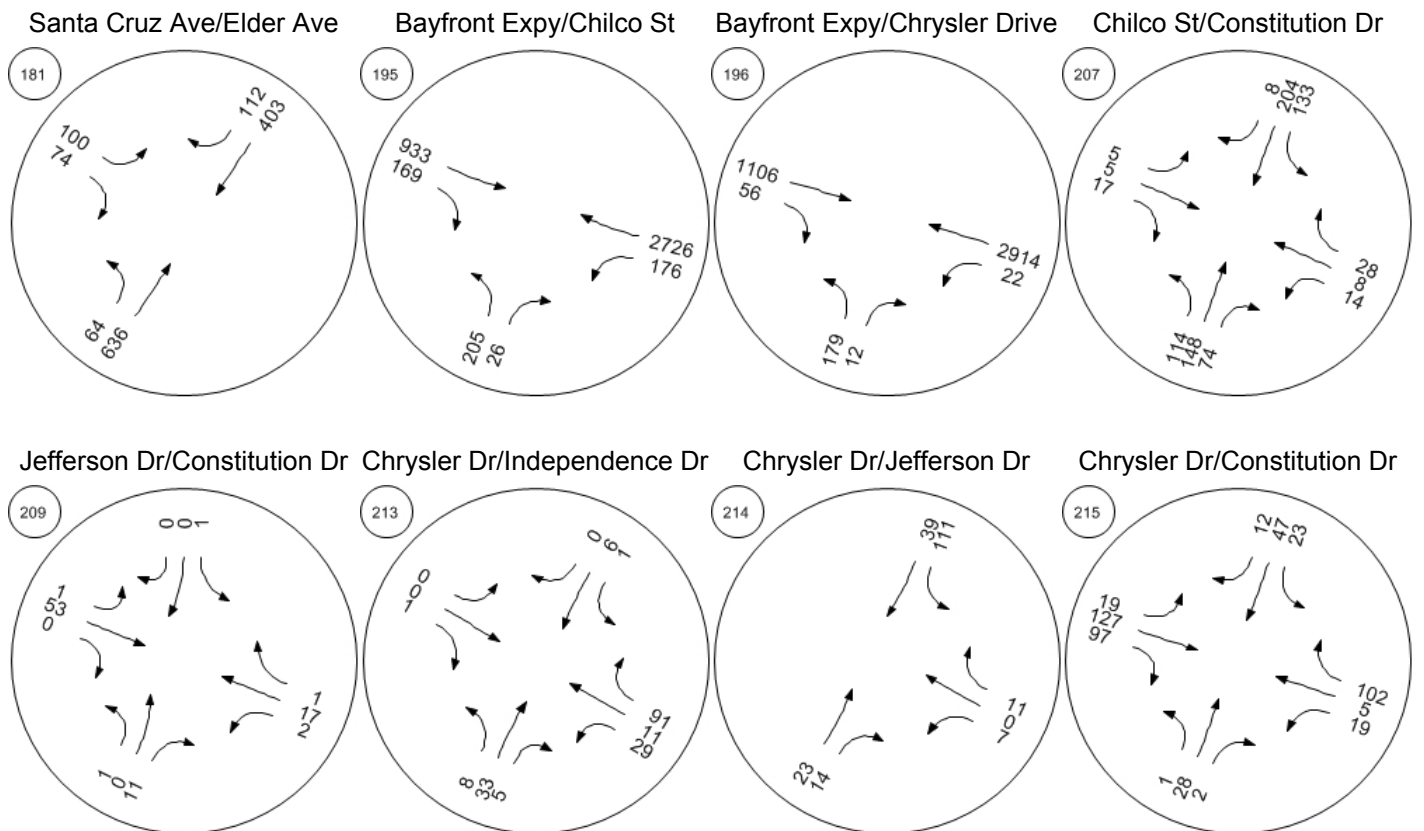
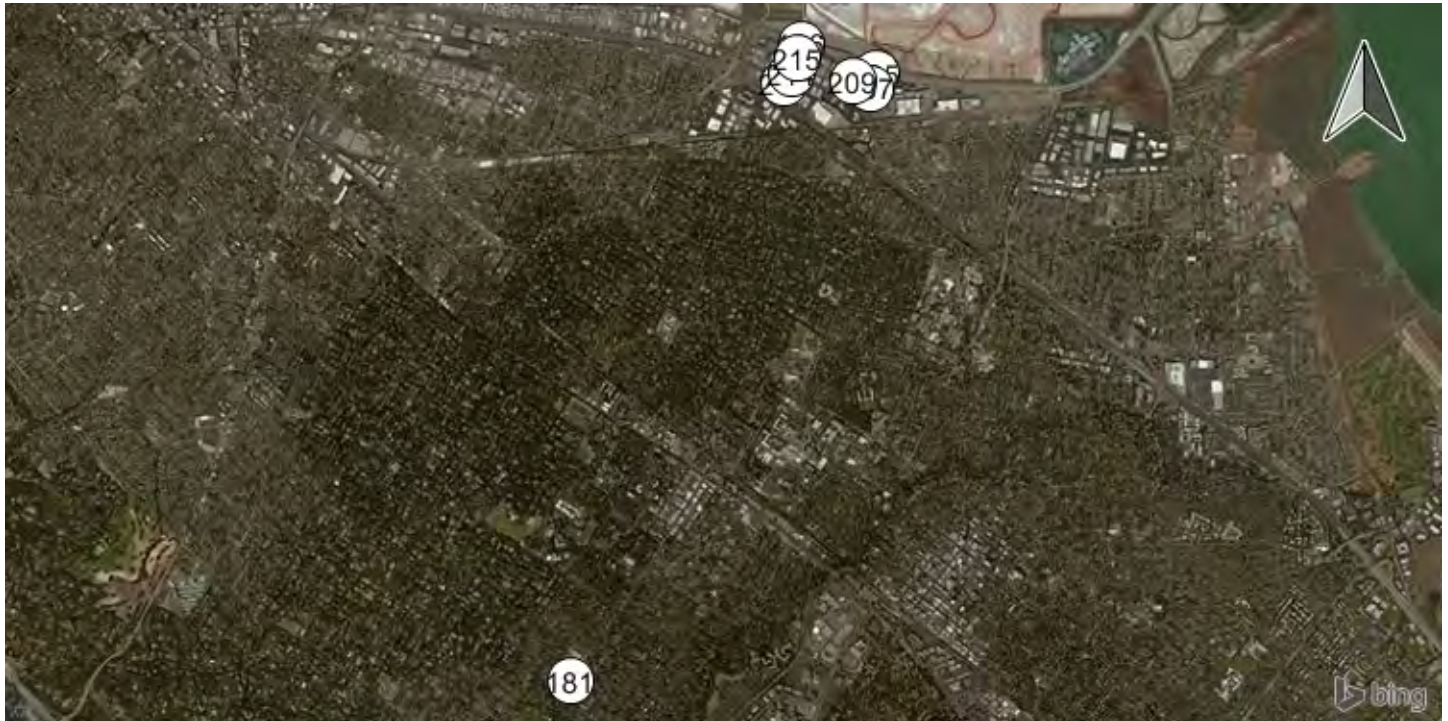
Branner Dr/Sand Hill Rd

Sharon Park Dr/ Sand Hill Rd

Bayfront Expy/Marsh Rd



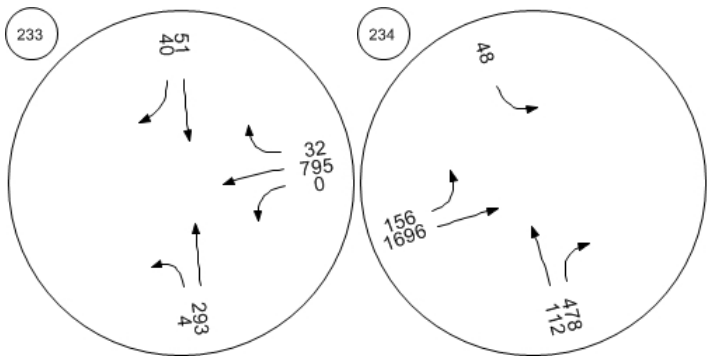
Traffic Volume - Base Volume



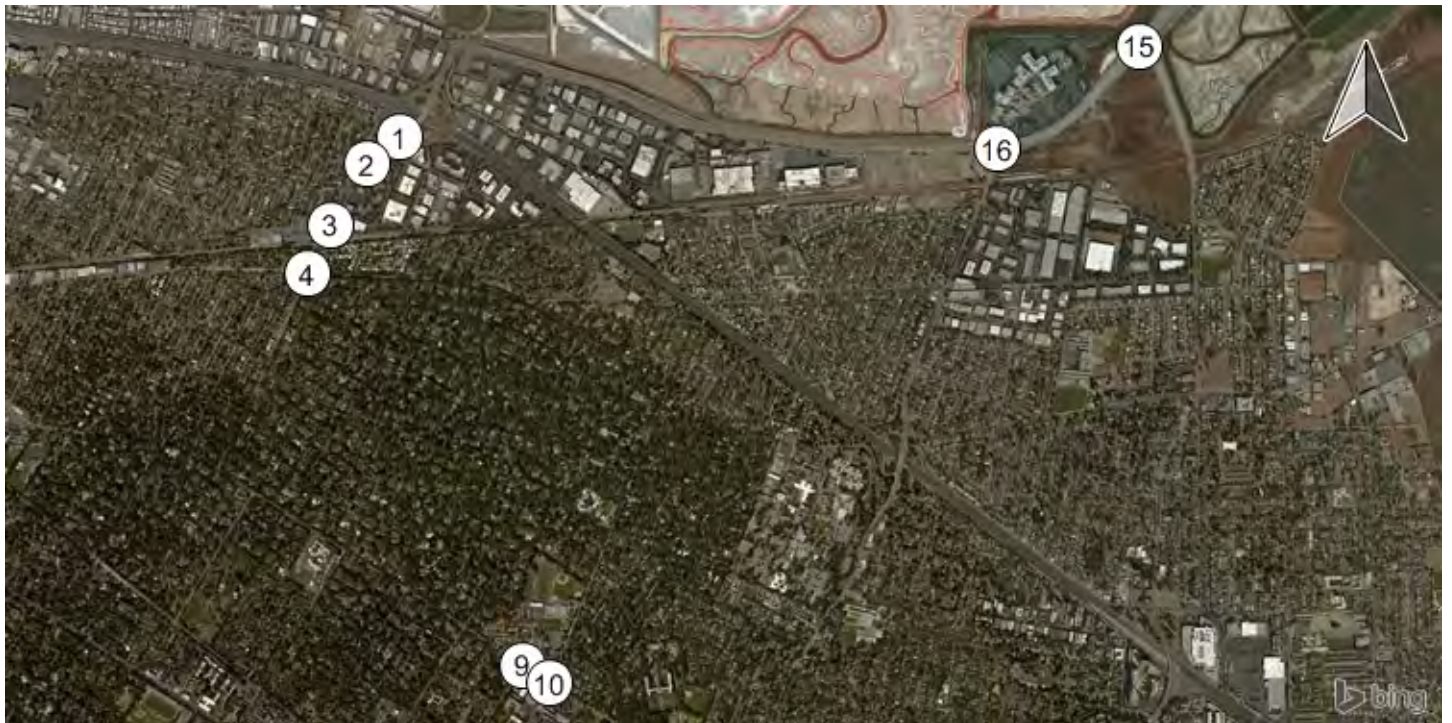
Traffic Volume - Base Volume



Sand Hill Road and Sand Hill Sand Hill Rd/Hwy 280 NB Off

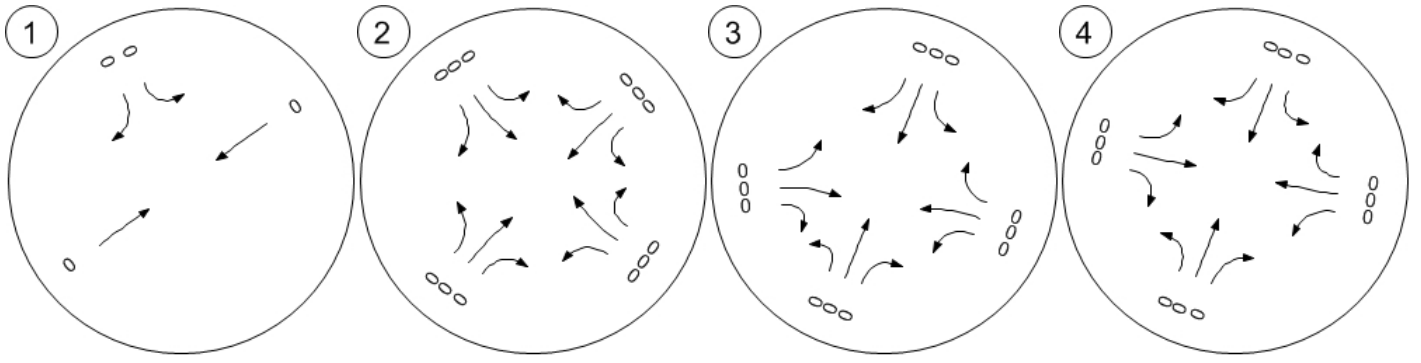


Traffic Volume - In-Process Volume



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

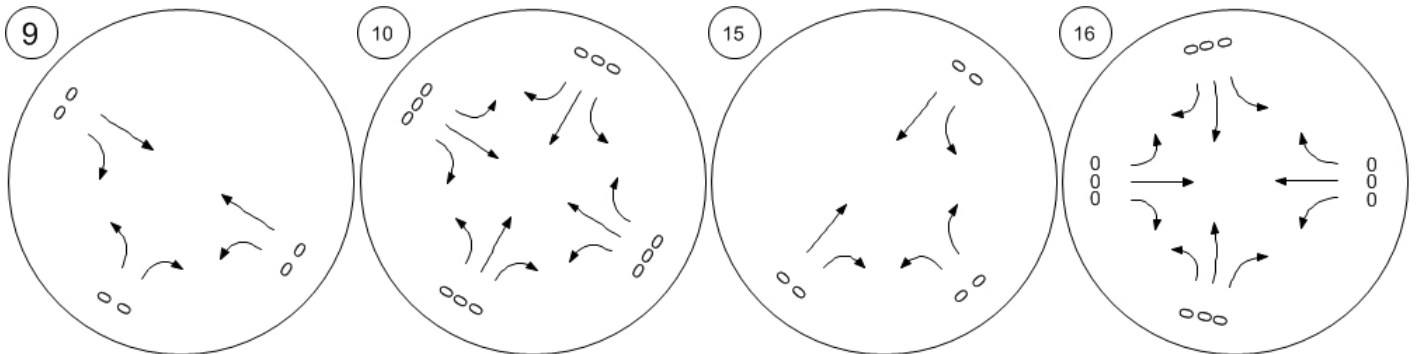


Middlefield Rd/Ravenswood

Middlefield Rd/Ringswood Av

Bayfront Expy (SR 84)/Univer

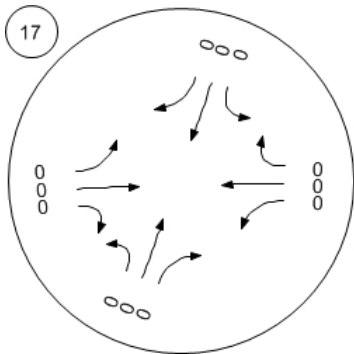
Bayfront Expy (SR 84)/Willow



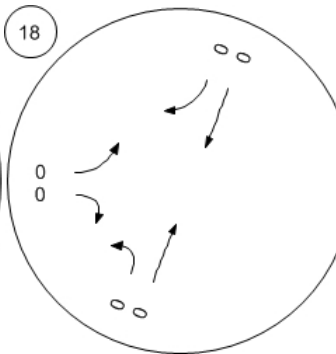
Traffic Volume - In-Process Volume



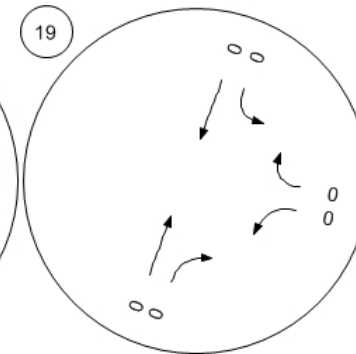
Willow Rd (SR 114)/Hamilton



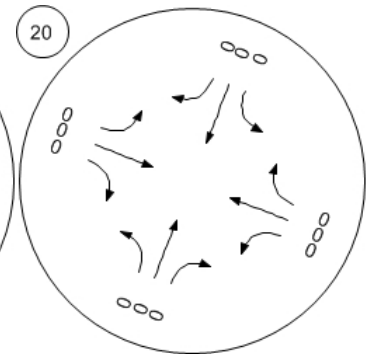
Willow Rd (SR 114)/Ivy Dr



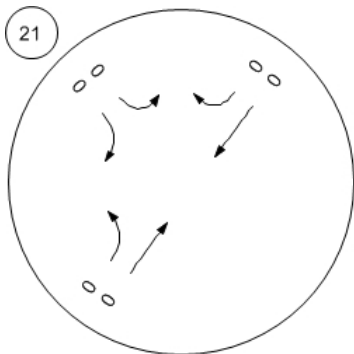
Willow Rd (SR 114)/O'Brien



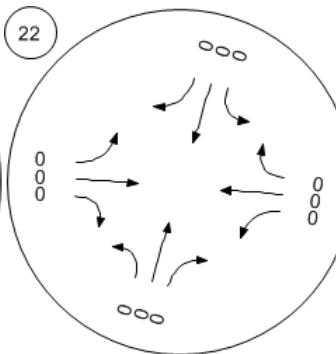
Willow Rd (SR 114)/Newbrid



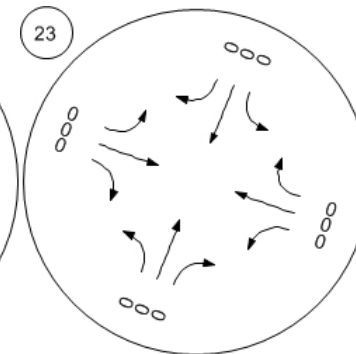
Willow Rd/Bay Rd



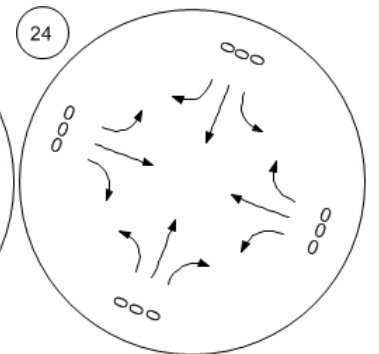
Willow Rd/Durham St-VA Me



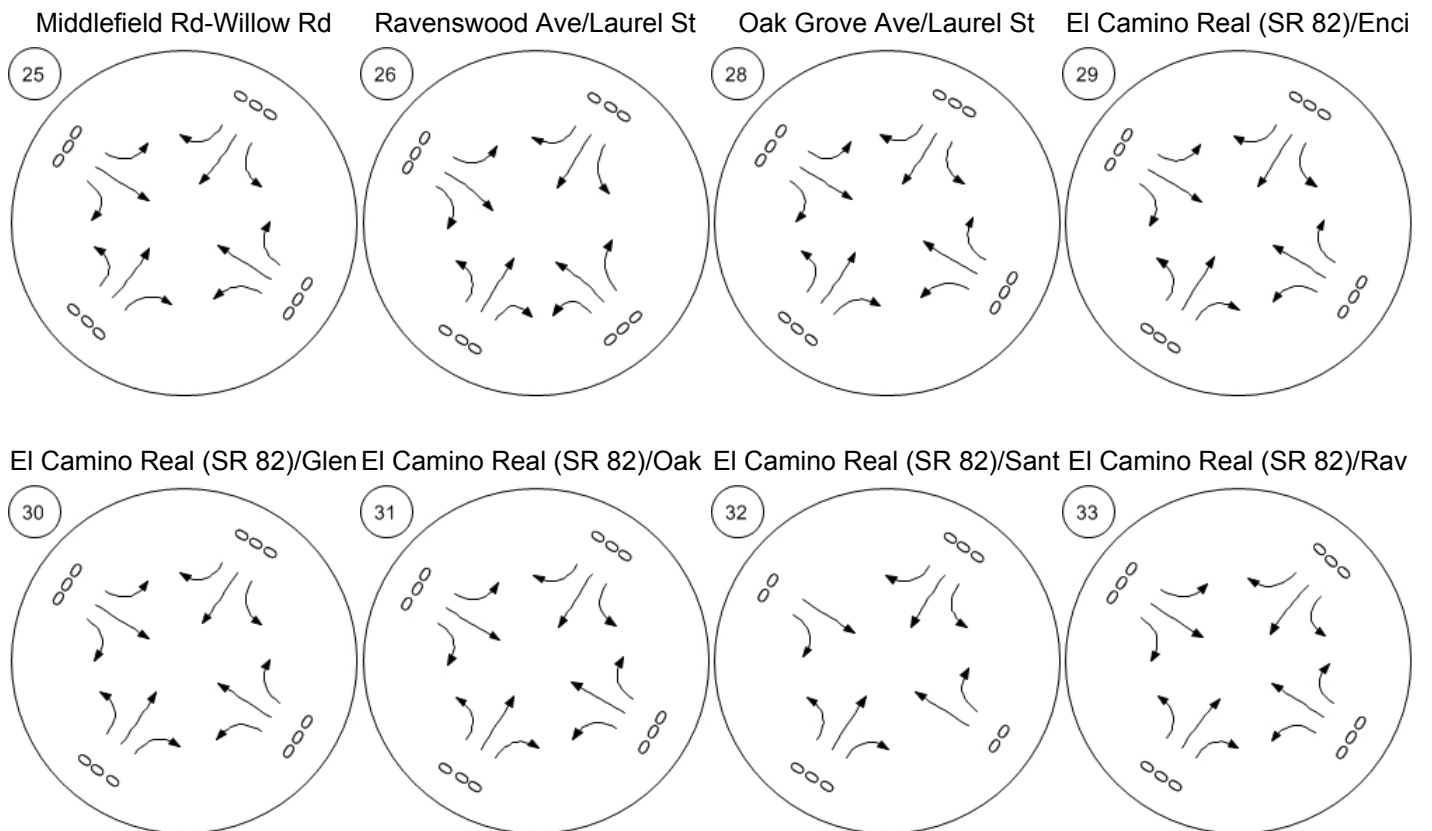
Willow Rd/Coleman Ave



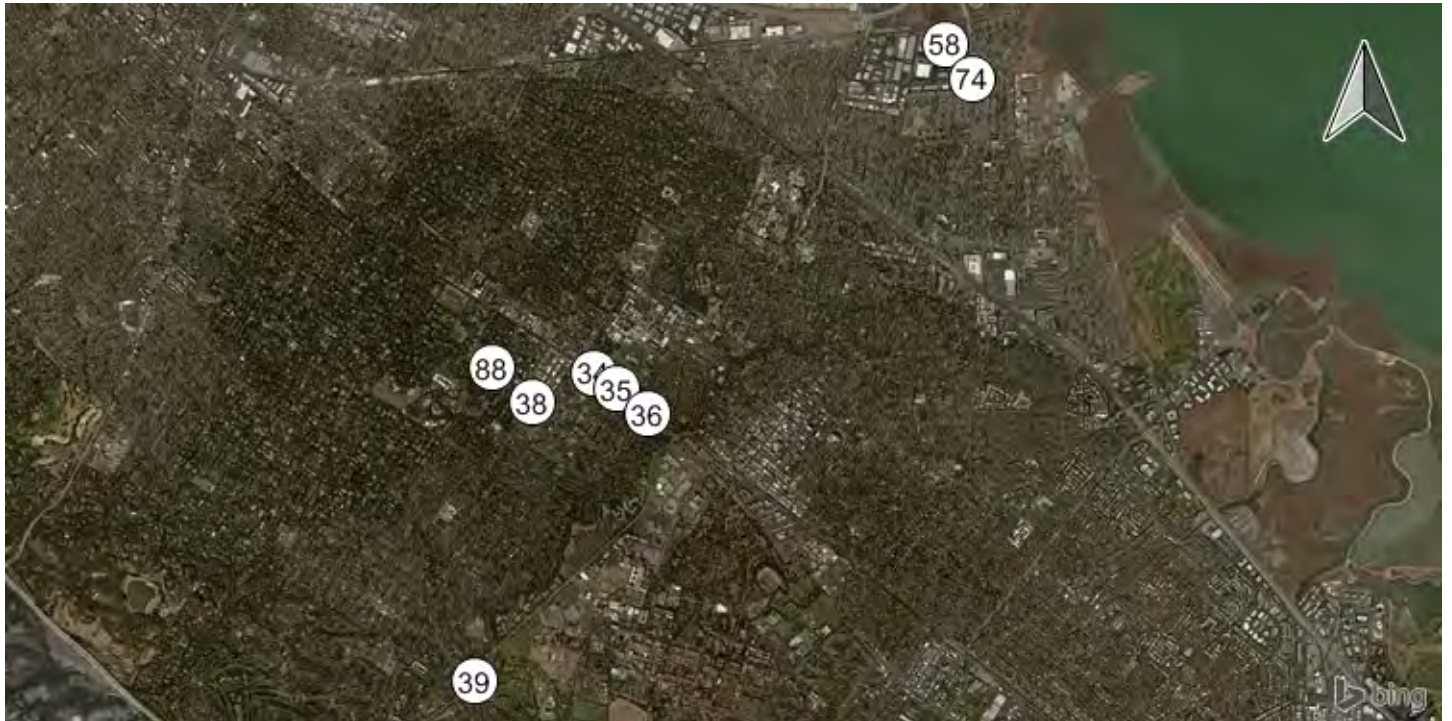
Willow Rd/Gilbert Ave



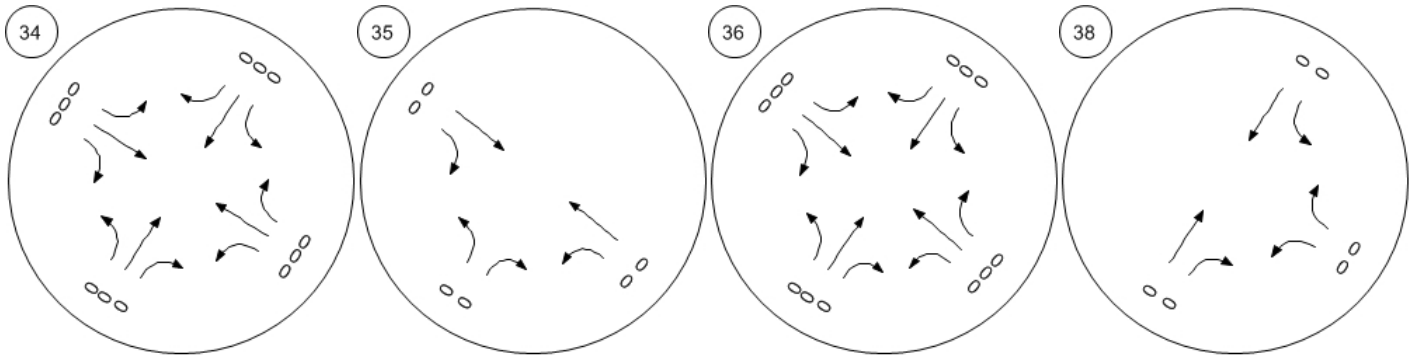
Traffic Volume - In-Process Volume



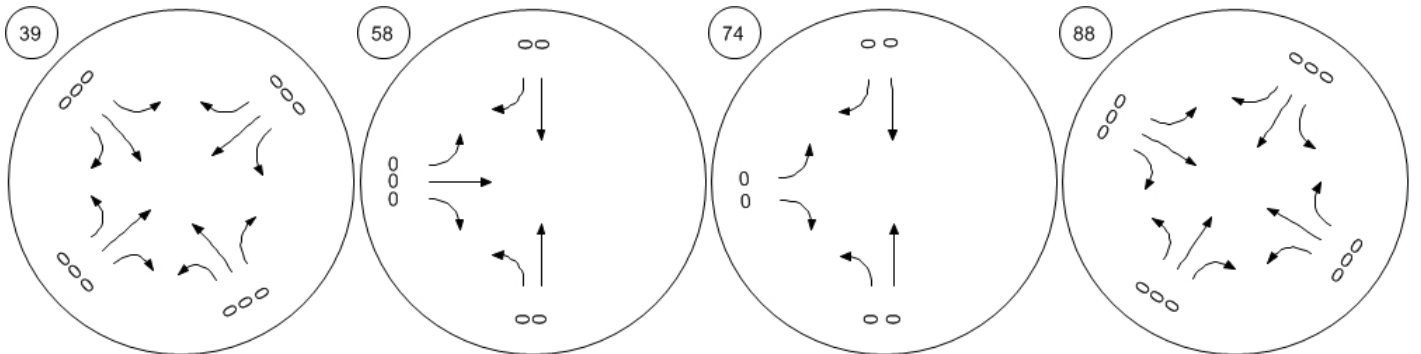
Traffic Volume - In-Process Volume



El Camino Real (SR 82)/Robl El Camino Real (SR 82)/Middle El Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



Santa Cruz Ave/Sand Hill Rd University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr

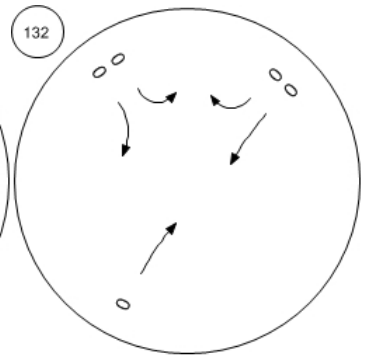
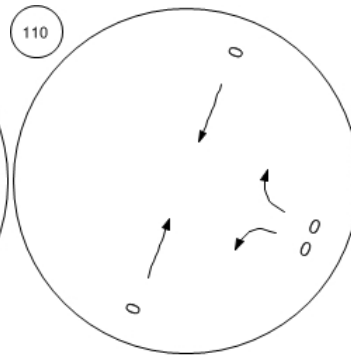
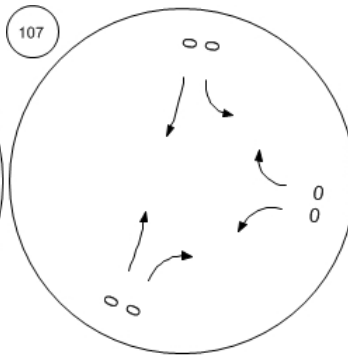
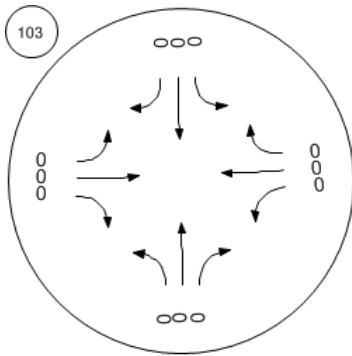


Traffic Volume - In-Process Volume



Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road and US 101 NB

Oak Ave/Sand Hill Rd

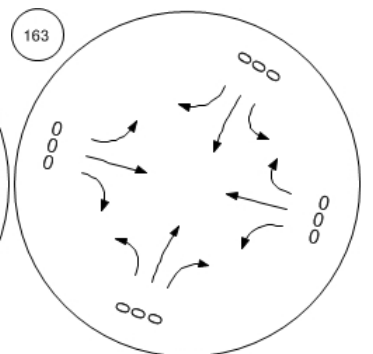
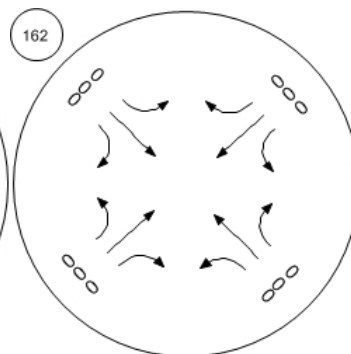
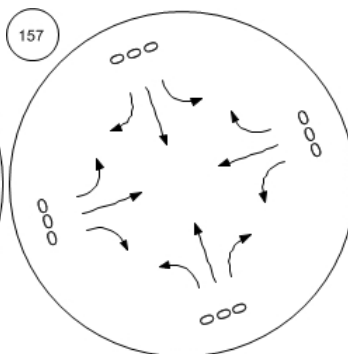
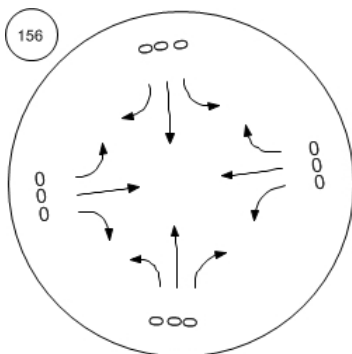


Saga Ln/Sand Hill Rd

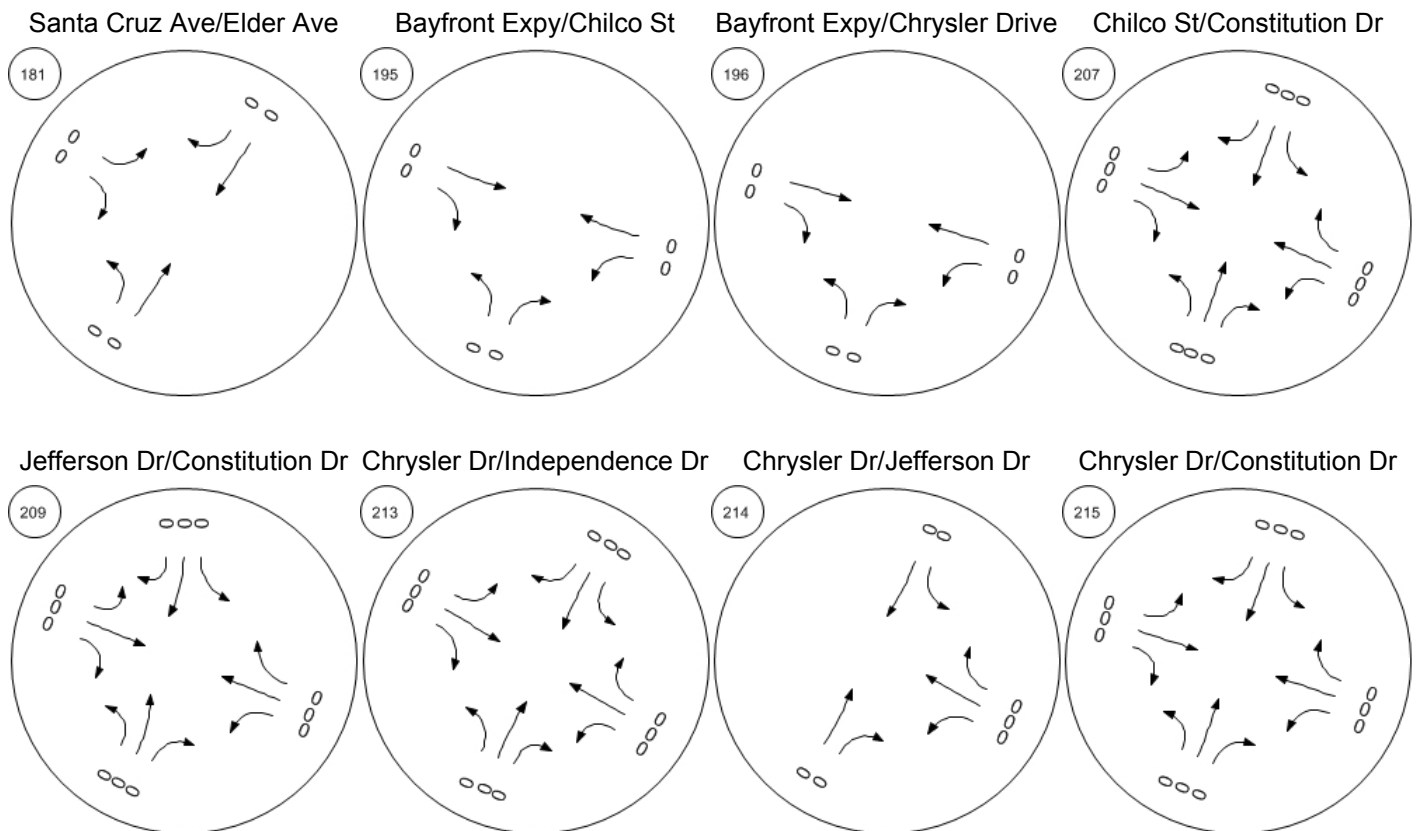
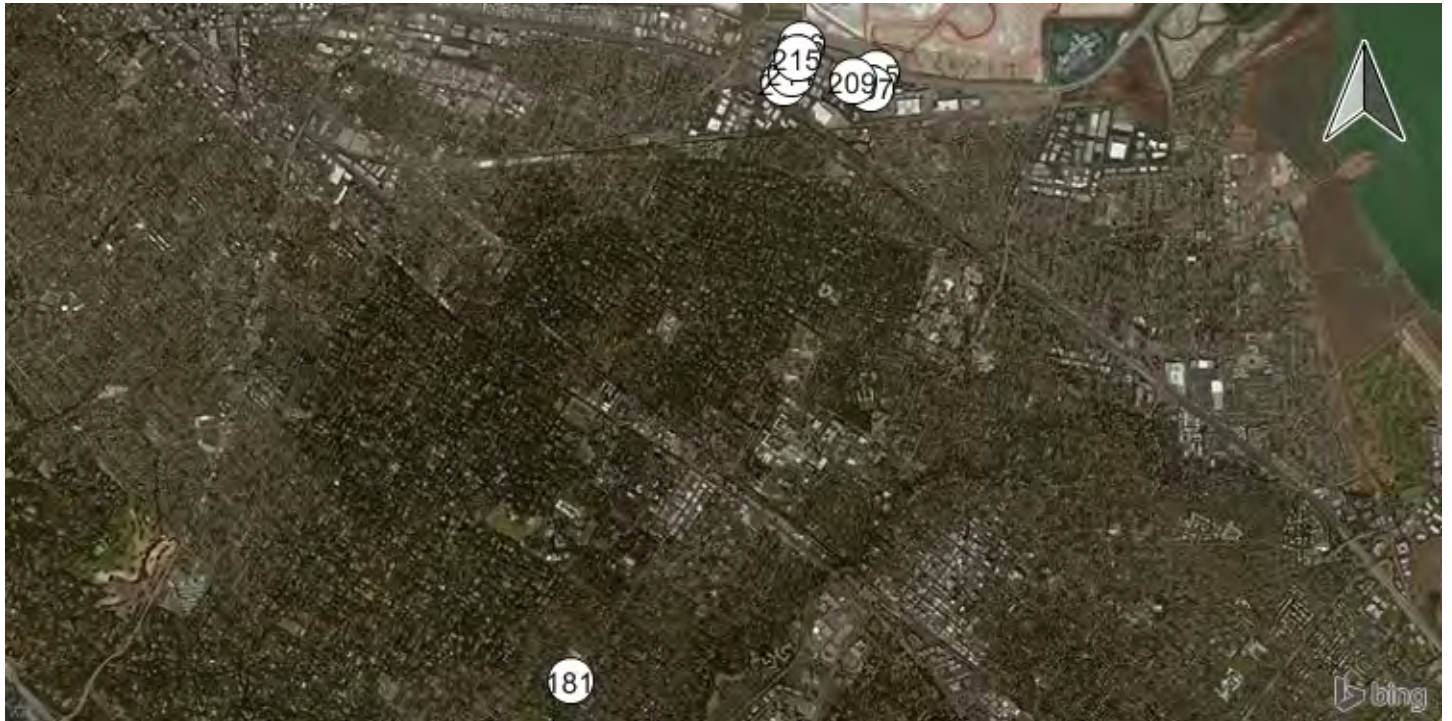
Branner Dr/Sand Hill Rd

Sharon Park Dr/ Sand Hill Rd

Bayfront Expy/Marsh Rd



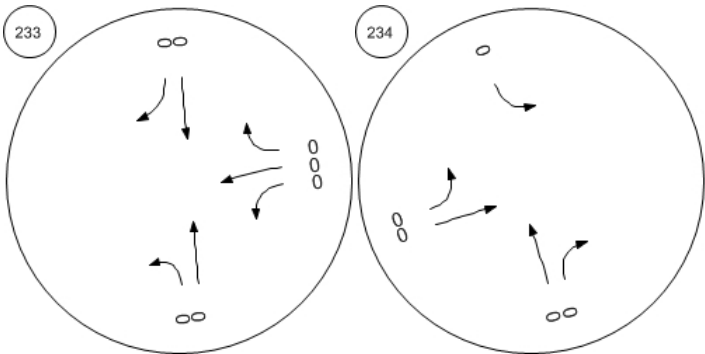
Traffic Volume - In-Process Volume



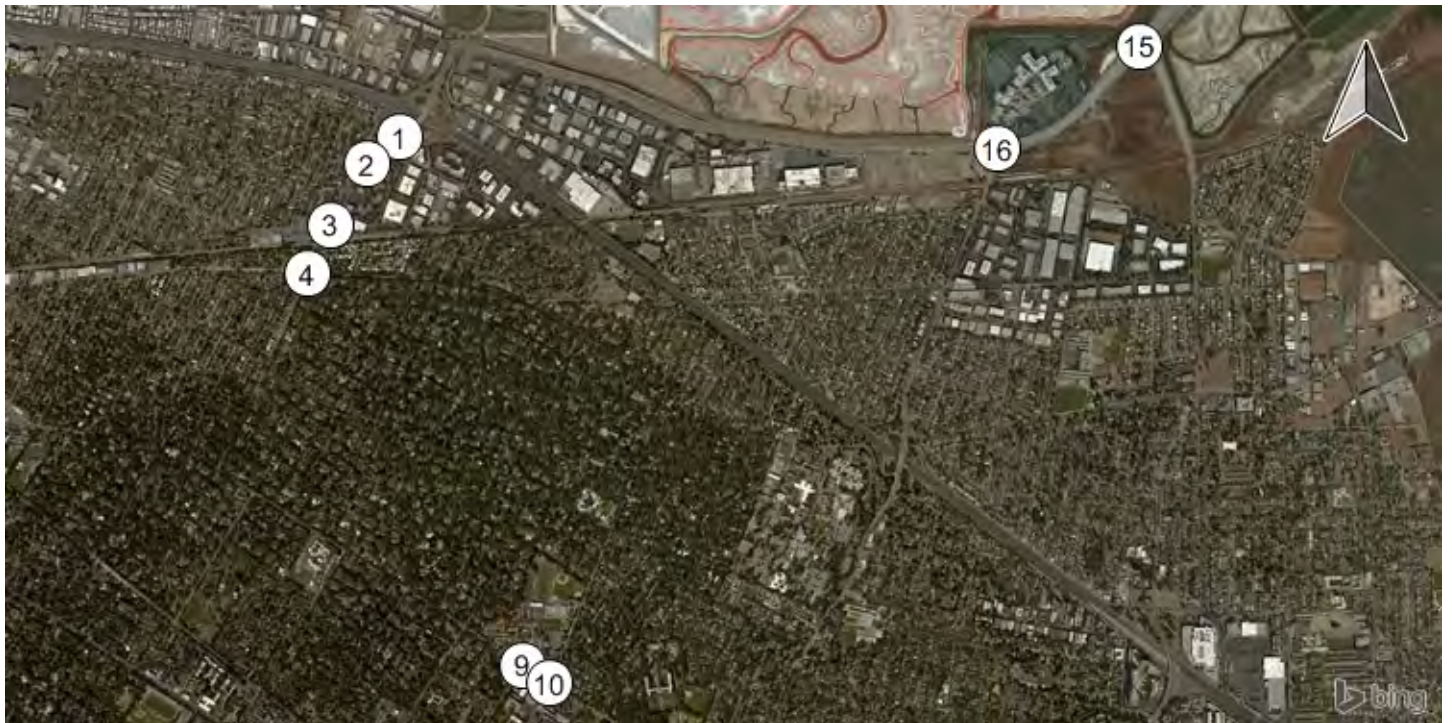
Traffic Volume - In-Process Volume



Sand Hill Road and Sand Hill Sand Hill Rd/Hwy 280 NB Off

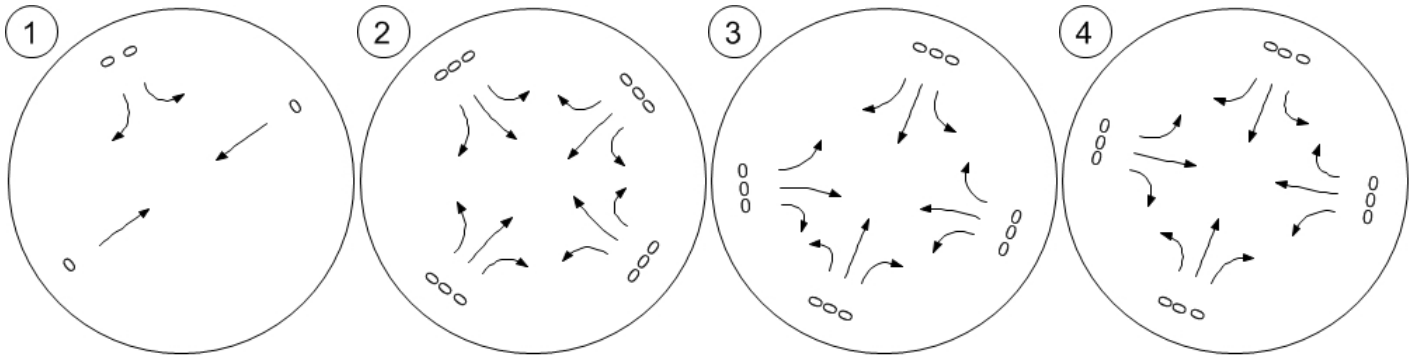


Traffic Volume - Net New Site Trips



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

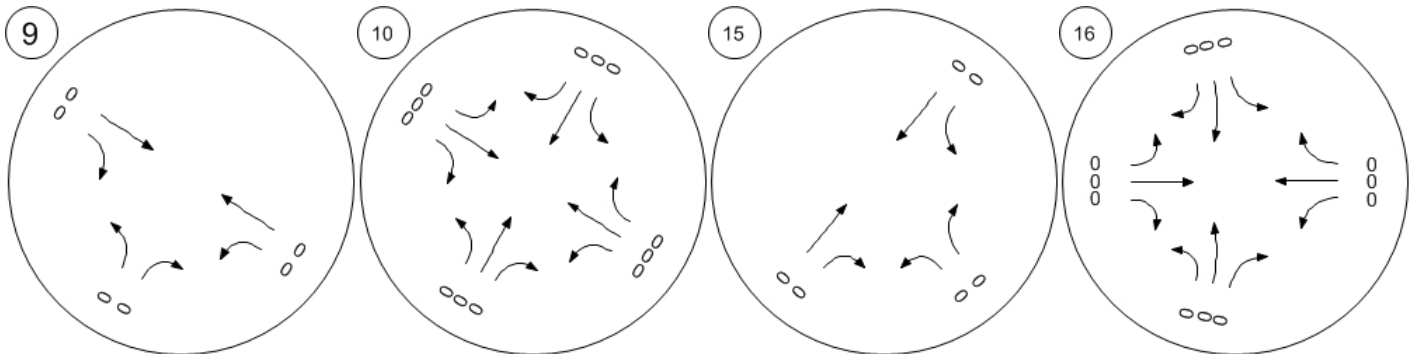


Middlefield Rd/Ravenswood

Middlefield Rd/Ringswood Av

Bayfront Expy (SR 84)/Univer

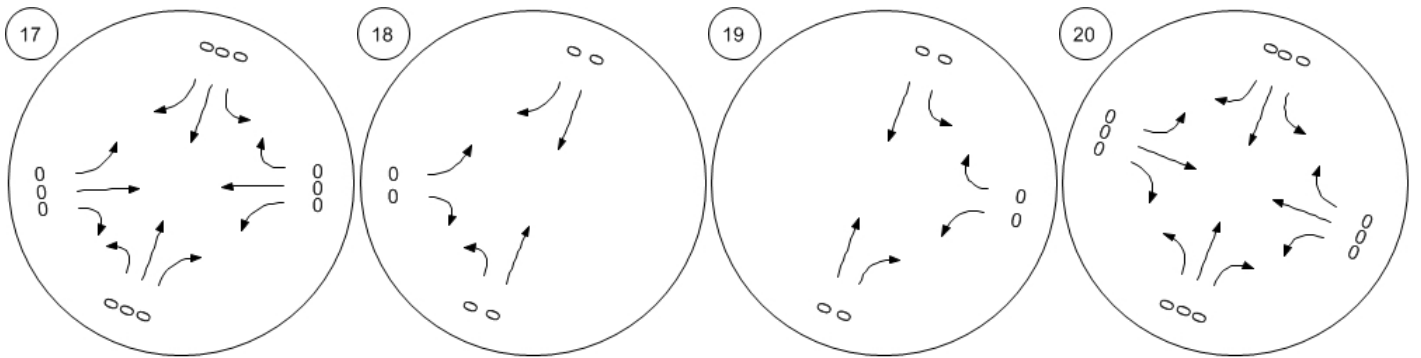
Bayfront Expy (SR 84)/Willow



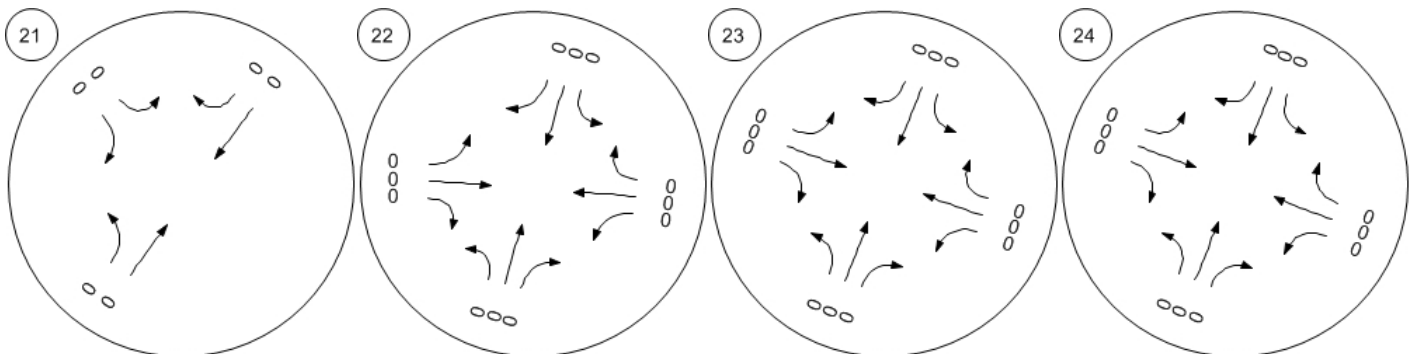
Traffic Volume - Net New Site Trips



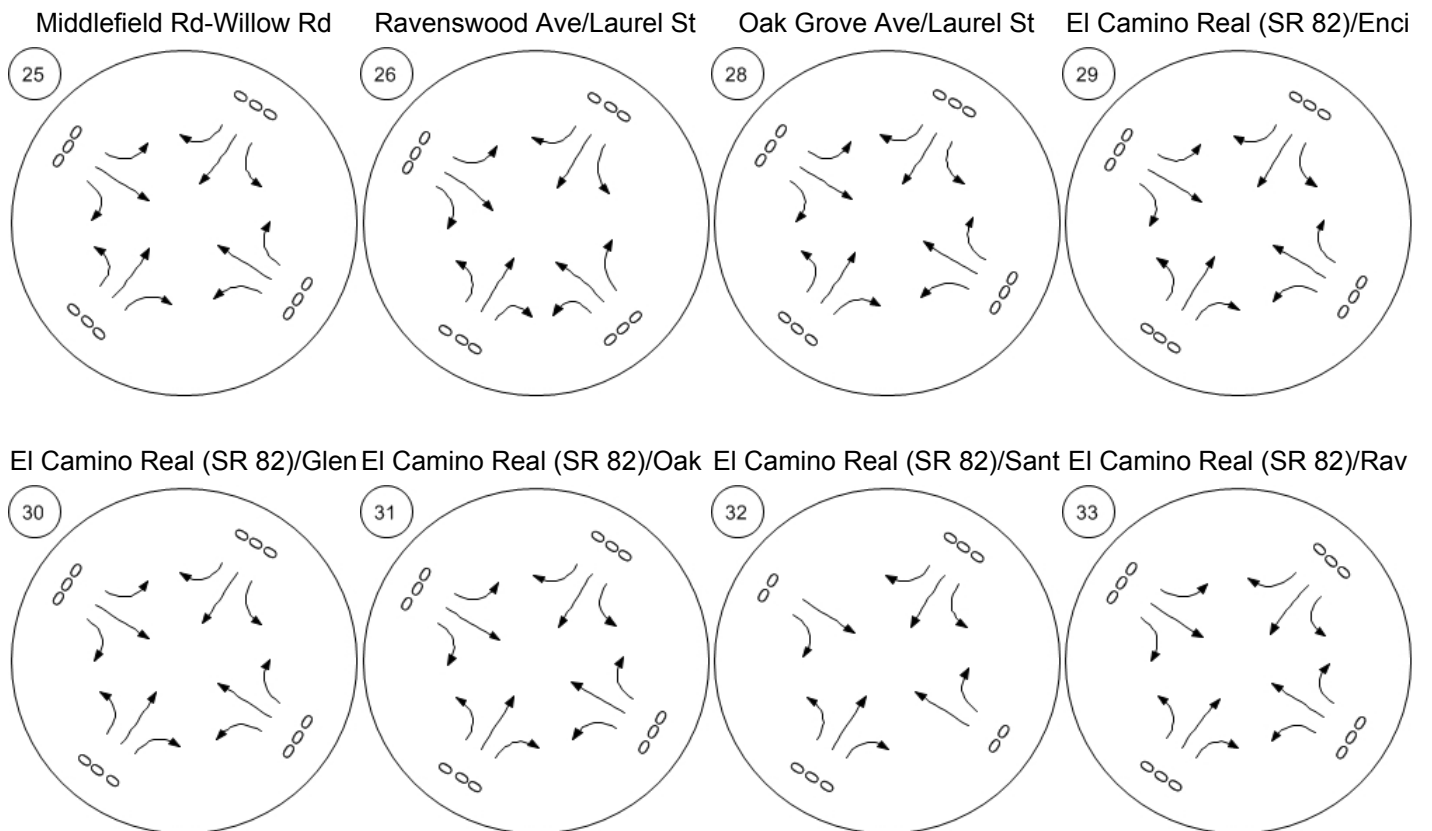
Willow Rd (SR 114)/Hamilton Willow Rd (SR 114)/Ivy Dr Willow Rd (SR 114)/O'Brien Willow Rd (SR 114)/Newbrid



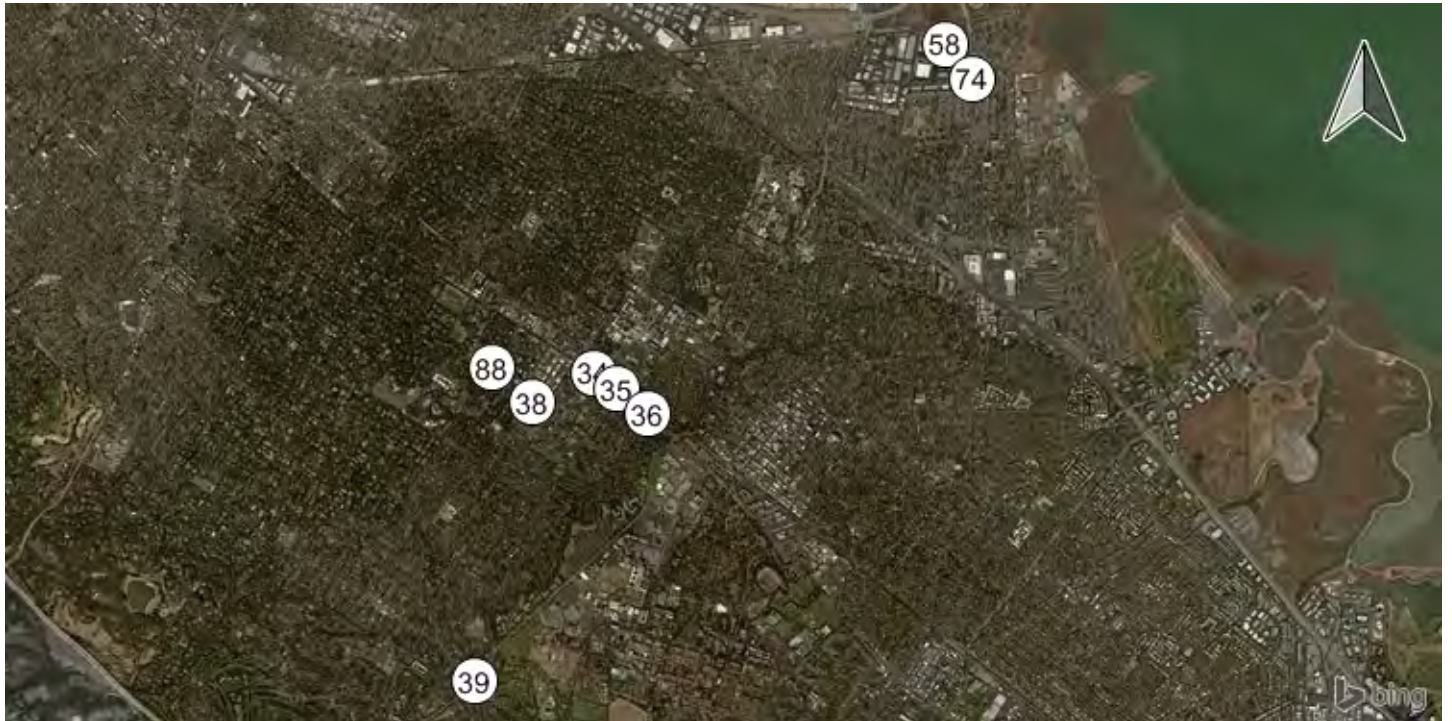
Willow Rd/Bay Rd Willow Rd/Durham St-VA Me Willow Rd/Coleman Ave Willow Rd/Gilbert Ave



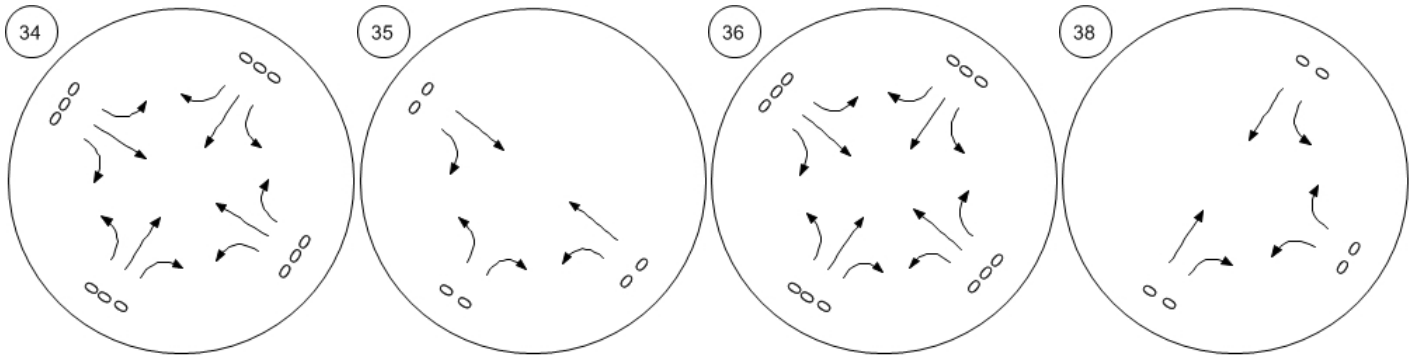
Traffic Volume - Net New Site Trips



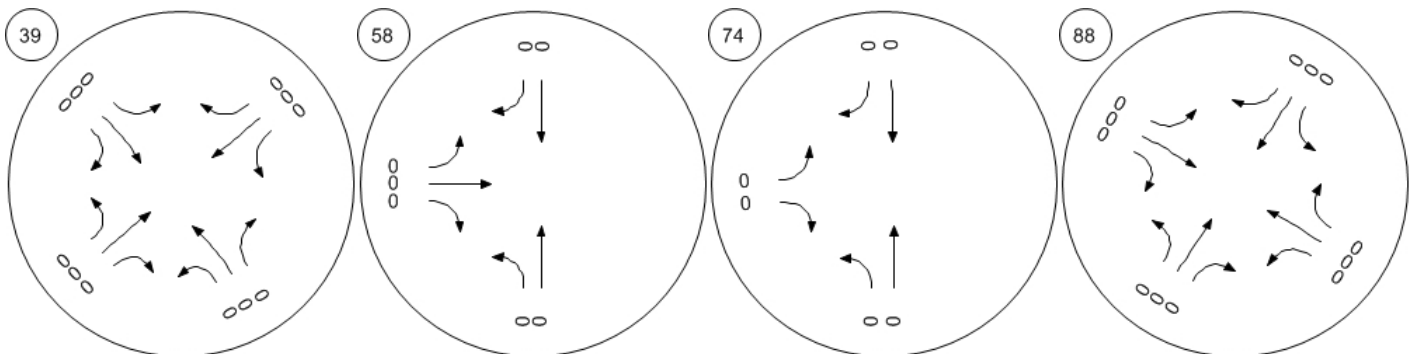
Traffic Volume - Net New Site Trips



El Camino Real (SR 82)/Robl El Camino Real (SR 82)/Middle El Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



Santa Cruz Ave/Sand Hill Rd University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr

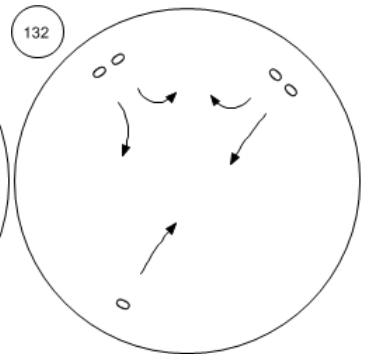
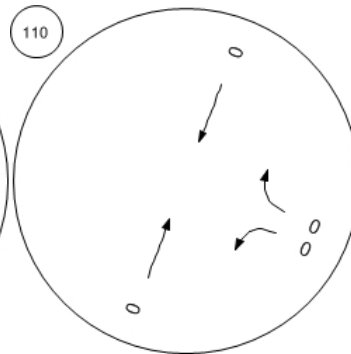
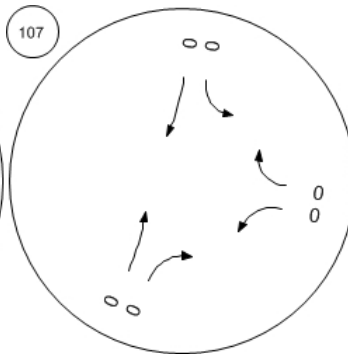
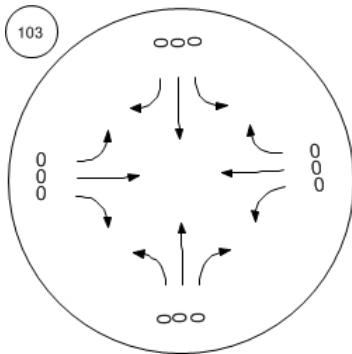


Traffic Volume - Net New Site Trips



Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road and US 101 NB

Oak Ave/Sand Hill Rd

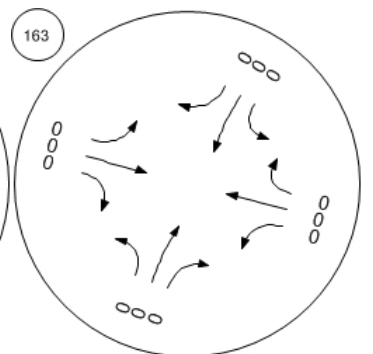
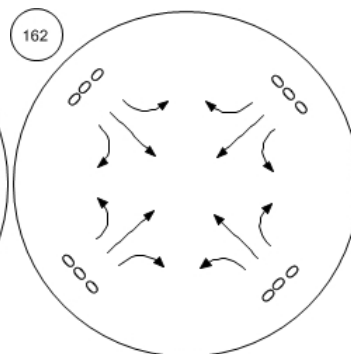
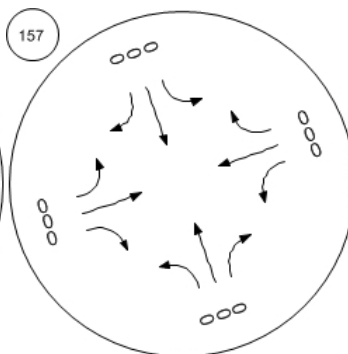
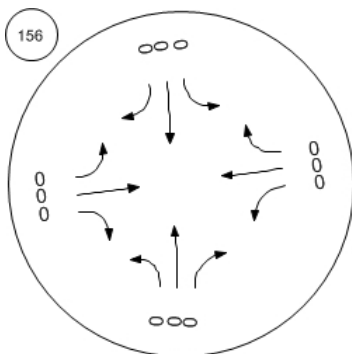


Saga Ln/Sand Hill Rd

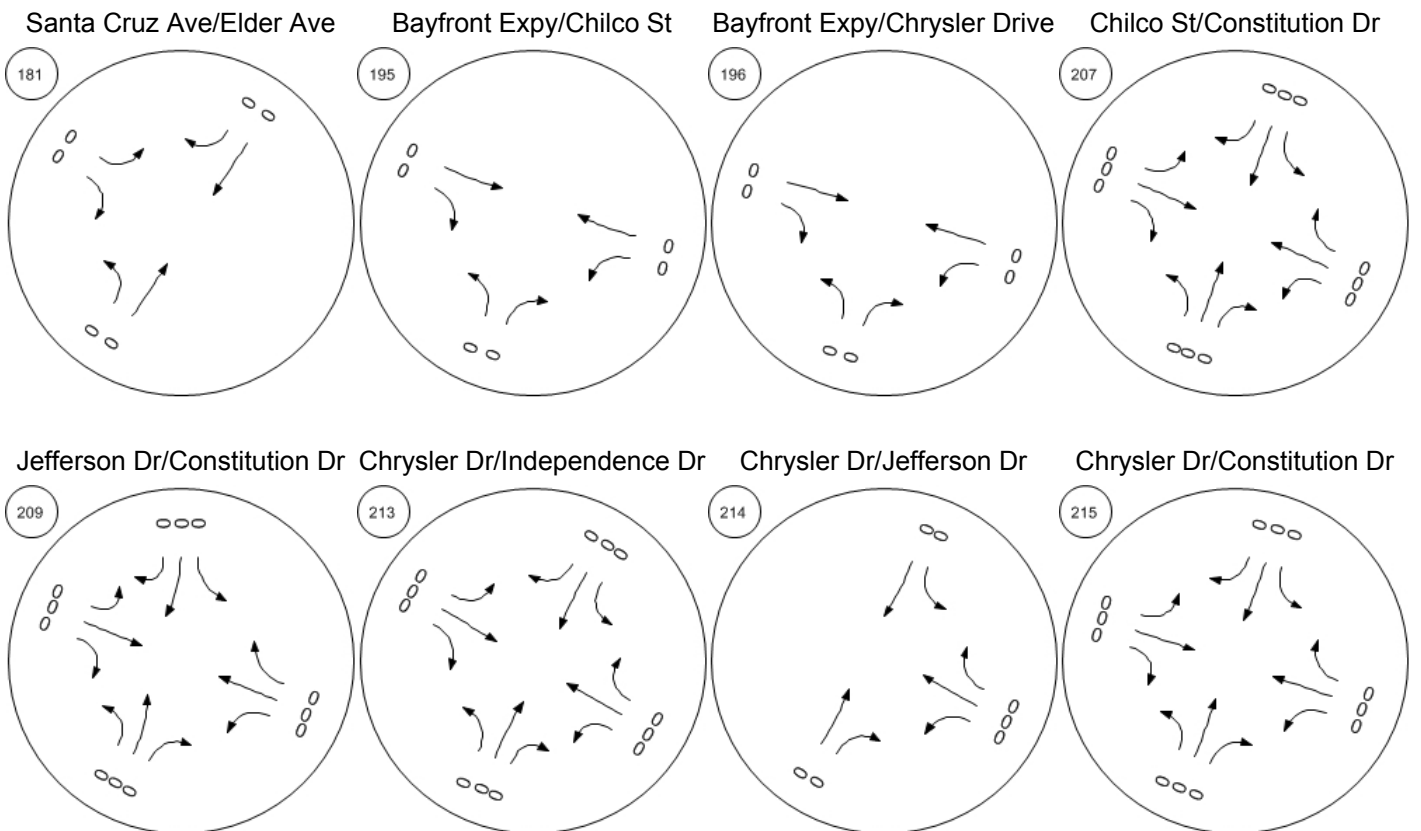
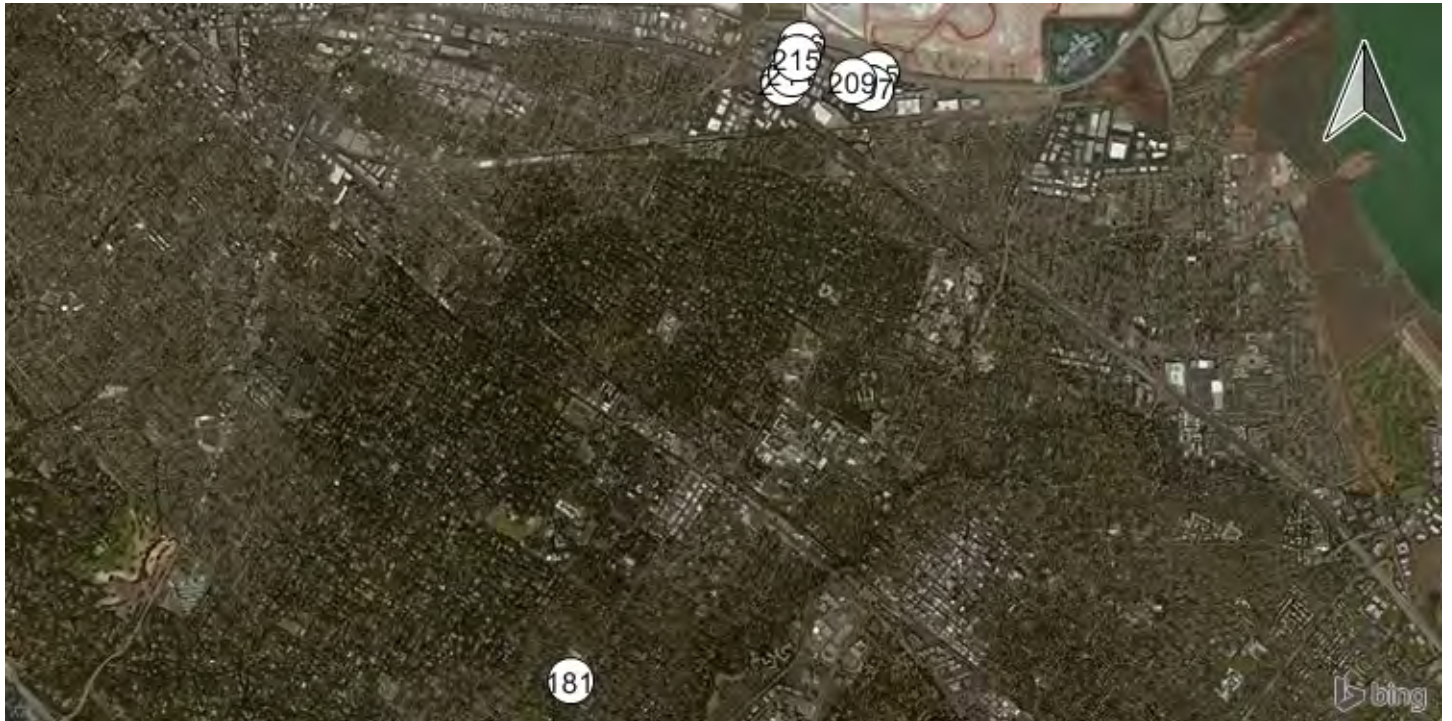
Branner Dr/Sand Hill Rd

Sharon Park Dr/ Sand Hill Rd

Bayfront Expy/Marsh Rd



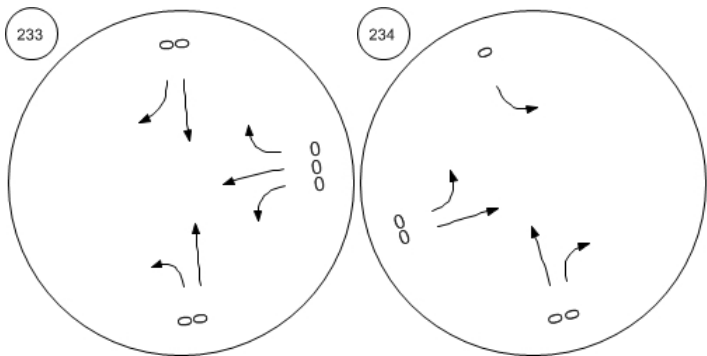
Traffic Volume - Net New Site Trips



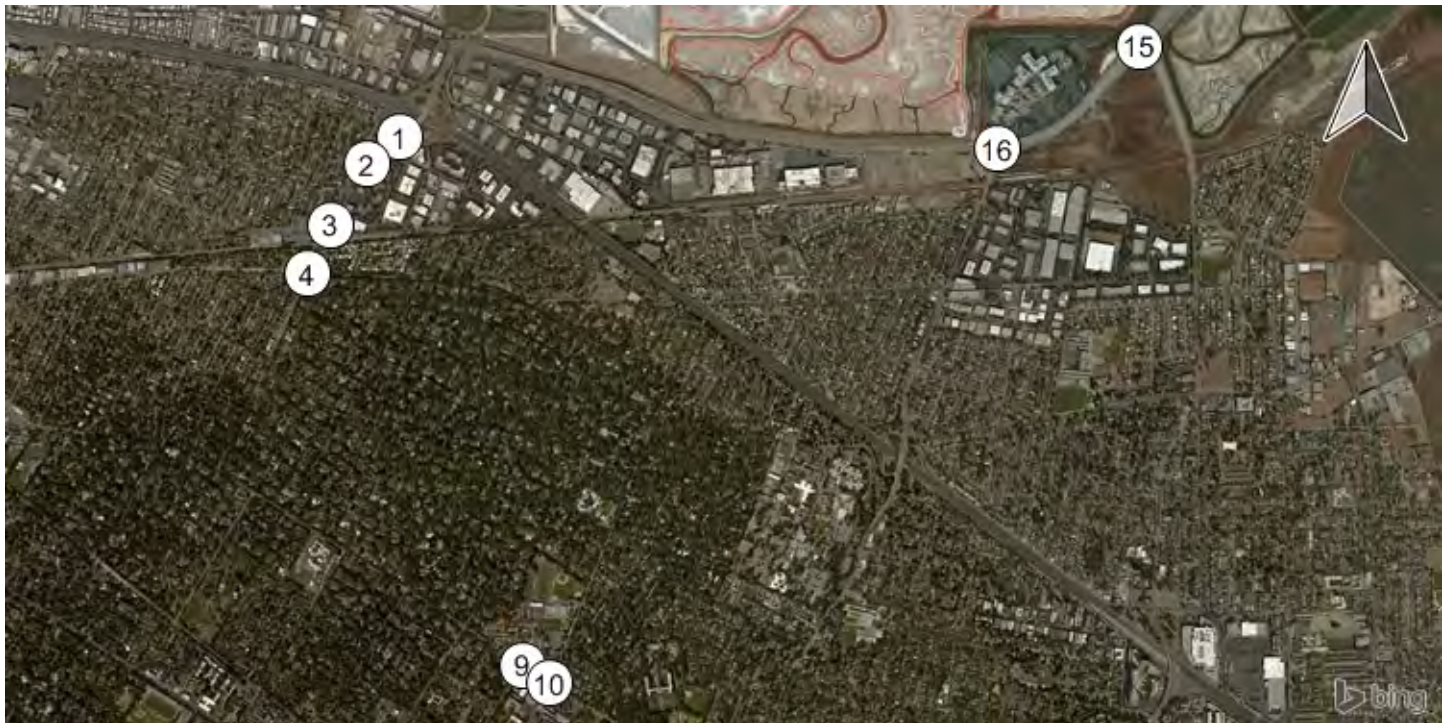
Traffic Volume - Net New Site Trips



Sand Hill Road and Sand Hill Sand Hill Rd/Hwy 280 NB Off

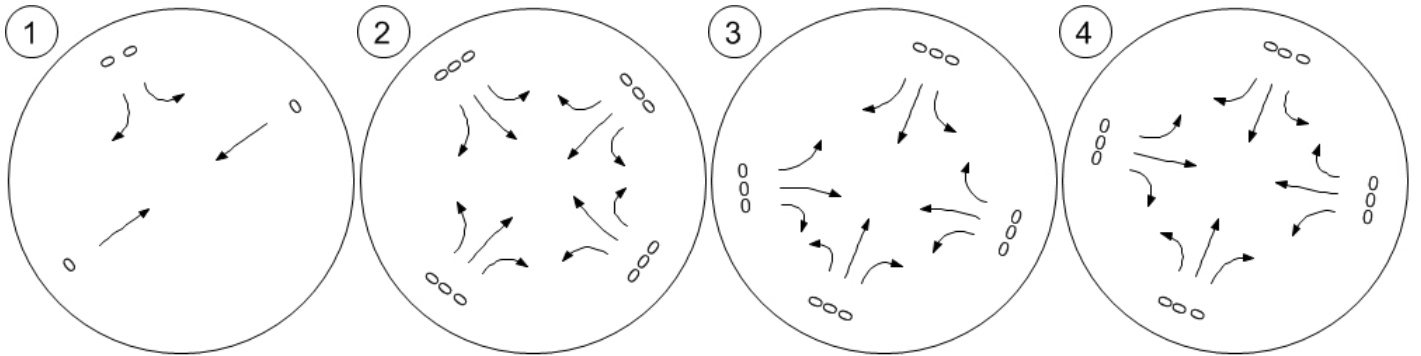


Traffic Volume - Other Volume



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

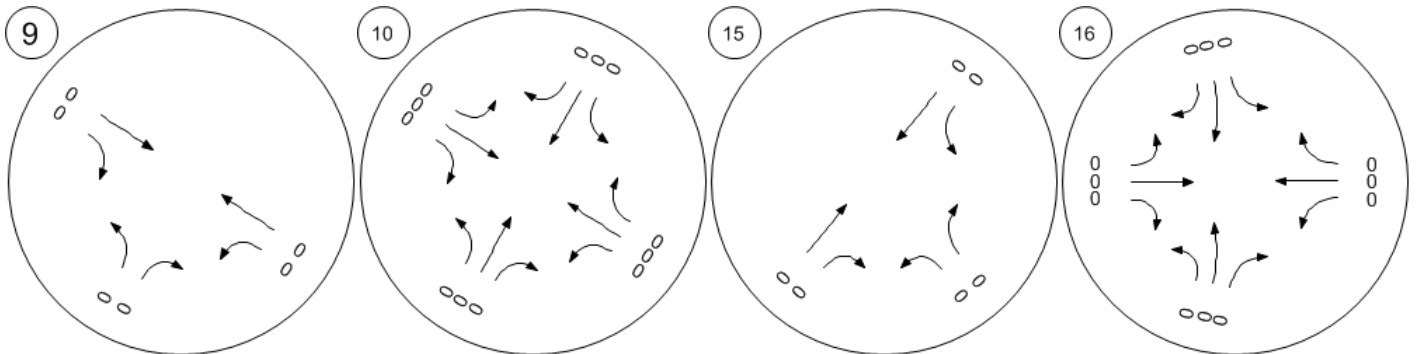


Middlefield Rd/Ravenswood

Middlefield Rd/Ringswood Av

Bayfront Expy (SR 84)/Univer

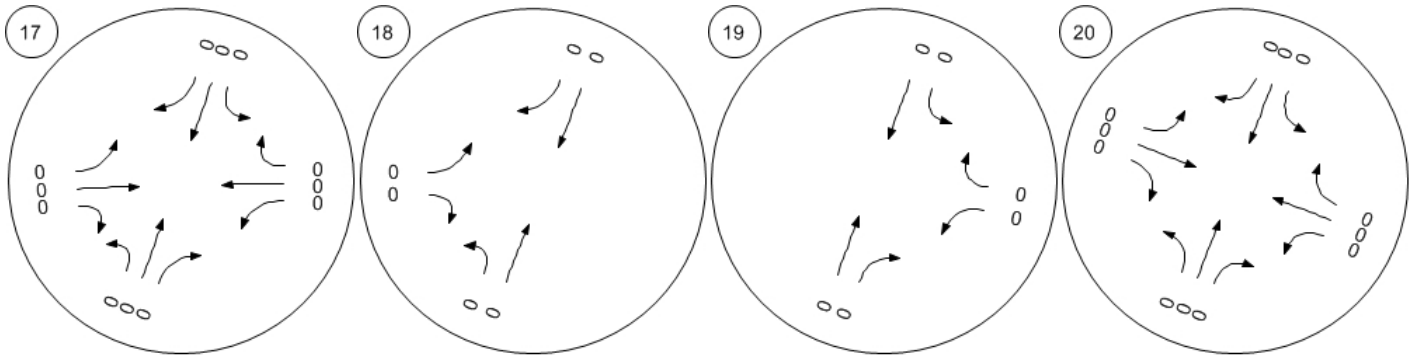
Bayfront Expy (SR 84)/Willow



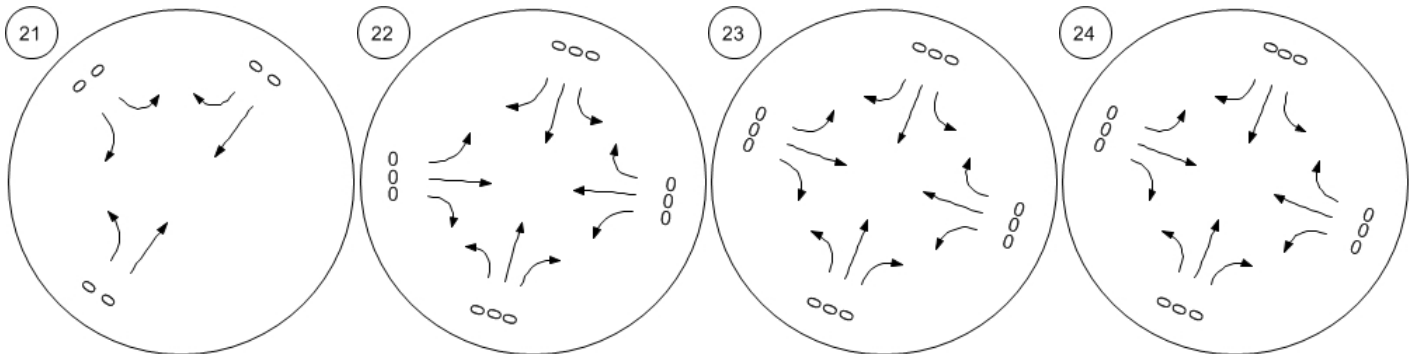
Traffic Volume - Other Volume



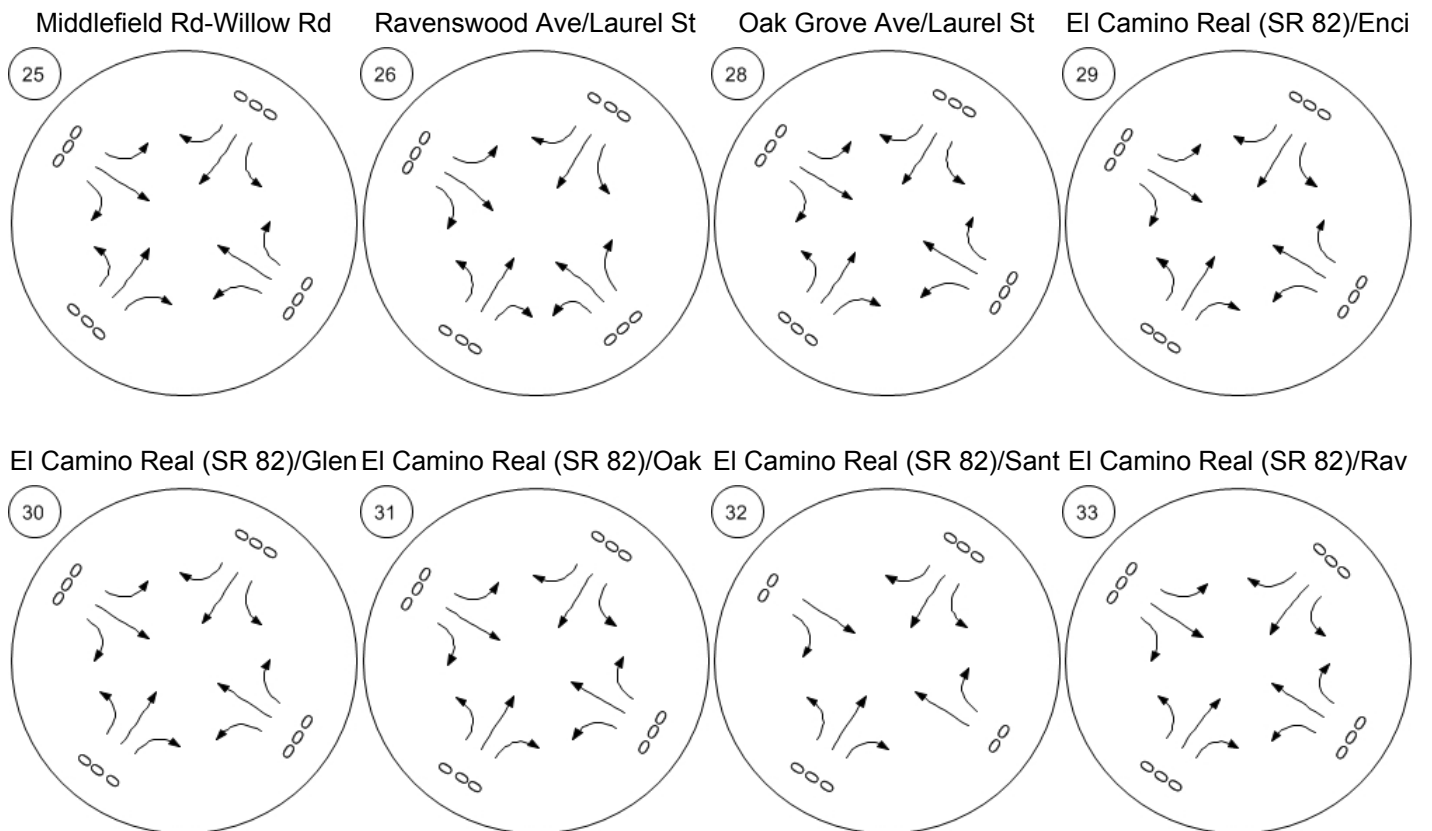
Willow Rd (SR 114)/Hamilton Willow Rd (SR 114)/Ivy Dr Willow Rd (SR 114)/O'Brien Willow Rd (SR 114)/Newbrid



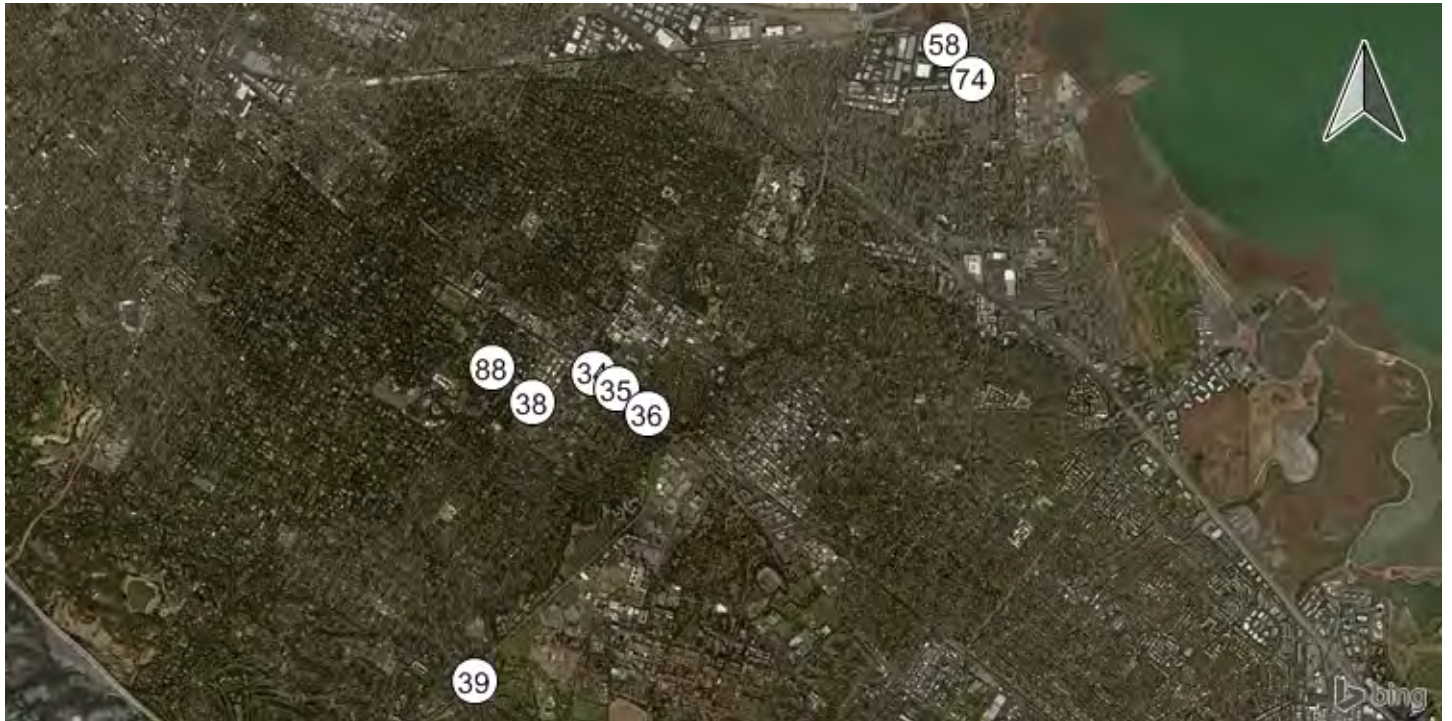
Willow Rd/Bay Rd Willow Rd/Durham St-VA Me Willow Rd/Coleman Ave Willow Rd/Gilbert Ave



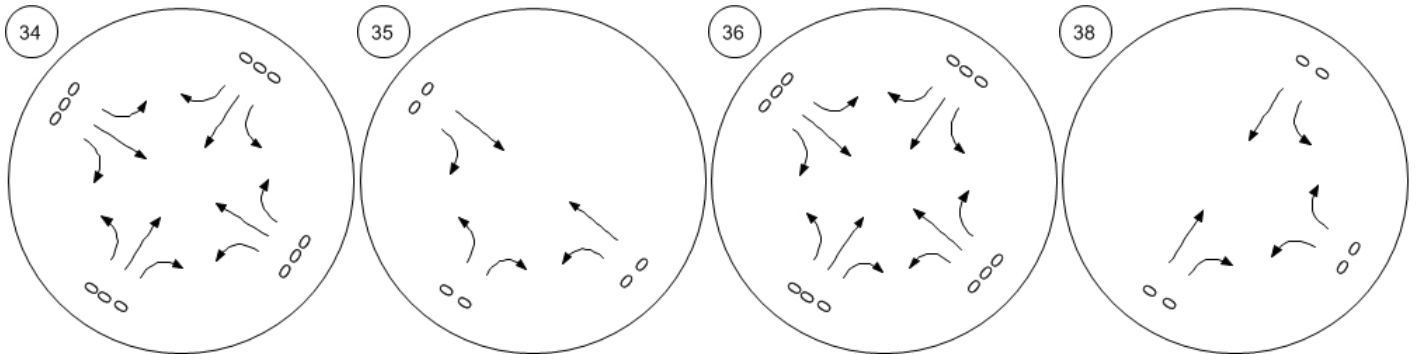
Traffic Volume - Other Volume



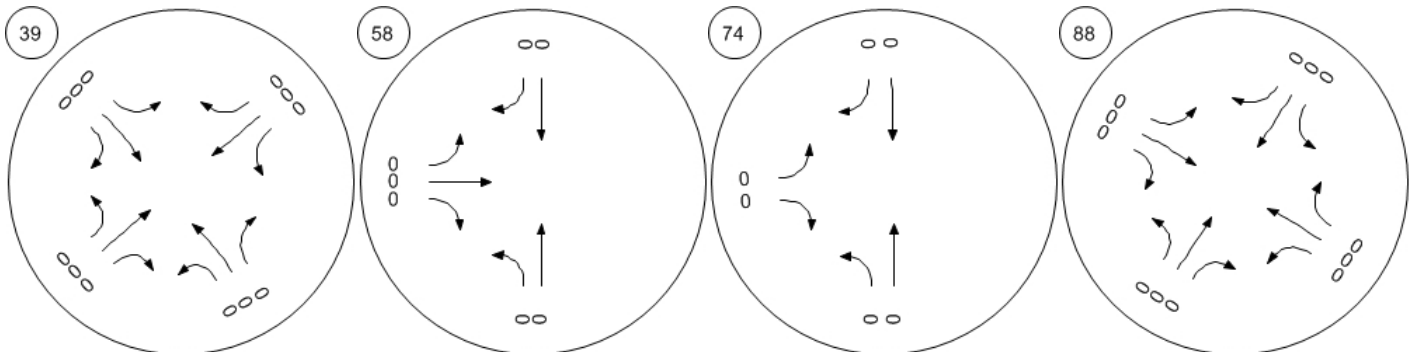
Traffic Volume - Other Volume



El Camino Real (SR 82)/Robl El Camino Real (SR 82)/Middle El Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



Santa Cruz Ave/Sand Hill Rd University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr

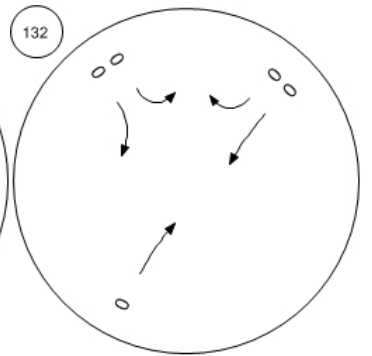
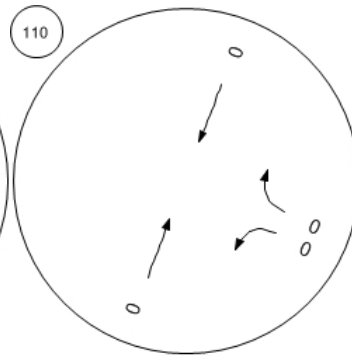
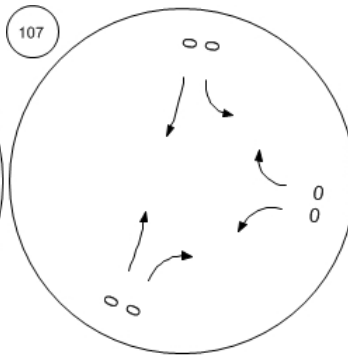
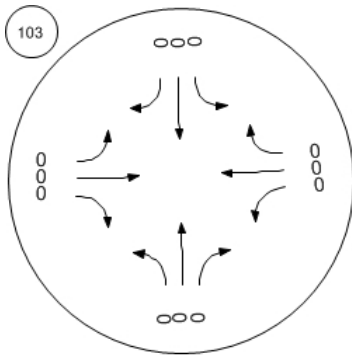


Traffic Volume - Other Volume



Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road and US 101 NB

Oak Ave/Sand Hill Rd

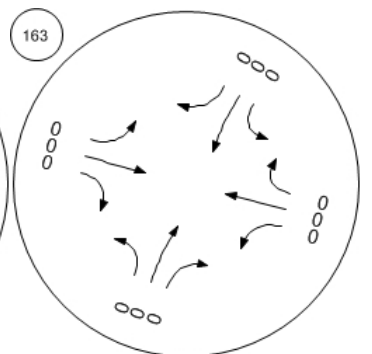
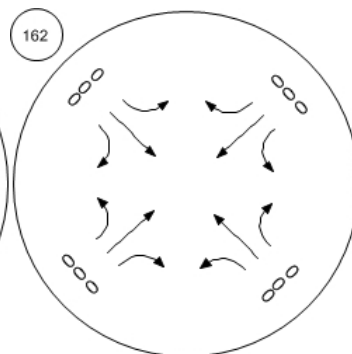
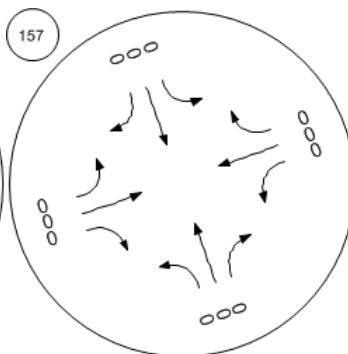
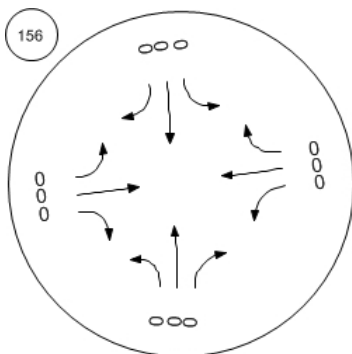


Saga Ln/Sand Hill Rd

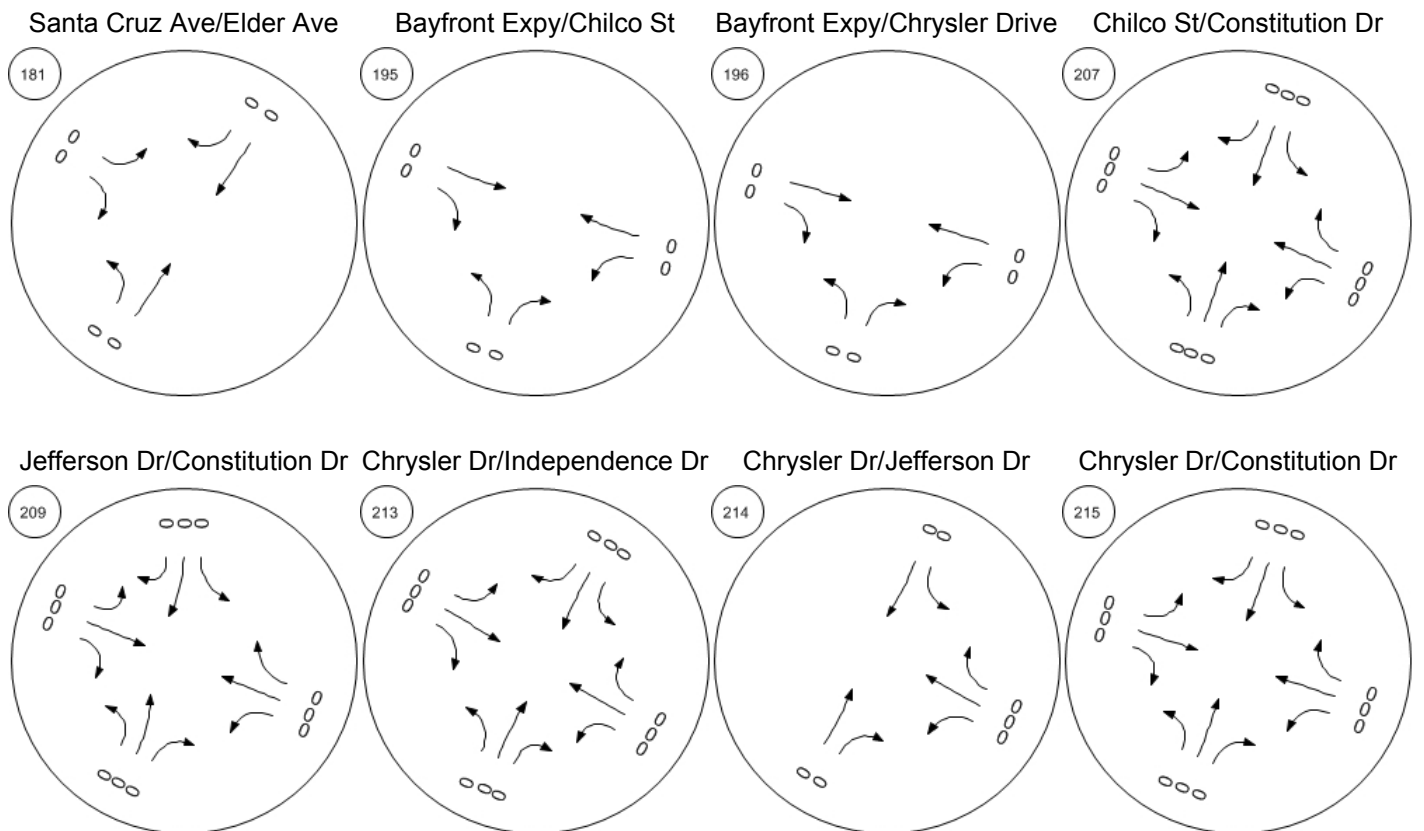
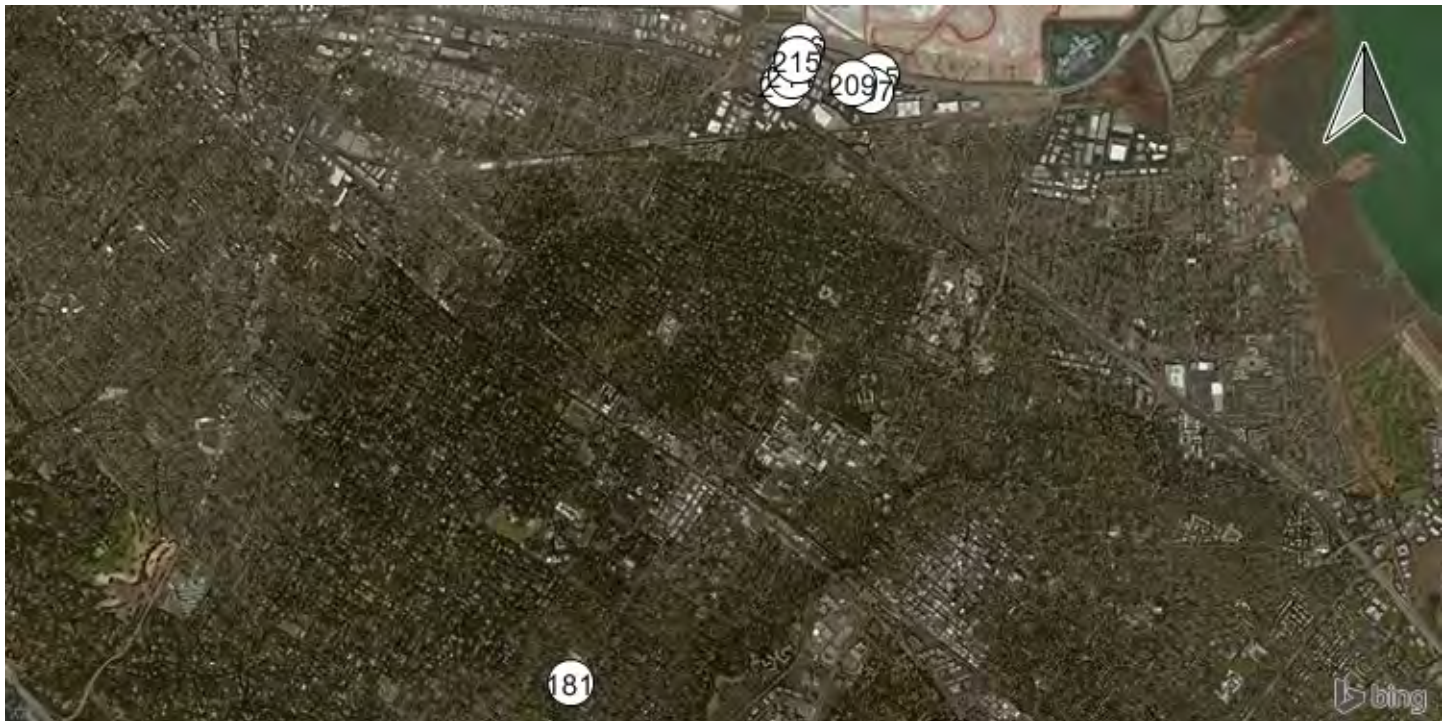
Branner Dr/Sand Hill Rd

Sharon Park Dr/ Sand Hill Rd

Bayfront Expy/Marsh Rd



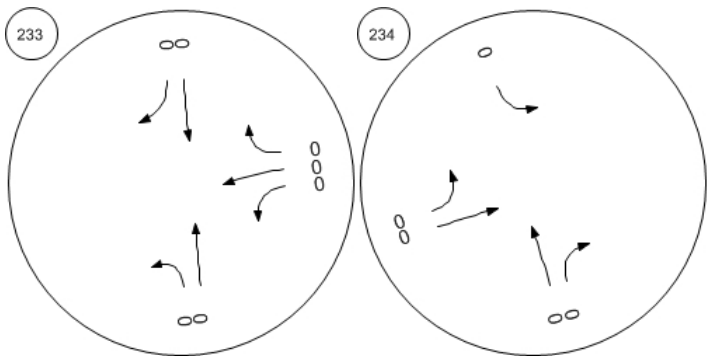
Traffic Volume - Other Volume



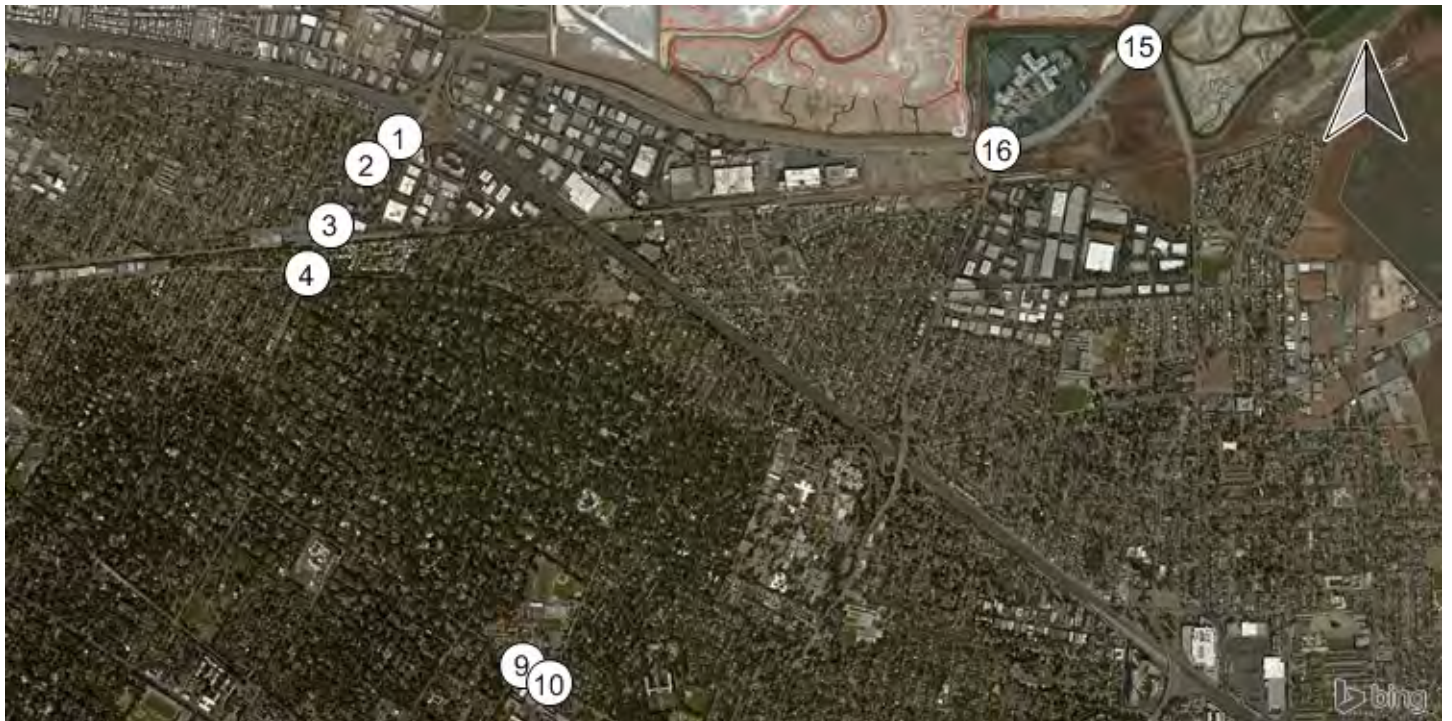
Traffic Volume - Other Volume



Sand Hill Road and Sand Hill Sand Hill Rd/Hwy 280 NB Off

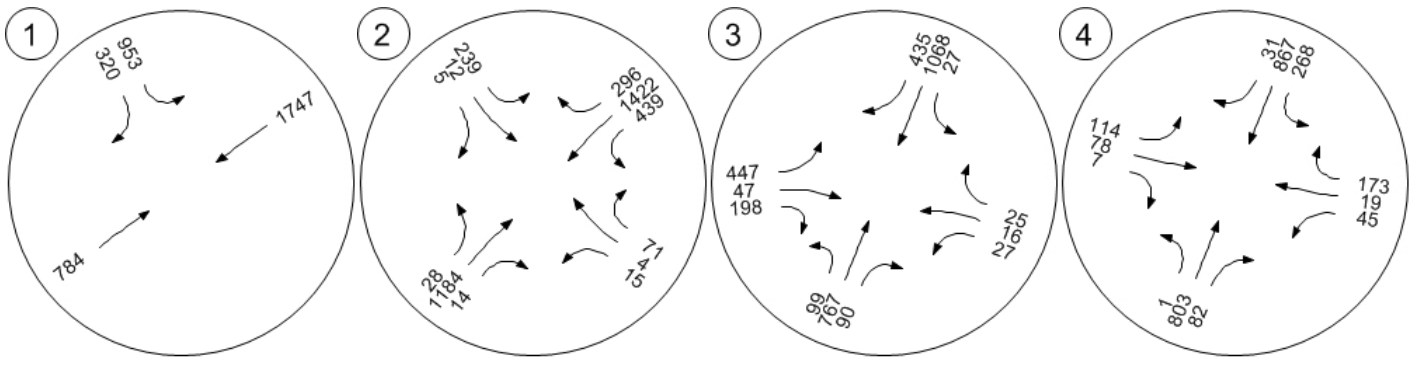


Traffic Volume - Future Total Volume



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

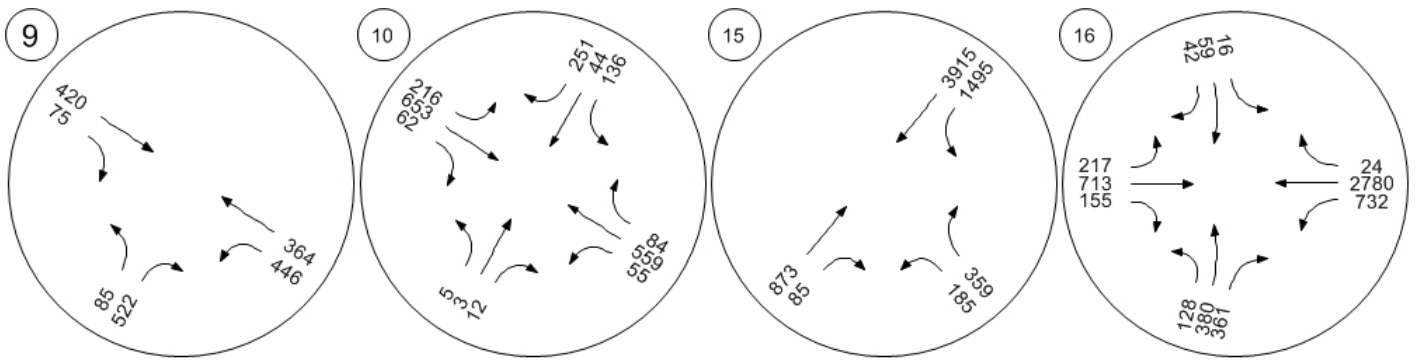


Middlefield Rd/Ravenswood

Middlefield Rd/Ringswood Av

Bayfront Expy (SR 84)/Univer

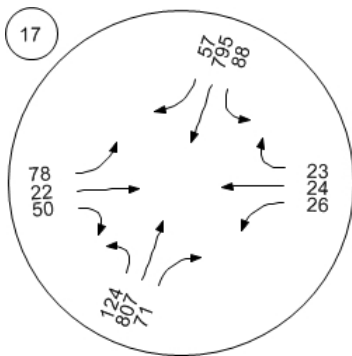
Bayfront Expy (SR 84)/Willow



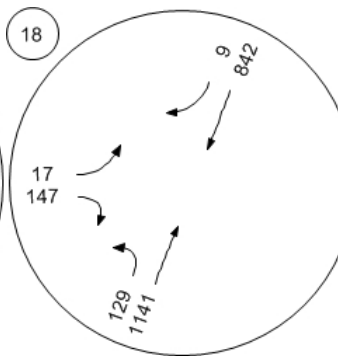
Traffic Volume - Future Total Volume



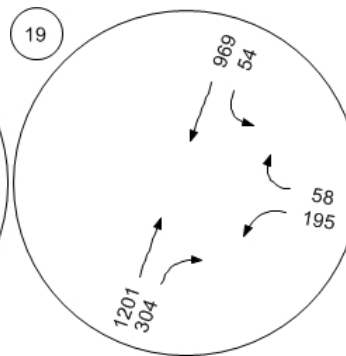
Willow Rd (SR 114)/Hamilton



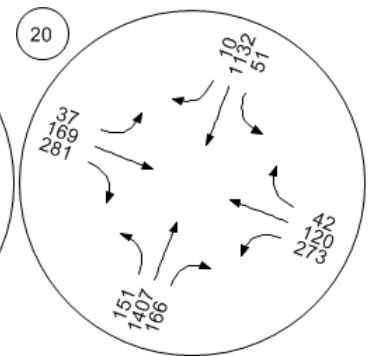
Willow Rd (SR 114)/Ivy Dr



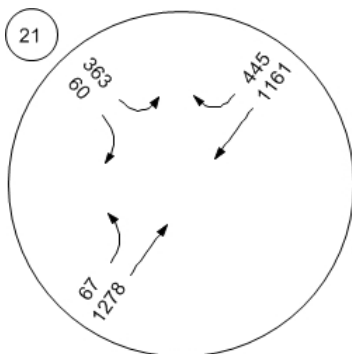
Willow Rd (SR 114)/O'Brien



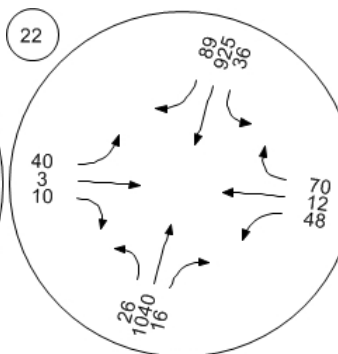
Willow Rd (SR 114)/Newbrid



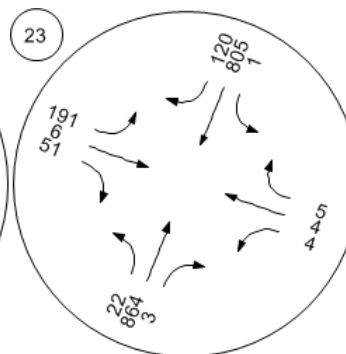
Willow Rd/Bay Rd



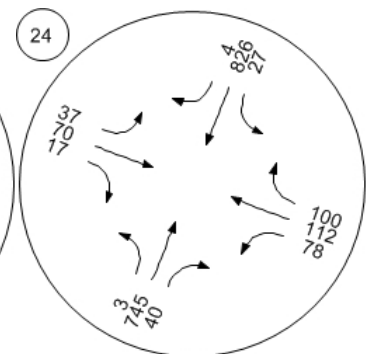
Willow Rd/Durham St-VA Me



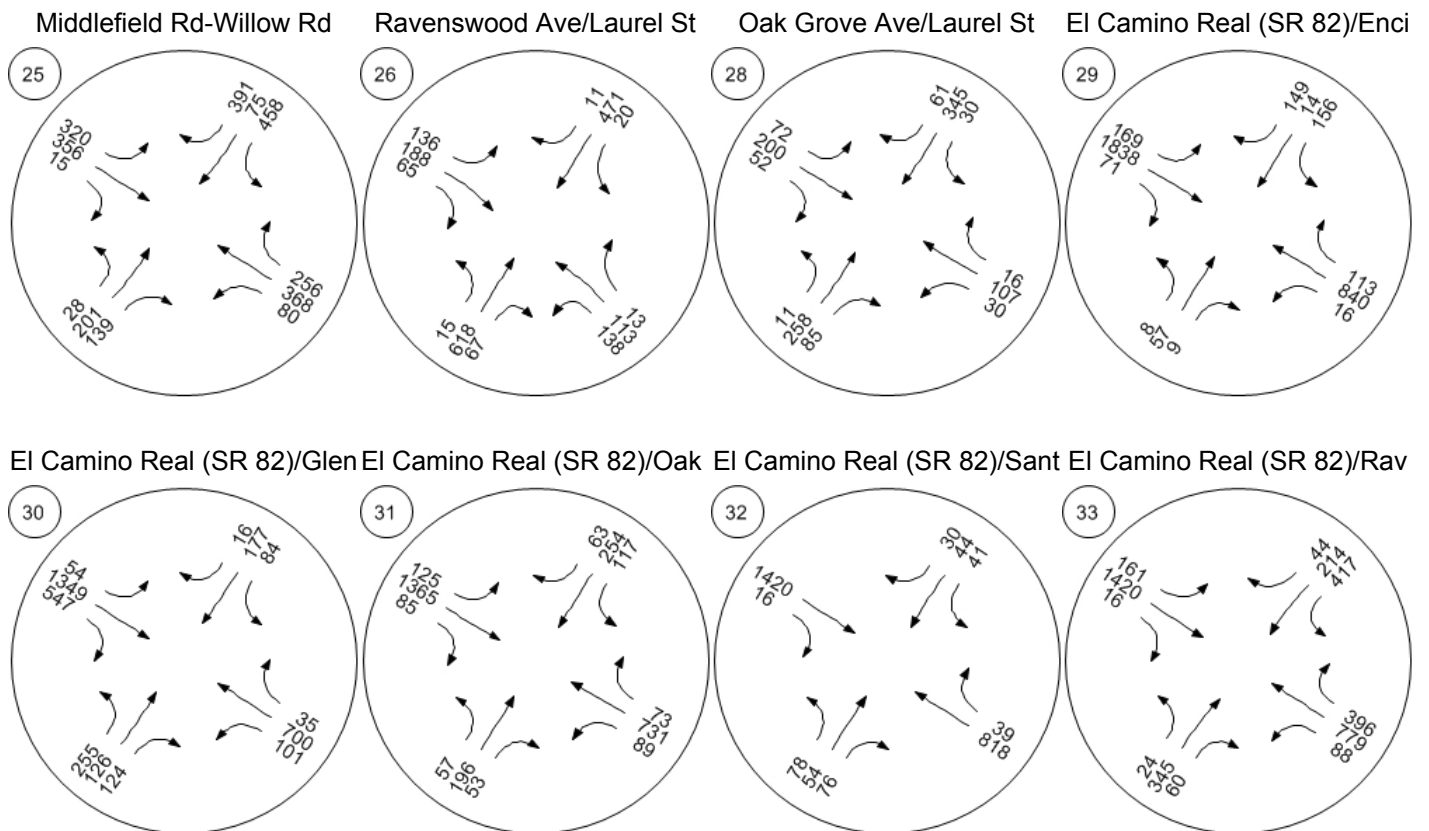
Willow Rd/Coleman Ave



Willow Rd/Gilbert Ave



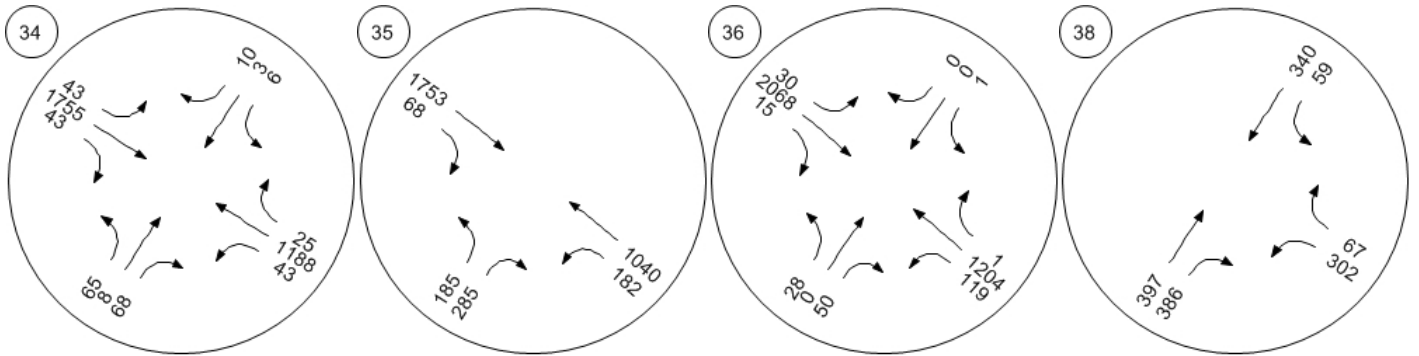
Traffic Volume - Future Total Volume



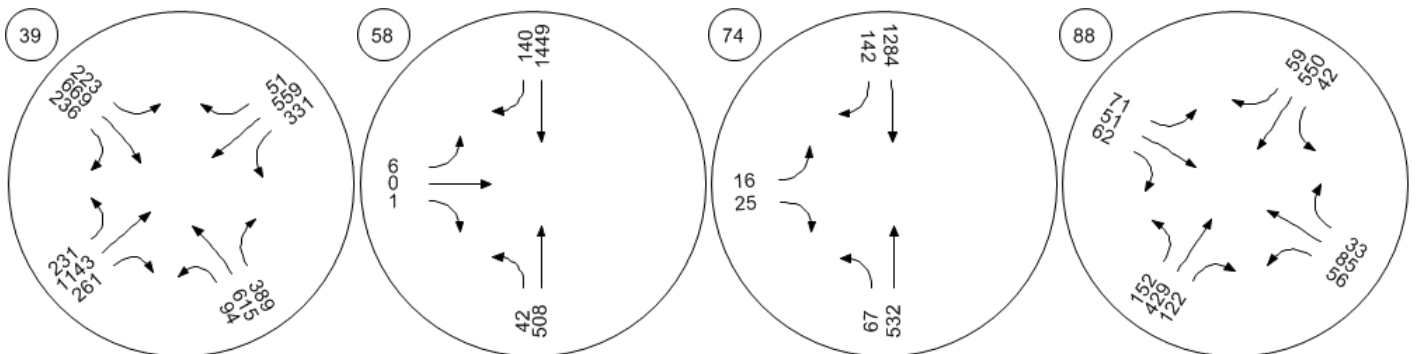
Traffic Volume - Future Total Volume



El Camino Real (SR 82)/Robl El Camino Real (SR 82)/Middle El Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



Santa Cruz Ave/Sand Hill Rd University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr

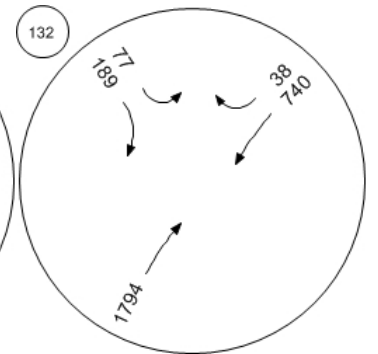
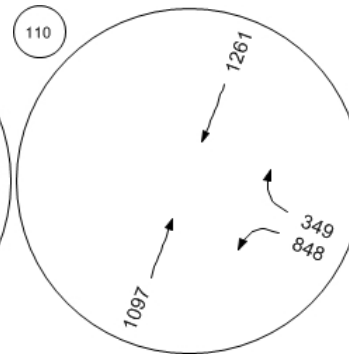
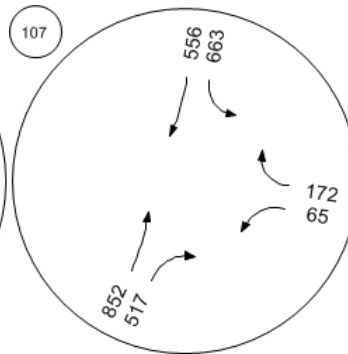
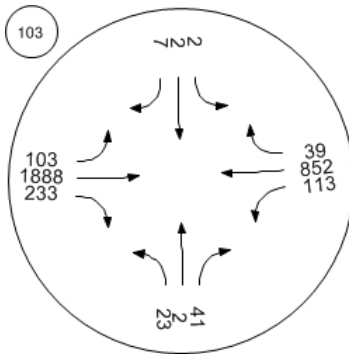


Traffic Volume - Future Total Volume



Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road and US 101 NB

Oak Ave/Sand Hill Rd

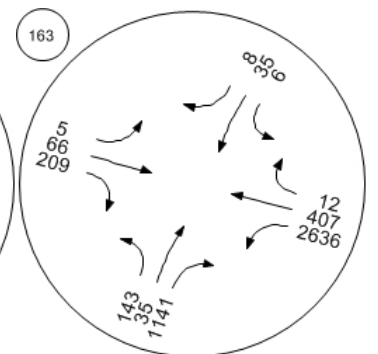
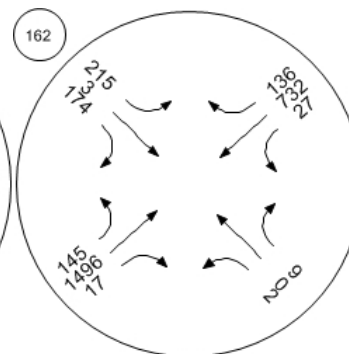
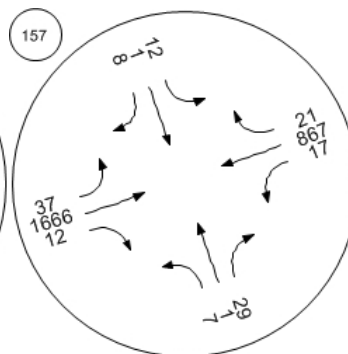
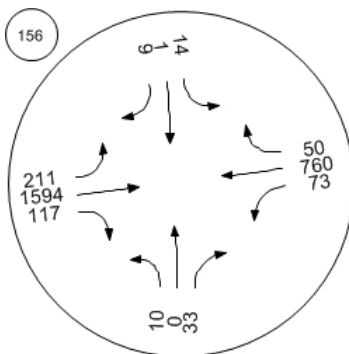


Saga Ln/Sand Hill Rd

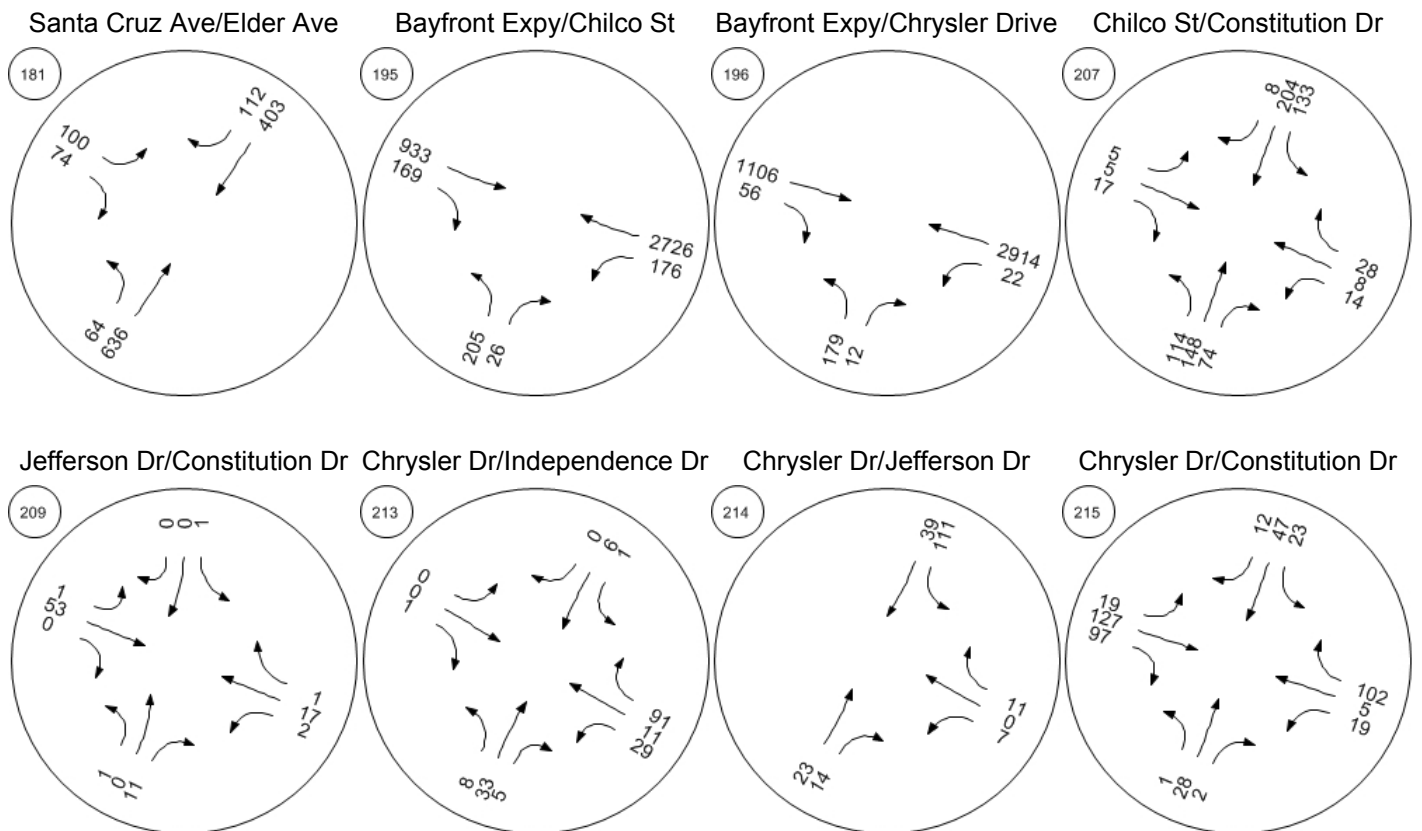
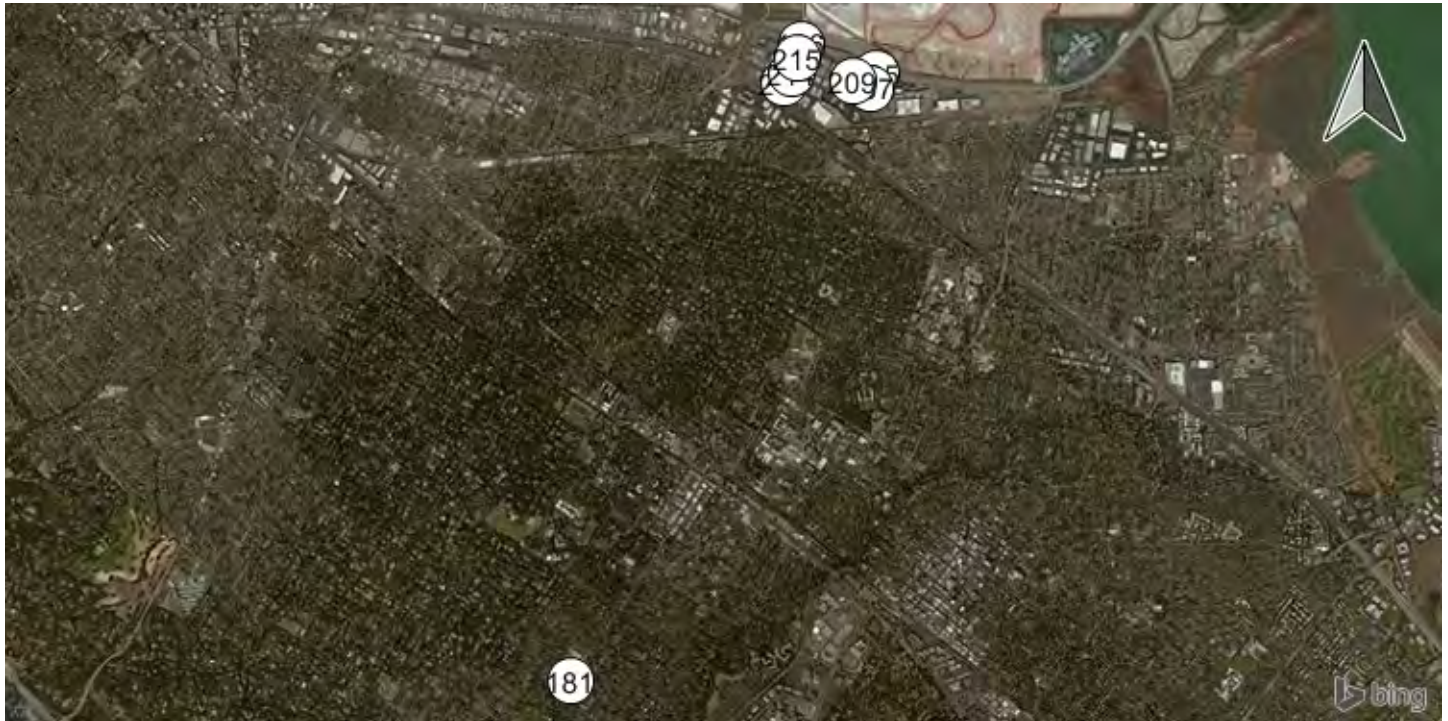
Branner Dr/Sand Hill Rd

Sharon Park Dr/ Sand Hill Rd

Bayfront Expy/Marsh Rd



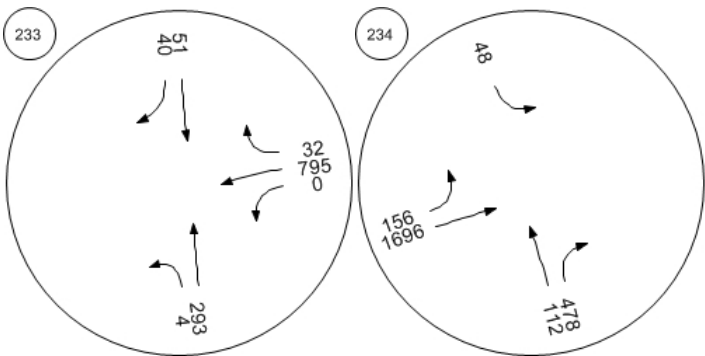
Traffic Volume - Future Total Volume



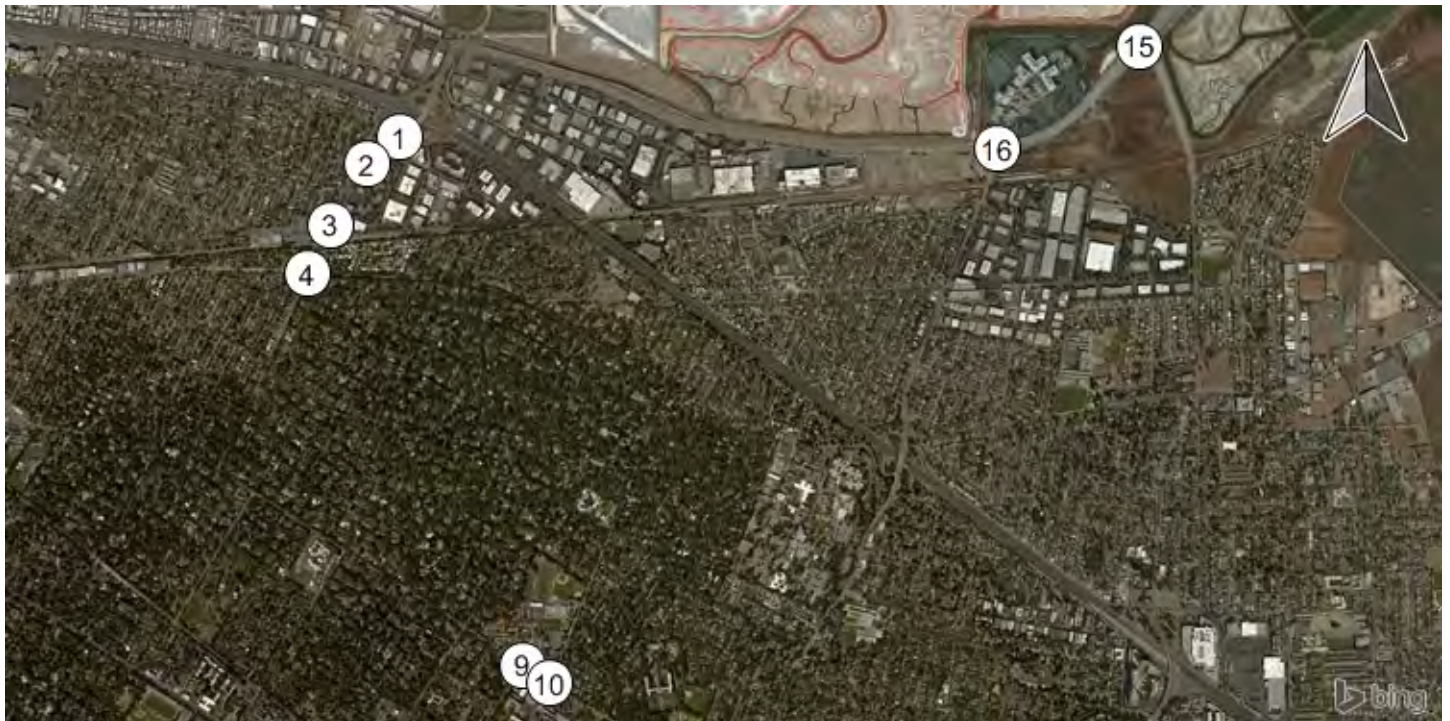
Traffic Volume - Future Total Volume



Sand Hill Road and Sand Hill Sand Hill Rd/Hwy 280 NB Off



Traffic Conditions



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

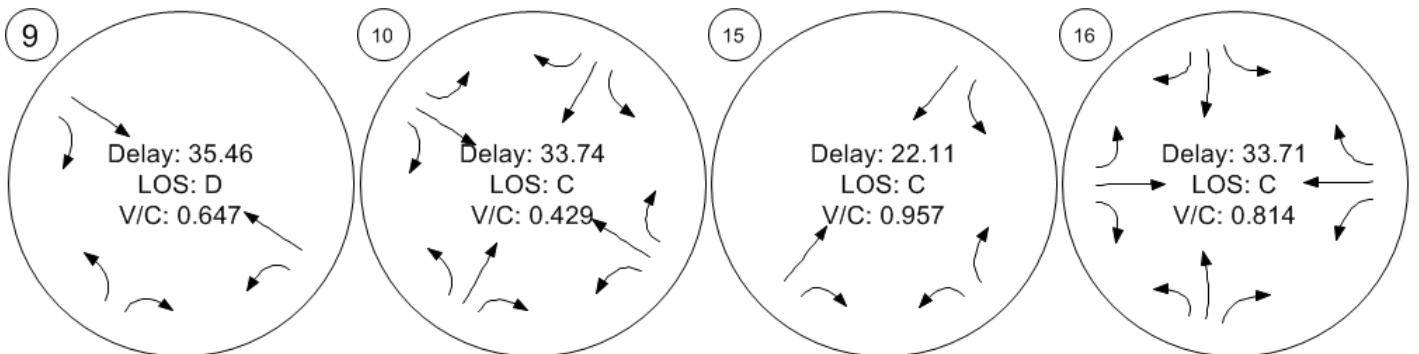


Middlefield Rd/Ravenswood

Middlefield Rd/Ringswood Av

Bayfront Expy (SR 84)/Univer

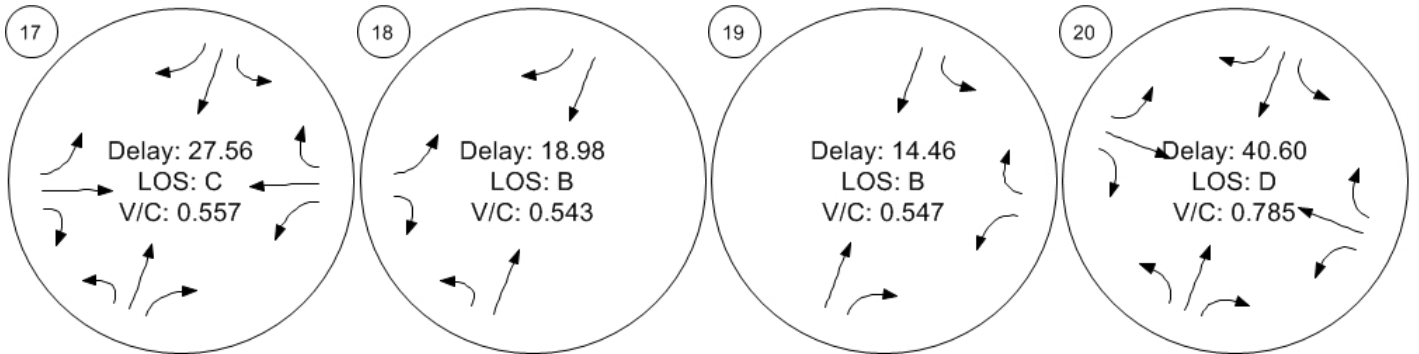
Bayfront Expy (SR 84)/Willow



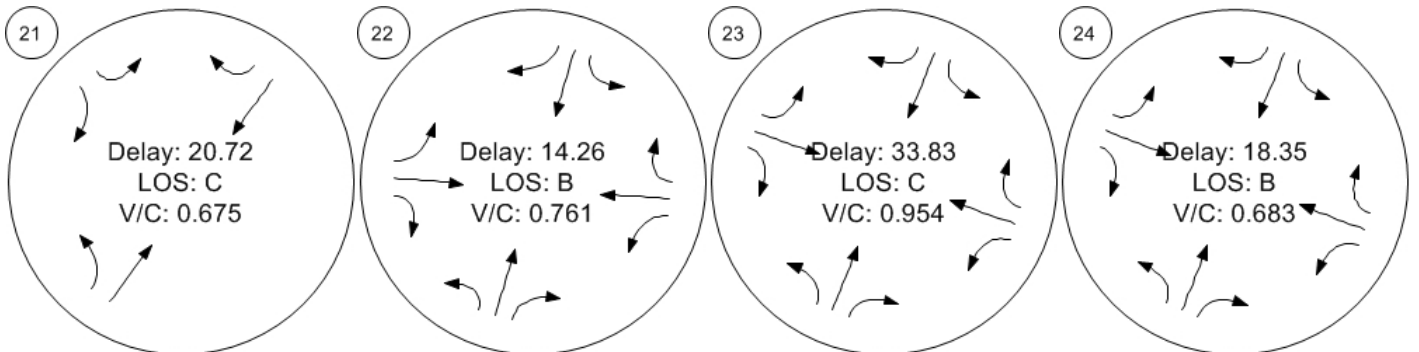
Traffic Conditions



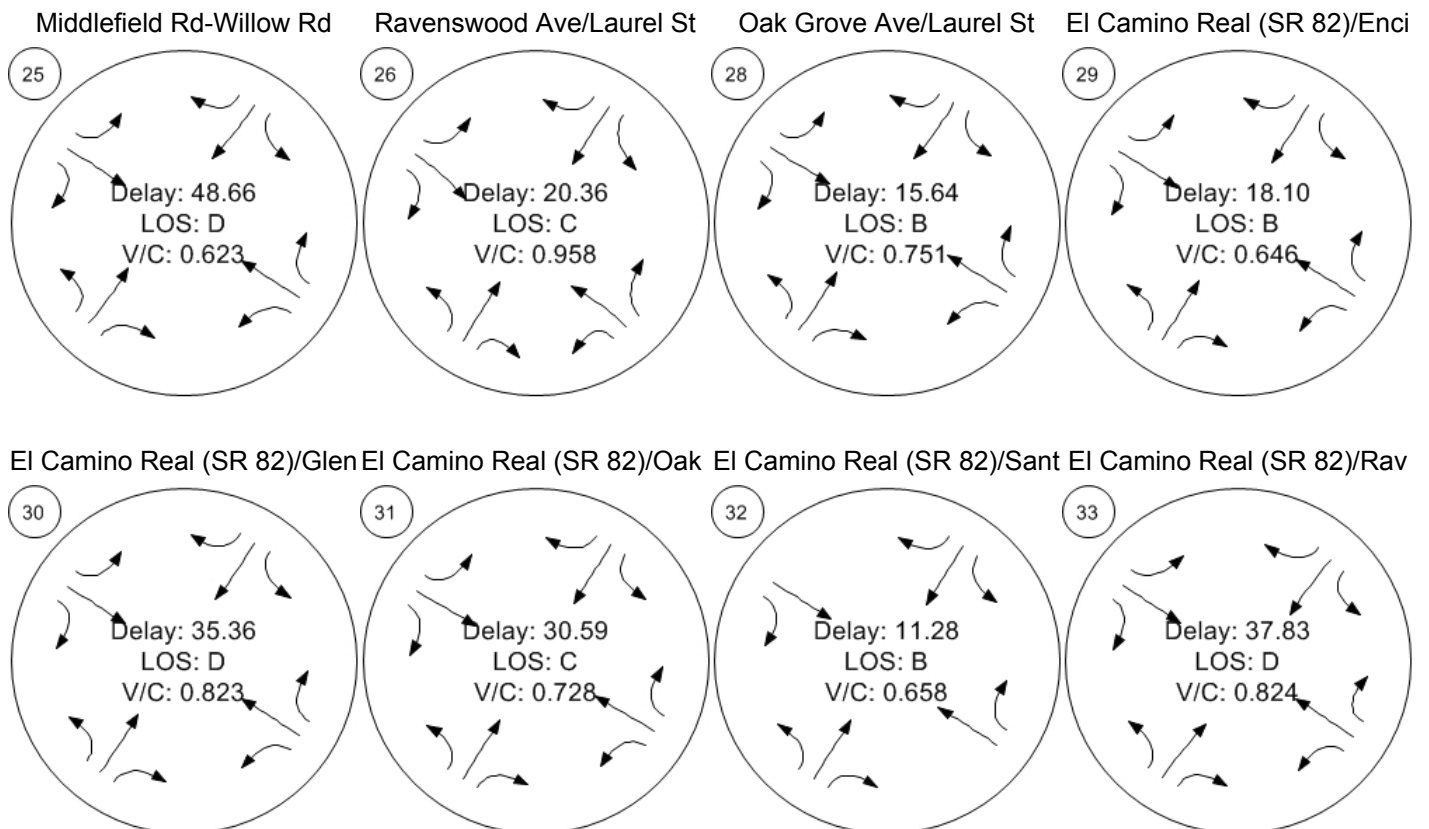
Willow Rd (SR 114)/Hamilton Willow Rd (SR 114)/Ivy Dr Willow Rd (SR 114)/O'Brien Willow Rd (SR 114)/Newbrid



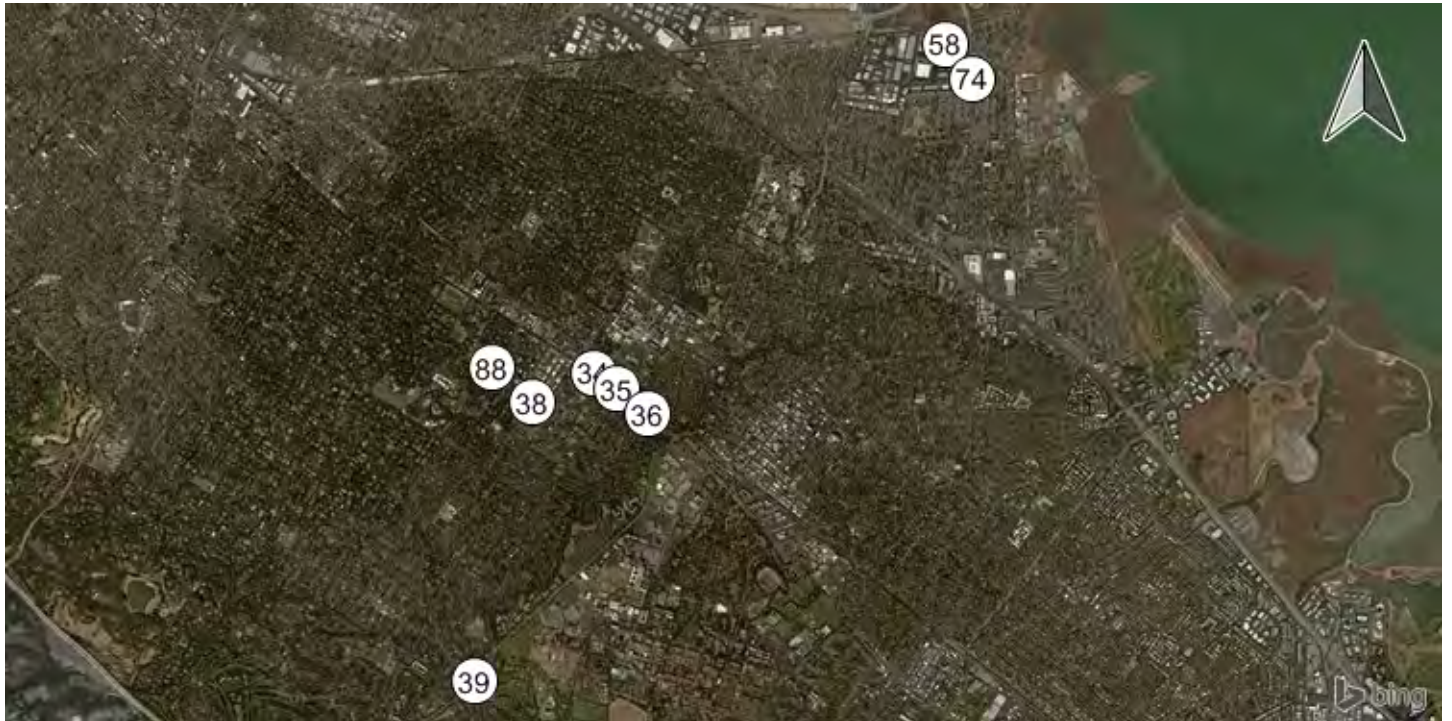
Willow Rd/Bay Rd Willow Rd/Durham St-VA Me Willow Rd/Coleman Ave Willow Rd/Gilbert Ave



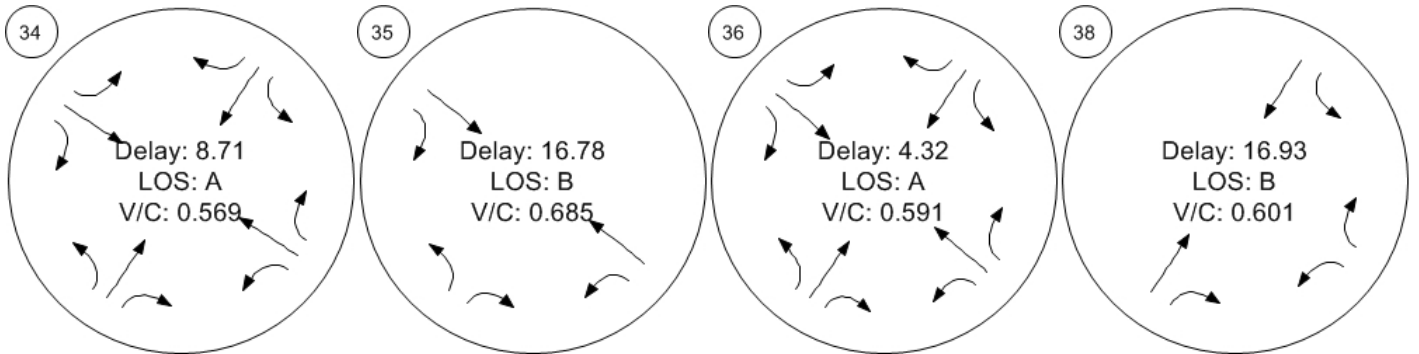
Traffic Conditions



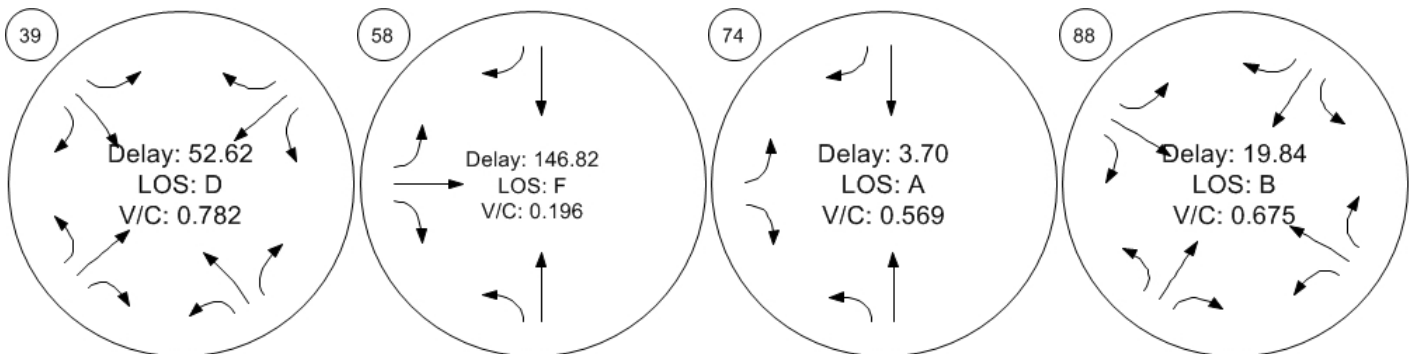
Traffic Conditions



El Camino Real (SR 82)/Robl El Camino Real (SR 82)/Middle El Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



Santa Cruz Ave/Sand Hill Rd University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr

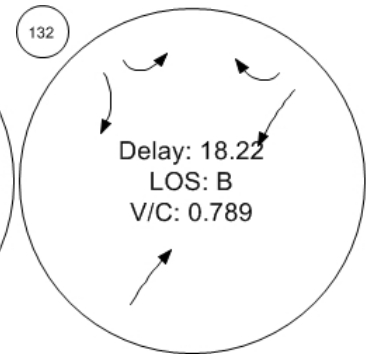
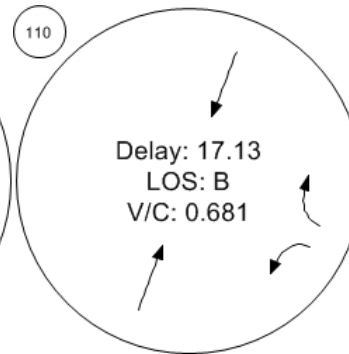
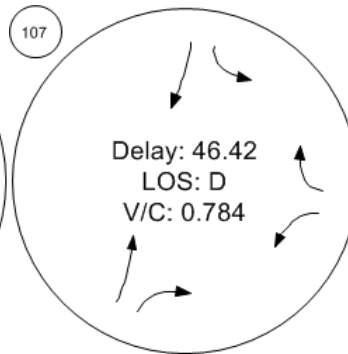
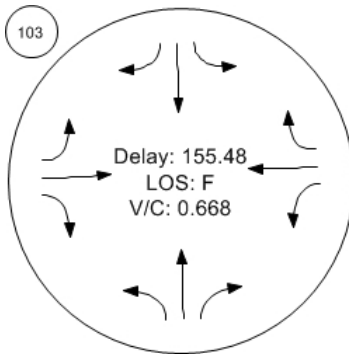


Traffic Conditions



Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road and US 101 NB

Oak Ave/Sand Hill Rd

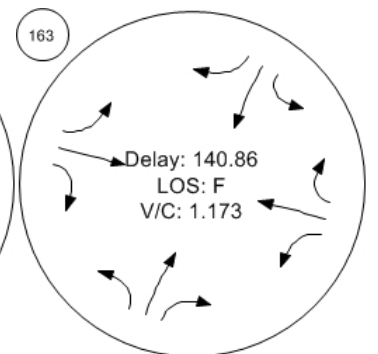
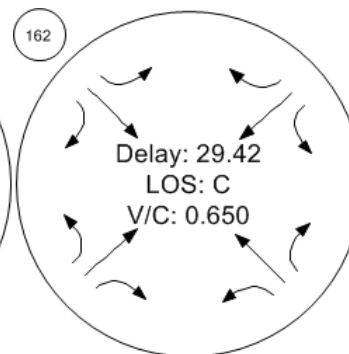
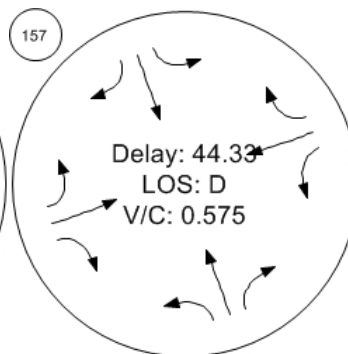
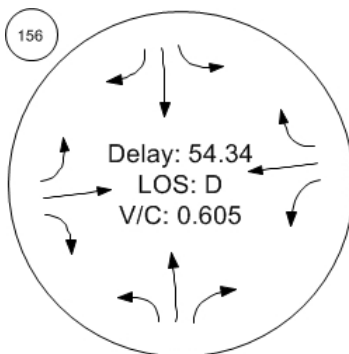


Saga Ln/Sand Hill Rd

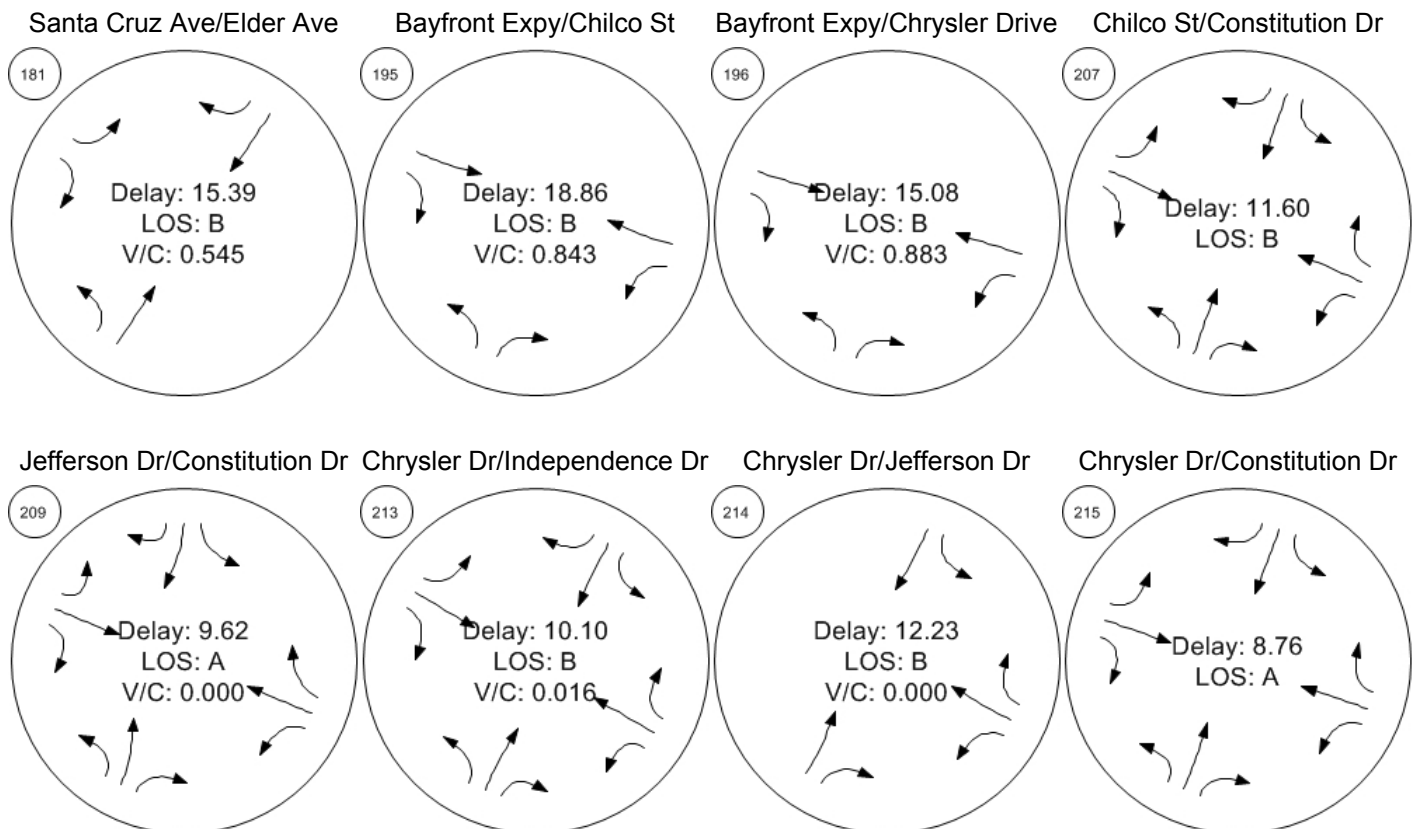
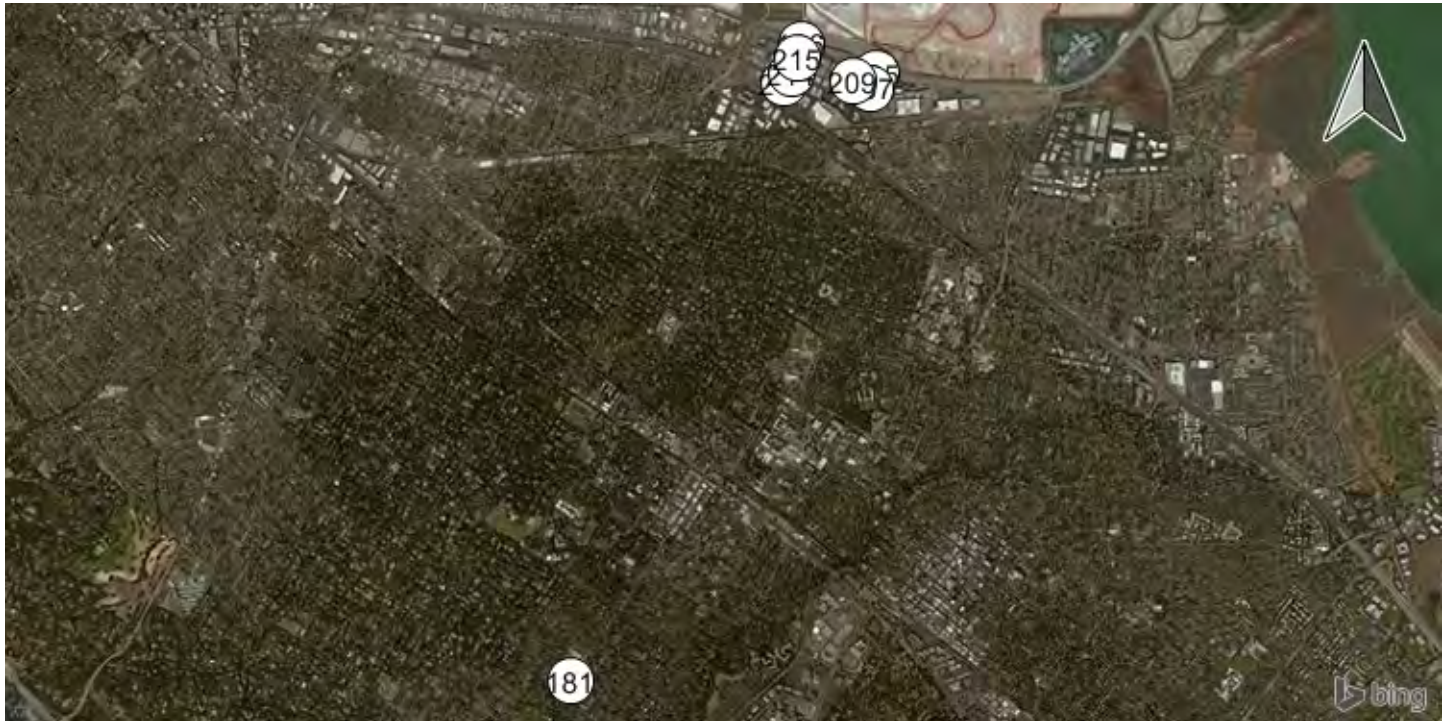
Branner Dr/Sand Hill Rd

Sharon Park Dr/ Sand Hill Rd

Bayfront Expy/Marsh Rd



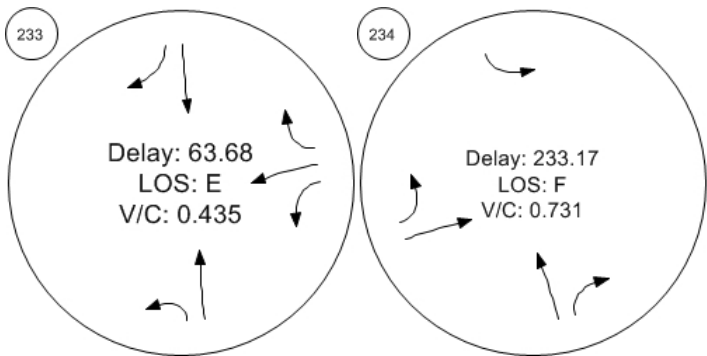
Traffic Conditions



Traffic Conditions



Sand Hill Road and Sand Hill Sand Hill Rd/Hwy 280 NB Off



Menlo Park GP Circulation Update

Vistro File: J:\...\Menlo Park PM_update.vistro
Report File: J:\...\Menlo Park_PM Results.pdf

Scenario 1: Existing PM
1/9/2015

Intersection Analysis Summary

ID	Intersection Name	Control Type	Method	Worst Mvmt	V/C	Delay (s/veh)	LOS
1	Marsh Rd (SR 84)/US 101 SB Offramp	Signalized	HCM2000	SWBT	1.000	25.5	C
2	Marsh Rd/Rolison Rd-Scott Dr	Signalized	HCM2000	SWBT	0.558	68.3	E
3	Marsh Rd/Florence St-Bohannon Dr	Signalized	HCM2000	SBR	0.699	79.4	E
4	Marsh Rd/Bay Rd	Signalized	HCM2000	WBR	0.584	26.2	C
9	Middlefield Rd/Ravenswood Ave	Signalized	HCM2000	NWBL	0.628	139.1	F
10	Middlefield Rd/Ringwood Ave	Signalized	HCM2000	SEBL	0.637	110.3	F
15	Bayfront Expy (SR 84)/University Ave (SR 109)	Signalized	HCM2000	NEBT	1.217	119.7	F
16	Bayfront Expy (SR 84)/Willow Rd (SR 114)	Signalized	HCM2000	SBL	0.901	48.4	D
17	Willow Rd (SR 114)/Hamilton Ave	Signalized	HCM2000	EBL	0.665	29.2	C
18	Willow Rd (SR 114)/Ivy Dr	Signalized	HCM2000	EBR	0.552	14.4	B
19	Willow Rd (SR 114)/O'Brien Dr	Signalized	HCM2000	SBL	0.568	16.1	B
20	Willow Rd (SR 114)/Newbridge St	Signalized	HCM2000	WBT	0.771	42.2	D
21	Willow Rd/Bay Rd	Signalized	HCM2000	SEBL	0.813	27.0	C
22	Willow Rd/Durham St-VA Med Entrance	Signalized	HCM2000	EBL	0.686	22.3	C
23	Willow Rd/Coleman Ave	Signalized	HCM2000	EBL	0.644	14.1	B
24	Willow Rd/Gilbert Ave	Signalized	HCM2000	WBL	0.574	25.6	C
25	Middlefield Rd-Willow Rd	Signalized	HCM2000	NEBR	0.561	46.8	D
26	Ravenswood Ave/Laurel St	Signalized	HCM2000	NWBL	0.972	23.0	C
28	Oak Grove Ave/Laurel St	Signalized	HCM2000	NWBT	0.716	15.4	B
29	El Camino Real (SR 82)/Encinal Ave-Menlo College Entrance	Signalized	HCM2000	SEBL	0.782	19.5	B
30	El Camino Real (SR 82)/Glenwood Ave-Valparaiso Ave	Signalized	HCM2000	SEBL	0.855	38.6	D

31	El Camino Real (SR 82)/Oak Grove Ave	Signalized	HCM2000	SWBL	0.755	31.9	C
32	El Camino Real (SR 82)/Santa Cruz Ave	Signalized	HCM2000	SWBT	0.730	13.7	B
33	El Camino Real (SR 82)/Ravenswood Ave-Menlo Ave	Signalized	HCM2000	SEBL	0.872	44.6	D
34	El Camino Real (SR 82)/Roble Ave	Signalized	HCM2000	NEBL	0.586	10.4	B
35	El Camino Real (SR 82)/Middle Ave	Signalized	HCM2000	NEBL	0.682	19.5	B
36	El Camino Real (SR 82)/Cambridge Ave	Signalized	HCM2000	NEBL	0.622	10.4	B
38	Santa Cruz Ave/University Dr (S)	Signalized	HCM2000	SWBL	0.619	19.2	B
39	Sand Hill Rd/Santa Cruz Ave	Signalized	HCM2000	SEBL	0.709	49.9	D
58	University Avenue and Adams Drive	Two-way stop	HCM2000	EBT	0.000	87.6	F
74	University Ave/O'Brien Dr	Signalized	HCM2000	NBL	0.710	9.7	A
88	Valparaiso Ave/ University Dr	Signalized	HCM2000	NWBL	0.731	31.5	C
103	Addison Wesley/Sand Hill Rd	Signalized	HCM2000	WBT	0.650	81.9	F
107	Alpine Rd/Santa Cruz Ave&Junipero Serra Blvd	Signalized	HCM2000	NEBT	0.732	51.3	D
110	Marsh Road/101 NB Ramps	Signalized	HCM2000	NBT	0.851	49.8	D
132	Oak Ave/Sand Hill Rd	Signalized	HCM2000	SEBR	0.556	6.2	A
156	Saga Ln/Sand Hill Rd	Signalized	HCM2000	WBT	0.516	35.2	D
157	Branner Dr/Sand Hill Rd	Signalized	HCM2000	EBL	0.451	21.0	C
162	Sharon Park Dr/ Sand Hill Rd	Signalized	HCM2000	SWBT	0.684	51.3	D
163	Bayfront Expy/Marsh Rd	Signalized	HCM2000	NBR	0.964	320.4	F
181	Santa Cruz Ave/Elder Ave	Signalized	HCM2000	SEBR	0.514	10.0	A
195	Bayfront Expy/Chilco St	Signalized	HCM2000	NBR	0.701	12.7	B
196	Bayfront Expy/Chrysler Drive	Signalized	HCM2000	WBL	0.795	21.6	C
207	Chilco St/Constitution Dr	All-way stop	HCM2000	NWBR		23.6	C
209	Jefferson Dr/Constitution Dr	Two-way stop	HCM2000	SBL	0.037	15.5	C
213	Chrysler Dr/Independence Dr	Two-way stop	HCM2000	SEBT	0.004	10.0	A
214	Chrysler Dr/Jefferson Dr	Two-way stop	HCM2000	NWBL	0.002	10.4	B
215	Chrysler Dr/Constitution Dr	All-way stop	HCM2000	EBT		14.4	B

233	Sand Hill Circle/Sand Hill Road	Signalized	HCM2000	WBT	0.712	361.8	F
234	Sand Hill Rd/Hwy 280 NB Off-Ramp	Signalized	HCM2000	NBR	0.360	31.3	C

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value; for all other control types, they are taken for the whole intersection.

**Intersection Level Of Service Report
#1: Marsh Rd (SR 84)/US 101 SB Offramp**

Control Type:	Signalized	Delay (sec / veh):	25.5
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	1.000

Intersection Setup

Name	Marsh Road		Marsh Road (SR 84)		US 101 SB Offramp	
Approach	Northeastbound		Southwestbound		Southeastbound	
Lane Configuration	⇕⇕		⇕⇕		⇐⇐⇐	
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	1
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	420.00
Speed [mph]	35.00		35.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	no		yes		no	

Volumes

Name	Marsh Road		Marsh Road (SR 84)		US 101 SB Offramp	
Base Volume Input [veh/h]	0	980	893	279	1547	401
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	2.60	2.70	2.15	3.60	0.50
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	980	893	279	1547	401
Peak Hour Factor	1.0000	0.9600	0.9600	1.0000	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	255	233	70	403	104
Total Analysis Volume [veh/h]	0	1021	930	279	1611	418
Presence of On-Street Parking	no		no		no	
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		0		0	
Bicycle Volume [bicycles/h]	0		0		0	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	100
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	55.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permissive	Permissive	Permissive	Permissive	Protected	Overlap
Signal Group	0	2	6	0	4	1
Auxiliary Signal Groups						1,2,4
Lead / Lag	-	-	-	-	Lead	-
Minimum Green [s]	0	8	8	0	6	4
Maximum Green [s]	0	0	0	0	0	0
Amber [s]	0.0	3.6	3.6	0.0	3.1	3.1
All red [s]	0.0	0.5	0.5	0.0	1.0	0.0
Split [s]	0	41	33	0	59	8
Vehicle Extension [s]	0.0	2.0	2.0	0.0	2.0	1.5
Walk [s]	0	0	7	0	7	0
Pedestrian Clearance [s]	0	0	16	0	22	0
I1, Start-Up Lost Time [s]	0.0	2.0	2.0	0.0	2.0	2.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		yes	yes		no	no
Maximum Recall		no	no		no	no
Pedestrian Recall		no	no		no	no
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	20.0	0.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	L	R
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	39	31	57	97
g / C, Green / Cycle	0.39	0.31	0.57	0.97
(v / s)_i Volume / Saturation Flow Rate	0.26	0.26	0.48	0.26
Total Saturation Flow Adjustment	1.05	0.93	0.89	0.85
s, saturation flow rate [veh/h]	4000	3522	3384	1607
c, Capacity [veh/h]	1560	1092	1929	1559
d1, Uniform Delay [s]	24.98	32.34	17.65	0.06
k, delay calibration	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	2.16	8.41	4.47	0.42
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00

Lane Group Results

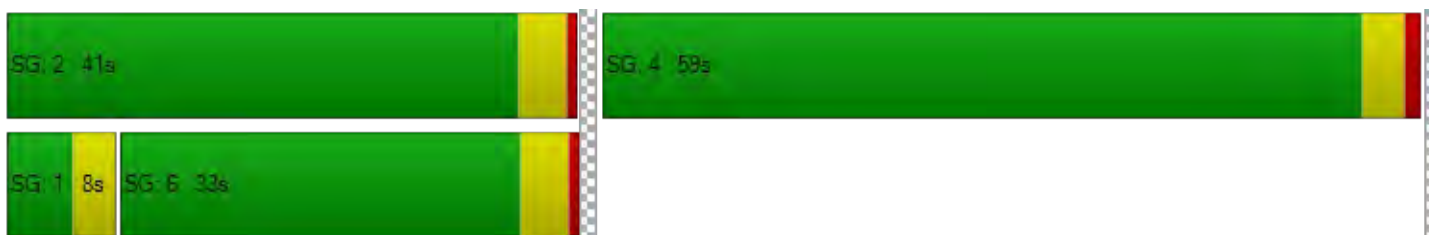
X, volume / capacity	0.65	0.85	0.84	0.27
d, Delay for Lane Group [s/veh]	27.14	40.75	22.11	0.48
Lane Group LOS	C	D	C	A
Critical Lane Group	no	yes	yes	yes
50th-Percentile Queue Length [veh]	14.12	16.30	23.90	1.08
50th-Percentile Queue Length [ft]	353.05	407.49	597.59	27.07
95th-Percentile Queue Length [veh]	23.43	26.70	38.45	2.60
95th-Percentile Queue Length [ft]	585.83	667.62	961.16	65.12

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	0.00	27.14	40.75	0.00	22.11	0.48
Movement LOS		C	D		C	A
d_A, Approach Delay [s/veh]	27.14		40.75		17.66	
Approach LOS	C		D		B	
d_I, Intersection Delay [s/veh]	25.49					
Intersection LOS	C					
Intersection V/C	1.000					

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	1	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#2: Marsh Rd/Rolison Rd-Scott Dr**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 68.3
 Level Of Service: E
 Volume to Capacity (v/c): 0.558

Intersection Setup

Name	Marsh Road			Marsh Road			Scott Drive			Rolison Drive		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	2	0	0	1	0	1	0	0	0
Pocket Length [ft]	155.00	100.00	100.00	350.00	100.00	100.00	60.00	100.00	35.00	100.00	100.00	100.00
Speed [mph]	35.00			35.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Marsh Road			Marsh Road			Scott Drive			Rolison Drive		
Base Volume Input [veh/h]	29	1083	4	66	1014	203	27	9	467	229	13	1
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	3.40	2.40	0.00	4.50	1.50	2.50	3.70	0.00	1.70	1.30	7.70	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	326	0	0	0
Total Hourly Volume [veh/h]	29	1083	4	66	1014	203	27	9	141	229	13	1
Peak Hour Factor	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	7	276	1	17	259	52	7	2	36	58	3	0
Total Analysis Volume [veh/h]	30	1105	4	67	1035	207	28	9	144	234	13	1
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	2			0			0			0		
Bicycle Volume [bicycles/h]	0			1			0			0		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	140
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	27.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Split	Split	Split	Split	Split	Split
Signal Group	5	2	0	1	6	0	0	3	0	0	4	0
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lead	-	-	-	-	-	-	-	-
Minimum Green [s]	4	10	0	4	10	0	0	6	0	0	4	0
Maximum Green [s]	0	0	0	0	0	0	0	0	0	0	0	0
Amber [s]	3.0	4.0	0.0	3.0	4.0	0.0	0.0	3.2	0.0	0.0	3.2	0.0
All red [s]	1.0	1.0	0.0	1.0	1.0	0.0	0.0	1.0	0.0	0.0	1.0	0.0
Split [s]	14	50	0	12	48	0	0	41	0	0	37	0
Vehicle Extension [s]	2.5	3.5	0.0	2.0	3.5	0.0	0.0	2.5	0.0	0.0	3.0	0.0
Walk [s]	0	7	0	0	7	0	0	8	0	0	8	0
Pedestrian Clearance [s]	0	21	0	0	21	0	0	28	0	0	24	0
I1, Start-Up Lost Time [s]	2.0	2.0	0.0	2.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	yes		no	yes			no			no	
Pedestrian Recall	no	no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	0.0	20.0	20.0	0.0	0.0	20.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	R	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	12	48	10	46	39	39	35	35
g / C, Green / Cycle	0.09	0.34	0.07	0.33	0.28	0.28	0.25	0.25
(v / s)_i Volume / Saturation Flow Rate	0.02	0.22	0.02	0.36	0.02	0.05	0.13	0.01
Total Saturation Flow Adjustment	0.92	0.89	0.88	0.91	0.96	0.74	0.94	0.92
s, saturation flow rate [veh/h]	1746	5052	3354	3468	1831	2811	1782	1745
c, Capacity [veh/h]	150	1732	240	1140	510	783	445	436
d1, Uniform Delay [s]	59.54	38.73	61.59	47.00	37.18	38.40	45.33	39.69
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	2.99	1.83	2.89	54.65	0.28	0.52	4.38	0.14
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

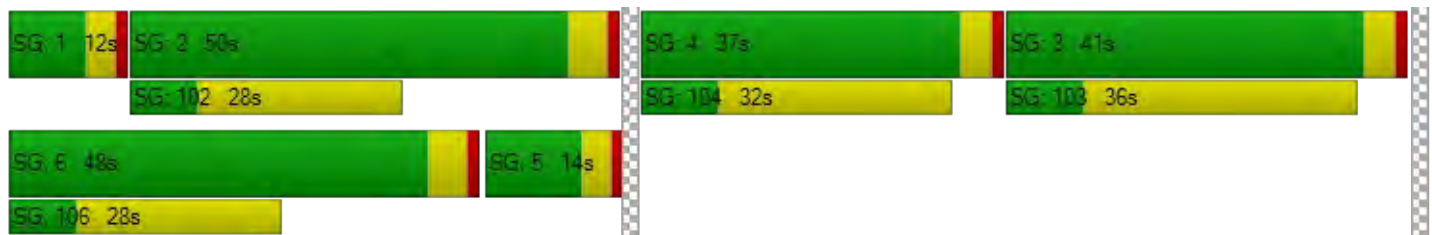
X, volume / capacity	0.20	0.64	0.28	1.09	0.07	0.18	0.53	0.03
d, Delay for Lane Group [s/veh]	62.53	40.56	64.48	101.65	37.46	38.92	49.71	39.83
Lane Group LOS	E	D	E	F	D	D	D	D
Critical Lane Group	yes	no	no	yes	no	yes	yes	no
50th-Percentile Queue Length [veh]	1.19	15.22	1.41	38.73	1.14	2.60	8.80	0.44
50th-Percentile Queue Length [ft]	29.70	380.58	35.23	968.16	28.38	65.08	220.02	11.01
95th-Percentile Queue Length [veh]	2.84	25.08	3.32	61.98	2.72	5.71	15.60	1.11
95th-Percentile Queue Length [ft]	70.93	627.05	82.94	1549.48	68.03	142.79	389.88	27.70

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	62.53	40.56	40.56	64.48	101.65	101.65	37.46	37.46	38.92	49.71	39.83	39.83
Movement LOS	E	D	D	E	F	F	D	D	D	D	D	D
d_A, Approach Delay [s/veh]	41.14			99.74			38.62			49.15		
Approach LOS	D			F			D			D		
d_I, Intersection Delay [s/veh]	68.34											
Intersection LOS	E											
Intersection V/C	0.558											

Sequence

Ring 1	1	2	4	3	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	5	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#3: Marsh Rd/Florence St-Bohannon Dr**

Control Type:	Signalized	Delay (sec / veh):	79.4
Analysis Method:	HCM2000	Level Of Service:	E
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.699

Intersection Setup

Name	Marsh Road			Marsh Road			Florence Street			Bohannon Avenue		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	T T T			T T T			T T T			T T T		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	1	0	0	1	0	0
Pocket Length [ft]	145.00	100.00	100.00	135.00	100.00	100.00	155.00	100.00	100.00	90.00	100.00	100.00
Speed [mph]	35.00			35.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Marsh Road			Marsh Road			Florence Street			Bohannon Avenue		
Base Volume Input [veh/h]	185	720	84	30	697	377	475	15	130	98	41	72
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.70	3.20	6.00	6.70	2.20	4.00	2.50	0.00	0.80	4.10	0.00	6.90
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	174	0	0	0
Total Hourly Volume [veh/h]	185	720	84	30	697	377	475	15	0	98	41	72
Peak Hour Factor	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	51	198	23	8	191	104	130	4	0	27	11	20
Total Analysis Volume [veh/h]	203	791	92	33	766	414	522	16	0	108	45	79
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	16			1			7			3		
Bicycle Volume [bicycles/h]	3			1			1			4		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	140
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	31.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Split	Split	Split	Split	Split	Split
Signal Group	5	2	2	1	6	6	4	3	4	8	4	8
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lag	-	-	Lag	-	-	-	-	-
Minimum Green [s]	4	12	12	4	12	12	8	8	8	0	8	0
Maximum Green [s]	0	0	0	0	0	0	0	0	0	0	0	0
Amber [s]	3.0	3.6	3.6	3.0	3.6	3.6	3.2	3.0	3.2	0.0	3.2	0.0
All red [s]	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	0.0	1.0	0.0
Split [s]	22	55	55	12	45	45	37	36	37	0	37	0
Vehicle Extension [s]	2.0	5.0	5.0	2.0	5.0	5.0	2.5	2.5	2.5	0.0	2.5	0.0
Walk [s]	0	7	7	0	7	7	7	7	7	0	7	0
Pedestrian Clearance [s]	0	19	19	0	16	16	25	25	25	0	25	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	yes		no	yes			no			no	
Pedestrian Recall	no	no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	R	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	20	53	10	43	34	34	35	35
g / C, Green / Cycle	0.14	0.38	0.07	0.31	0.24	0.24	0.25	0.25
(v / s)_i Volume / Saturation Flow Rate	0.12	0.26	0.02	0.35	0.16	0.00	0.06	0.07
Total Saturation Flow Adjustment	0.93	0.91	0.89	0.88	0.91	0.84	0.91	0.90
s, saturation flow rate [veh/h]	1758	3446	1692	3329	3450	1602	1734	1703
c, Capacity [veh/h]	251	1304	121	1023	838	389	433	426
d1, Uniform Delay [s]	58.14	36.35	61.56	48.50	47.54	40.13	41.99	42.47
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	23.73	2.84	5.50	80.62	3.77	0.00	1.37	1.73
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

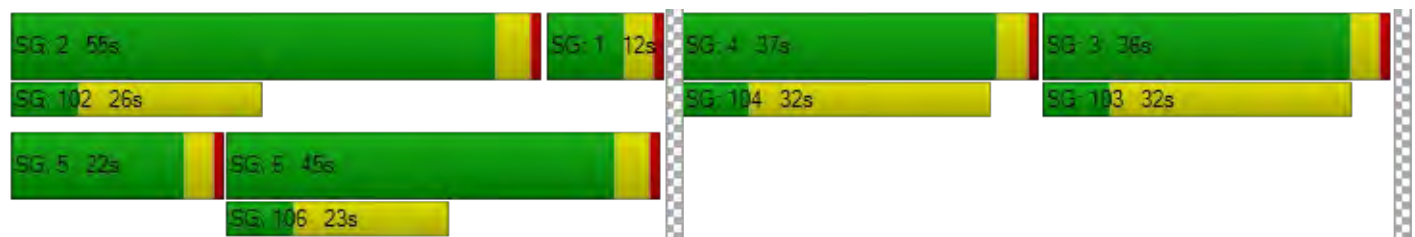
X, volume / capacity	0.81	0.68	0.27	1.15	0.64	0.00	0.25	0.29
d, Delay for Lane Group [s/veh]	81.88	39.18	67.06	129.12	51.31	40.13	43.36	44.20
Lane Group LOS	F	D	E	F	D	D	D	D
Critical Lane Group	yes	no	no	yes	yes	no	no	yes
50th-Percentile Queue Length [veh]	9.55	17.38	1.35	39.50	11.32	0.00	3.64	4.25
50th-Percentile Queue Length [ft]	238.71	434.47	33.67	987.57	282.99	0.00	91.12	106.23
95th-Percentile Queue Length [veh]	16.69	28.34	3.18	63.22	19.29	0.00	7.59	8.62
95th-Percentile Queue Length [ft]	417.29	708.59	79.60	1580.48	482.20	0.00	189.75	215.39

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	81.88	39.18	39.18	67.06	129.12	129.12	51.31	51.31	40.13	43.36	44.20	44.20
Movement LOS	F	D	D	E	F	F	D	D	D	D	D	D
d_A, Approach Delay [s/veh]	47.16			127.43			51.31			43.81		
Approach LOS	D			F			D			D		
d_I, Intersection Delay [s/veh]	79.36											
Intersection LOS	E											
Intersection V/C	0.699											

Sequence

Ring 1	1	2	4	3	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#4: Marsh Rd/Bay Rd**

Control Type:	Signalized	Delay (sec / veh):	26.2
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.584

Intersection Setup

Name	Marsh Road			Marsh Road			Bay Road			Bay Road		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	⇌⇌			⇌⇌			⊕			⊕		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	1	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	260.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	35.00			35.00			25.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	no			yes			yes			no		

Volumes

Name	Marsh Road			Marsh Road			Bay Road			Bay Road		
Base Volume Input [veh/h]	2	681	108	194	841	54	45	29	6	84	18	143
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	2.30	0.90	1.00	1.00	0.00	2.20	6.90	0.00	1.20	0.00	2.80
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	2	681	108	194	841	54	45	29	6	84	18	143
Peak Hour Factor	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	1	193	31	55	239	15	13	8	2	24	5	41
Total Analysis Volume [veh/h]	2	774	123	220	956	61	51	33	7	95	20	163
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0			7			7			0		
Bicycle Volume [bicycles/h]	1			7			7			11		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	140
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	33.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	2	6	2	5	2	6	4	4	4	8	4	8
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lag	-	-	-	-	-
Minimum Green [s]	6	6	6	4	6	6	6	6	6	0	6	0
Maximum Green [s]	0	0	0	0	0	0	0	0	0	0	0	0
Amber [s]	3.6	3.6	3.6	3.1	3.6	3.6	3.6	3.6	3.6	0.0	3.6	0.0
All red [s]	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	0.0	1.0	0.0
Split [s]	107	74	107	33	107	74	33	33	33	0	33	0
Vehicle Extension [s]	3.0	3.0	3.0	2.0	3.0	3.0	3.6	3.6	3.6	0.0	3.6	0.0
Walk [s]	7	0	7	0	7	0	7	7	7	0	7	0
Pedestrian Clearance [s]	12	0	12	0	12	0	20	20	20	0	20	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no		no	no			no			no	
Maximum Recall		yes		no	yes			no			no	
Pedestrian Recall		no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	L	C	C	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	0.00	2.00	2.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	72	31	105	31	31
g / C, Green / Cycle	0.51	0.22	0.75	0.22	0.22
(v / s)_i Volume / Saturation Flow Rate	0.27	0.12	0.29	0.08	0.19
Total Saturation Flow Adjustment	0.87	0.94	0.93	0.60	0.77
s, saturation flow rate [veh/h]	3304	1787	3548	1137	1472
c, Capacity [veh/h]	1699	396	2661	252	326
d1, Uniform Delay [s]	22.69	48.39	6.13	46.12	52.31
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	1.18	5.54	0.42	3.99	23.60
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.53	0.56	0.38	0.36	0.85
d, Delay for Lane Group [s/veh]	23.87	53.93	6.55	50.11	75.91
Lane Group LOS	C	D	A	D	E
Critical Lane Group	yes	yes	no	no	yes
50th-Percentile Queue Length [veh]	13.82	8.57	8.48	3.32	13.18
50th-Percentile Queue Length [ft]	345.50	214.28	212.08	83.12	329.52
95th-Percentile Queue Length [veh]	22.98	15.26	15.13	7.03	22.03
95th-Percentile Queue Length [ft]	574.58	381.44	378.20	175.74	550.84

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	23.87	23.87	23.87	53.93	6.55	6.55	50.11	50.11	50.11	75.91	75.91	75.91
Movement LOS	C	C	C	D	A	A	D	D	D	E	E	E
d_A, Approach Delay [s/veh]	23.87			14.98			50.11			75.91		
Approach LOS	C			B			D			E		
d_I, Intersection Delay [s/veh]	26.21											
Intersection LOS	C											
Intersection V/C	0.584											

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	5	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#9: Middlefield Rd/Ravenswood Ave**

Control Type:	Signalized	Delay (sec / veh):	139.1
Analysis Method:	HCM2000	Level Of Service:	F
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.628

Intersection Setup

Name	Ravenswood Avenue		Middlefield Road		Middlefield Road	
Approach	Northeastbound		Northwestbound		Southeastbound	
Lane Configuration						
Turning Movement	Left	Right	Left	Thru	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	1	0	0	0	0
Pocket Length [ft]	100.00	120.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00		35.00		35.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		no	

Volumes

Name	Ravenswood Avenue		Middlefield Road		Middlefield Road	
Base Volume Input [veh/h]	199	610	457	673	357	60
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.10	1.30	0.60	1.40	6.70
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	1000	0	0	0	0
Total Hourly Volume [veh/h]	199	0	457	673	357	60
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	52	0	120	177	94	16
Total Analysis Volume [veh/h]	209	0	481	708	376	63
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	6		13		0	
Bicycle Volume [bicycles/h]	9		26		13	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	160
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Protected	Permissive	Protected	Permissive	Permissive	Permissive
Signal Group	3	2	1	6	2	4
Auxiliary Signal Groups						
Lead / Lag	Lead	-	Lead	-	-	-
Minimum Green [s]	4	10	4	10	10	0
Maximum Green [s]	0	0	0	0	0	0
Amber [s]	3.2	3.6	3.0	3.6	3.6	0.0
All red [s]	1.0	1.0	1.0	1.0	1.0	0.0
Split [s]	80	53	27	80	53	0
Vehicle Extension [s]	2.5	3.6	3.0	3.6	3.6	0.0
Walk [s]	7	7	0	7	7	0
Pedestrian Clearance [s]	12	12	0	12	12	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	no		no	no	no	
Maximum Recall	no		no	yes	yes	
Pedestrian Recall	no		no	no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	R	L	C	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	78	78	25	78	51
g / C, Green / Cycle	0.49	0.49	0.16	0.49	0.32
(v / s)_i Volume / Saturation Flow Rate	0.12	0.00	0.27	0.37	0.24
Total Saturation Flow Adjustment	0.93	0.83	0.94	0.99	0.96
s, saturation flow rate [veh/h]	1770	1582	1782	1889	1832
c, Capacity [veh/h]	863	771	278	921	584
d1, Uniform Delay [s]	23.83	21.01	67.50	33.61	48.83
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.67	0.00	342.14	6.14	8.66
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00

Lane Group Results

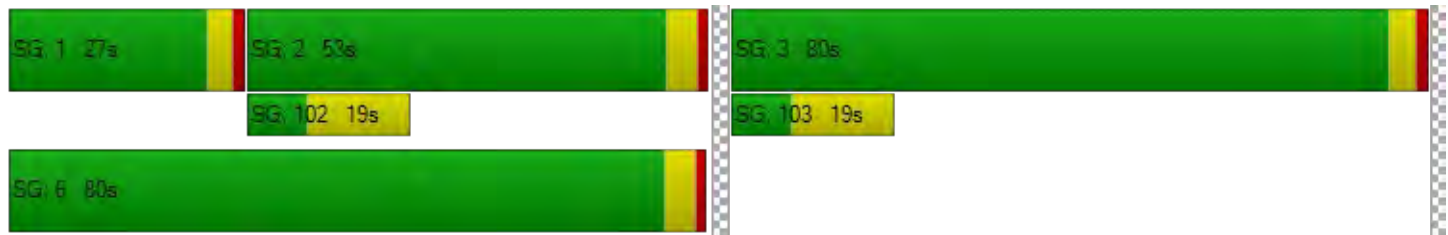
X, volume / capacity	0.24	0.00	1.73	0.77	0.75
d, Delay for Lane Group [s/veh]	24.49	21.01	409.64	39.76	57.49
Lane Group LOS	C	C	F	D	E
Critical Lane Group	yes	no	yes	no	yes
50th-Percentile Queue Length [veh]	5.89	0.00	48.26	30.38	20.52
50th-Percentile Queue Length [ft]	147.20	0.00	1206.56	759.40	513.00
95th-Percentile Queue Length [veh]	11.23	0.00	77.22	48.67	33.17
95th-Percentile Queue Length [ft]	280.85	0.00	1930.58	1216.79	829.27

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	24.49	21.01	409.64	39.76	57.49	57.49
Movement LOS	C	C	F	D	E	E
d_A, Approach Delay [s/veh]	24.49		189.39		57.49	
Approach LOS	C		F		E	
d_I, Intersection Delay [s/veh]	139.11					
Intersection LOS	F					
Intersection V/C	0.628					

Sequence

Ring 1	1	2	3	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#10: Middlefield Rd/Ringwood Ave**

Control Type:	Signalized	Delay (sec / veh):	110.3
Analysis Method:	HCM2000	Level Of Service:	F
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.637

Intersection Setup

Name	D Street			Ringwood Avenue			Middlefield Road			Middlefield Road		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	↵↵			↵↵			↵↵↵			↵↵		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	0	0	1	1	0	1	1	0	0
Pocket Length [ft]	30.00	100.00	100.00	100.00	100.00	250.00	175.00	100.00	100.00	125.00	100.00	100.00
Speed [mph]	20.00			30.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	D Street			Ringwood Avenue			Middlefield Road			Middlefield Road		
Base Volume Input [veh/h]	58	70	38	63	1	272	10	775	97	379	568	5
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.70	0.00	0.00	0.00	0.00	2.20	0.00	1.70	0.00	2.10	1.80	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	8	0	0	57	0	0	0
Total Hourly Volume [veh/h]	58	70	38	63	1	264	10	775	40	379	568	5
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	15	18	10	17	0	69	3	204	11	100	149	1
Total Analysis Volume [veh/h]	61	74	40	66	1	278	11	816	42	399	598	5
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	8			14			5			0		
Bicycle Volume [bicycles/h]	14			14			17			16		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	160
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	58.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	2	8	2	6	8	6	5	2	8	1	6	4
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	10	6	10	10	6	10	4	10	6	4	10	0
Maximum Green [s]	0	0	0	0	0	0	0	0	0	0	0	0
Amber [s]	3.6	3.2	3.6	3.6	3.2	3.6	3.5	3.6	3.2	3.0	3.6	0.0
All red [s]	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	0.0
Split [s]	53	25	53	80	25	80	55	53	25	27	80	0
Vehicle Extension [s]	3.6	2.9	3.6	3.6	2.9	3.6	3.0	3.6	2.9	3.0	3.6	0.0
Walk [s]	7	7	7	7	7	7	0	7	7	0	7	0
Pedestrian Clearance [s]	12	21	12	12	21	12	0	12	21	0	12	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	yes		no	yes	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	C	R	L	C	R	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	23	23	23	23	53	51	51	25	78
g / C, Green / Cycle	0.14	0.14	0.14	0.14	0.33	0.31	0.31	0.15	0.48
(v / s)_i Volume / Saturation Flow Rate	0.05	0.06	0.07	0.18	0.01	0.23	0.03	0.23	0.17
Total Saturation Flow Adjustment	0.59	0.93	0.51	0.81	0.95	0.94	0.82	0.93	0.93
s, saturation flow rate [veh/h]	1115	1776	971	1531	1805	3557	1562	1768	3549
c, Capacity [veh/h]	157	251	137	216	587	1113	489	271	1698
d1, Uniform Delay [s]	63.60	64.25	64.58	70.00	37.34	49.93	39.54	69.00	26.70
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	7.06	5.86	11.97	159.32	0.06	4.29	0.35	231.20	0.58
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

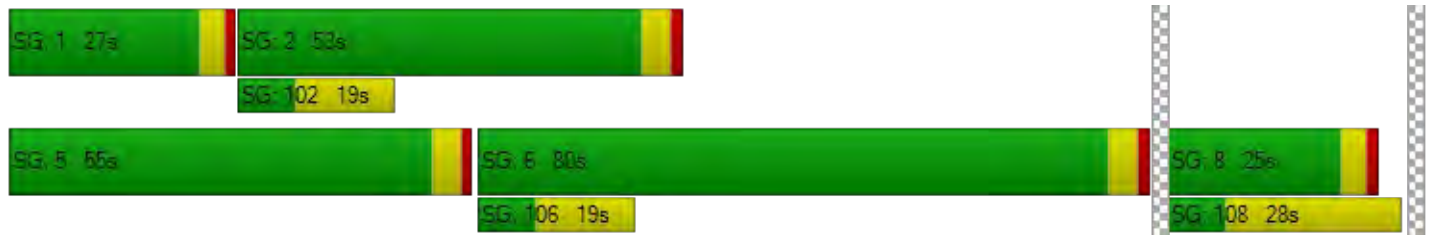
X, volume / capacity	0.39	0.46	0.49	1.29	0.02	0.73	0.09	1.47	0.36
d, Delay for Lane Group [s/veh]	70.66	70.11	76.55	229.32	37.40	54.22	39.89	300.20	27.28
Lane Group LOS	E	E	E	F	D	D	D	F	C
Critical Lane Group	no	no	no	yes	no	yes	no	yes	no
50th-Percentile Queue Length [veh]	2.80	5.27	3.19	22.42	0.36	20.15	1.44	35.98	9.88
50th-Percentile Queue Length [ft]	70.07	131.73	79.83	560.59	9.02	503.81	36.03	899.51	246.93
95th-Percentile Queue Length [veh]	6.08	10.27	6.80	36.13	0.91	32.60	3.39	57.60	17.17
95th-Percentile Queue Length [ft]	152.11	256.70	169.88	903.27	22.83	815.04	84.65	1439.89	429.34

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	70.66	70.11	70.11	76.55	76.55	229.32	37.40	54.22	39.89	300.20	27.28	27.28
Movement LOS	E	E	E	E	E	F	D	D	D	F	C	C
d_A, Approach Delay [s/veh]	70.30			199.65			53.32			135.96		
Approach LOS	E			F			D			F		
d_I, Intersection Delay [s/veh]	110.31											
Intersection LOS	F											
Intersection V/C	0.637											

Sequence




Ring 1	1	2	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#15: Bayfront Expy (SR 84)/University Ave (SR 109)**

Control Type:	Signalized	Delay (sec / veh):	119.7
Analysis Method:	HCM2000	Level Of Service:	F
Analysis Period:	15 minutes	Volume to Capacity (v/c):	1.217

Intersection Setup

Name	Bayfront Expressway (SR84)		Bayfront Expressway (SR84)		University Avenue (SR109)	
Approach	Northeastbound		Southwestbound		Northwestbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	1	2	0	1	2
Pocket Length [ft]	100.00	430.00	830.00	100.00	175.00	1000.00
Speed [mph]	55.00		55.00		35.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	no		yes		yes	

Volumes

Name	Bayfront Expressway (SR84)		Bayfront Expressway (SR84)		University Avenue (SR109)	
Base Volume Input [veh/h]	3800	62	304	813	78	1577
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.40	16.10	4.90	3.80	9.00	1.50
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	3800	62	304	813	78	1577
Peak Hour Factor	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	969	16	78	207	20	402
Total Analysis Volume [veh/h]	3878	63	310	830	80	1609
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		0		8	
Bicycle Volume [bicycles/h]	1		0		0	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	160
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	9.00

Phasing & Timing

Control Type	Permissive	Permissive	Protected	Permissive	Protected	Overlap
Signal Group	2	2	1	6	4	1
Auxiliary Signal Groups						1,4
Lead / Lag	-	-	Lead	-	Lead	-
Minimum Green [s]	4	4	4	4	8	4
Maximum Green [s]	16	16	4	24	16	4
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	100	100	48	148	12	48
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	0	1	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no		no	no	no	no
Maximum Recall	no		no	no	no	no
Pedestrian Recall	no		no	no	no	no
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	6.0	6.0	20.0	6.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	L	C	L	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	96	96	44	144	8	56
g / C, Green / Cycle	0.60	0.60	0.28	0.90	0.05	0.35
(v / s)_i Volume / Saturation Flow Rate	0.77	0.05	0.09	0.17	0.02	0.38
Total Saturation Flow Adjustment	0.89	0.73	0.88	0.87	0.85	0.74
s, saturation flow rate [veh/h]	5054	1391	3342	4986	3216	4224
c, Capacity [veh/h]	3033	835	919	4488	161	1479
d1, Uniform Delay [s]	32.00	13.41	46.35	0.96	74.04	52.00
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	128.12	0.18	0.99	0.09	10.59	51.32
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	1.28	0.08	0.34	0.18	0.50	1.09
d, Delay for Lane Group [s/veh]	160.12	13.58	47.34	1.05	84.63	103.32
Lane Group LOS	F	B	D	A	F	F
Critical Lane Group	yes	no	no	no	no	yes
50th-Percentile Queue Length [veh]	109.20	1.30	6.18	2.17	2.06	39.77
50th-Percentile Queue Length [ft]	2730.11	32.40	154.45	54.32	51.61	994.25
95th-Percentile Queue Length [veh]	174.73	3.07	11.68	4.88	4.67	63.65
95th-Percentile Queue Length [ft]	4368.18	76.84	292.02	122.09	116.73	1591.14

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	160.12	13.58	47.34	1.05	84.63	103.32
Movement LOS	F	B	D	A	F	F
d_A, Approach Delay [s/veh]	157.77		13.64		102.43	
Approach LOS	F		B		F	
d_I, Intersection Delay [s/veh]	119.70					
Intersection LOS	F					
Intersection V/C	1.217					

Sequence

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#16: Bayfront Expy (SR 84)/Willow Rd (SR 114)**

Control Type:	Signalized	Delay (sec / veh):	48.4
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.901

Intersection Setup

Name	Willow Road (SR 114)			Willow Road			Ba Ex			Ba Ex		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	3	0	0	1	0	1	2	0	1	2	0	0
Pocket Length [ft]	265.00	100.00	100.00	45.00	100.00	80.00	165.00	100.00	140.00	1000.00	100.00	100.00
Speed [mph]	40.00			20.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			no			yes			no		

Volumes

Name	Willow Road (SR 114)			Willow Road			Ba Ex			Ba Ex		
Base Volume Input [veh/h]	24	46	1404	116	191	116	35	2362	75	359	675	10
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	4.20	10.90	3.30	4.30	1.00	1.70	37.10	2.50	12.00	6.40	5.30	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	-300	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	70	0	0	45	0	0	1
Total Hourly Volume [veh/h]	24	46	1104	116	191	46	35	2362	30	359	675	9
Peak Hour Factor	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	6	12	282	30	49	12	9	603	8	92	172	2
Total Analysis Volume [veh/h]	24	47	1127	118	195	47	36	2410	31	366	689	9
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	1			0			7			0		
Bicycle Volume [bicycles/h]	0			7			0			0		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	12.00

Phasing & Timing

Control Type	Permiss	Permiss	Overlap	Split	Split	Split	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	8	8	1	4	4	4	5	2	2	1	6	6
Auxiliary Signal Groups			1,8									
Lead / Lag	Lag	-	-	Lag	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	4	16	16	16	4	16	16	4	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	19	19	32	21	21	21	15	58	58	32	75	75
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	0	1	1	1	0	1	1	0	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no	no		no		no	no		no	no	
Maximum Recall		no	no		no		no	no		no	no	
Pedestrian Recall		no	no		no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	6.0	6.0	20.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	R	L	C	R	L	C	R	L	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	2.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	0.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	15	15	47	17	17	17	11	54	54	28	71	71
g / C, Green / Cycle	0.12	0.12	0.36	0.13	0.13	0.13	0.08	0.42	0.42	0.22	0.55	0.55
(v / s)_i Volume / Saturation Flow Rate	0.01	0.02	0.34	0.07	0.05	0.03	0.01	0.41	0.02	0.11	0.12	0.01
Total Saturation Flow Adjustment	0.91	0.64	0.88	0.91	0.94	0.84	0.67	1.03	0.76	0.87	1.03	0.85
s, saturation flow rate [veh/h]	1732	2432	3343	1731	3582	1588	2557	5846	1442	3294	5846	1615
c, Capacity [veh/h]	200	281	1209	226	468	208	216	2428	599	710	3193	882
d1, Uniform Delay [s]	51.58	51.87	39.97	52.71	51.94	50.61	55.24	37.80	22.70	45.02	15.18	13.46
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	1.23	1.29	14.06	8.34	2.71	2.52	1.65	16.57	0.16	2.67	0.16	0.02
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.12	0.17	0.93	0.52	0.42	0.23	0.17	0.99	0.05	0.52	0.22	0.01
d, Delay for Lane Group [s/veh]	52.81	53.15	54.03	61.05	54.65	53.13	56.90	54.37	22.87	47.69	15.33	13.48
Lane Group LOS	D	D	D	E	D	D	E	D	C	D	B	B
Critical Lane Group	no	no	yes	yes	no	no	no	yes	no	no	no	no
50th-Percentile Queue Length [veh]	0.84	0.88	29.18	4.52	3.79	1.66	0.68	43.64	0.73	6.76	5.15	0.16
50th-Percentile Queue Length [ft]	21.06	22.04	729.51	113.04	94.66	41.58	17.12	1091.08	18.13	168.99	128.79	4.06
95th-Percentile Queue Length [veh]	2.06	2.15	46.77	9.07	7.83	3.85	1.69	69.84	1.79	12.56	10.08	0.42
95th-Percentile Queue Length [ft]	51.50	53.74	1169.35	226.63	195.84	96.34	42.31	1745.91	44.68	314.11	252.04	10.42

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	52.81	53.15	54.03	61.05	54.65	53.13	56.90	54.37	22.87	47.69	15.33	13.48
Movement LOS	D	D	D	E	D	D	E	D	C	D	B	B
d_A, Approach Delay [s/veh]	53.97			56.55			54.01			26.45		
Approach LOS	D			E			D			C		
d_I, Intersection Delay [s/veh]	48.43											
Intersection LOS	D											
Intersection V/C	0.901											

Sequence

Ring 1	1	2	-	4	-	8	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#17: Willow Rd (SR 114)/Hamilton Ave**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 29.2
 Level Of Service: C
 Volume to Capacity (v/c): 0.665

Intersection Setup

Name	Willow Road (SR 114)			Willow Road (SR 114)			Hamilton Avenue			Hamilton Avenue		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	↵↵↵			↵↵↵			⊕			⊕		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	0	0	0	0	0	0
Pocket Length [ft]	190.00	100.00	100.00	115.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	40.00			30.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road (SR 114)			Willow Road (SR 114)			Hamilton Avenue			Hamilton Avenue		
Base Volume Input [veh/h]	50	1299	3	13	623	30	163	15	39	139	17	61
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	3.50	33.30	7.70	3.50	0.00	0.60	26.70	5.10	0.70	5.90	1.60
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	50	1299	3	13	623	30	163	15	39	139	17	61
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	13	335	1	3	161	8	42	4	10	36	4	16
Total Analysis Volume [veh/h]	52	1339	3	13	642	31	168	15	40	143	18	63
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	3			1			5			3		
Bicycle Volume [bicycles/h]	4			12			3			3		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	9.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	5	2	2	1	6	6	4	4	4	8	8	8
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	4	16	16	4	16	16	16	16	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	26	88	88	8	70	70	34	34	34	34	34	34
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	0	1	1	0	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	no		no	no			no			no	
Pedestrian Recall	no	no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	6.0	6.0	20.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	2.00	2.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	22	84	4	66	30	30
g / C, Green / Cycle	0.17	0.65	0.03	0.51	0.23	0.23
(v / s)_i Volume / Saturation Flow Rate	0.03	0.38	0.01	0.19	0.23	0.17
Total Saturation Flow Adjustment	0.93	0.92	0.88	0.91	0.52	0.68
s, saturation flow rate [veh/h]	1770	3494	1676	3471	981	1295
c, Capacity [veh/h]	299	2258	52	1762	226	299
d1, Uniform Delay [s]	46.22	13.21	61.54	19.54	49.78	46.51
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	1.26	1.16	11.38	0.63	56.19	15.83
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.17	0.59	0.25	0.38	0.99	0.75
d, Delay for Lane Group [s/veh]	47.48	14.37	72.92	20.17	105.97	62.34
Lane Group LOS	D	B	E	C	F	E
Critical Lane Group	no	yes	yes	no	yes	no
50th-Percentile Queue Length [veh]	1.74	16.97	0.52	8.65	11.63	9.14
50th-Percentile Queue Length [ft]	43.50	424.25	13.01	216.24	290.79	228.60
95th-Percentile Queue Length [veh]	4.01	27.72	1.30	15.37	19.75	16.10
95th-Percentile Queue Length [ft]	100.32	693.05	32.53	384.33	493.66	402.47

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	47.48	14.37	14.37	72.92	20.17	20.17	105.97	105.97	105.97	62.34	62.34	62.34
Movement LOS	D	B	B	E	C	C	F	F	F	E	E	E
d_A, Approach Delay [s/veh]	15.61			21.17			105.97			62.34		
Approach LOS	B			C			F			E		
d_I, Intersection Delay [s/veh]	29.24											
Intersection LOS	C											
Intersection V/C	0.665											

Sequence

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#18: Willow Rd (SR 114)/Ivy Dr**

Control Type:	Signalized	Delay (sec / veh):	14.4
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.552

Intersection Setup

Name	Willow Road (SR 114)		Willow Road (SR 114)		Ivy Drive	
Approach	Northbound		Southbound		Eastbound	
Lane Configuration						
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	0	0	0
Pocket Length [ft]	85.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	40.00		40.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Willow Road (SR 114)		Willow Road (SR 114)		Ivy Drive	
Base Volume Input [veh/h]	68	1339	885	15	33	141
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.50	3.30	2.80	0.00	0.00	2.10
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	68	1339	885	15	33	141
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	18	352	233	4	9	37
Total Analysis Volume [veh/h]	72	1409	932	16	35	148
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	12		23		13	
Bicycle Volume [bicycles/h]	7		6		0	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	9.00

Phasing & Timing

Control Type	Protected	Permissive	Permissive	Permissive	Split	Split
Signal Group	5	2	6	6	4	4
Auxiliary Signal Groups						
Lead / Lag	Lead	-	-	-	Lag	-
Minimum Green [s]	4	4	4	4	4	4
Maximum Green [s]	4	24	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	21	109	88	88	21	21
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	0	1	1	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no	no	no		no	
Maximum Recall	no	no	no		no	
Pedestrian Recall	no	no	no		no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	6.0	6.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	17	105	84	17
g / C, Green / Cycle	0.13	0.81	0.65	0.13
(v / s)_i Volume / Saturation Flow Rate	0.04	0.40	0.27	0.11
Total Saturation Flow Adjustment	0.94	0.92	0.92	0.86
s, saturation flow rate [veh/h]	1778	3502	3510	1642
c, Capacity [veh/h]	233	2829	2268	215
d1, Uniform Delay [s]	51.18	4.02	11.15	55.27
k, delay calibration	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	3.43	0.63	0.57	32.48
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.31	0.50	0.42	0.85
d, Delay for Lane Group [s/veh]	54.62	4.65	11.72	87.75
Lane Group LOS	D	A	B	F
Critical Lane Group	no	yes	no	yes
50th-Percentile Queue Length [veh]	2.59	10.49	9.90	8.41
50th-Percentile Queue Length [ft]	64.78	262.37	247.48	210.29
95th-Percentile Queue Length [veh]	5.69	18.08	17.21	15.02
95th-Percentile Queue Length [ft]	142.23	451.95	430.14	375.57

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	54.62	4.65	11.72	11.72	87.75	87.75
Movement LOS	D	A	B	B	F	F
d_A, Approach Delay [s/veh]	7.08		11.72		87.75	
Approach LOS	A		B		F	
d_I, Intersection Delay [s/veh]	14.42					
Intersection LOS	B					
Intersection V/C	0.552					

Sequence




Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#19: Willow Rd (SR 114)/O'Brien Dr**

Control Type:	Signalized	Delay (sec / veh):	16.1
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.568

Intersection Setup

Name	Willow Road (SR 114)		Willow Road (SR 114)		O'Brien Drive	
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	1	1	0	1	0
Pocket Length [ft]	100.00	60.00	100.00	100.00	50.00	100.00
Speed [mph]	40.00		40.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	no		no		yes	

Volumes

Name	Willow Road (SR 114)		Willow Road (SR 114)		O'Brien Drive	
Base Volume Input [veh/h]	1368	217	72	966	218	44
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.90	6.50	2.80	2.70	1.80	6.80
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	1368	217	72	966	218	44
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	353	56	19	249	56	11
Total Analysis Volume [veh/h]	1410	224	74	996	225	45
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		0		3	
Bicycle Volume [bicycles/h]	6		9		3	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	9.00

Phasing & Timing

Control Type	Permissive	Permissive	Protected	Permissive	Split	Split
Signal Group	2	2	1	6	8	8
Auxiliary Signal Groups						
Lead / Lag	-	-	Lead	-	Lag	-
Minimum Green [s]	4	4	4	4	4	4
Maximum Green [s]	16	16	4	24	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	100	100	9	109	21	21
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	0	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no		no	no	no	
Maximum Recall	no		no	no	no	
Pedestrian Recall	no		no	no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	6.0	6.0	20.0	6.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	L	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	96	96	5	105	17
g / C, Green / Cycle	0.74	0.74	0.04	0.81	0.13
(v / s)_i Volume / Saturation Flow Rate	0.40	0.15	0.04	0.28	0.09
Total Saturation Flow Adjustment	0.93	0.80	0.92	0.93	0.83
s, saturation flow rate [veh/h]	3516	1516	1756	3522	3170
c, Capacity [veh/h]	2596	1120	68	2845	415
d1, Uniform Delay [s]	7.42	5.22	62.50	3.35	53.68
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.82	0.40	138.20	0.34	7.73
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.54	0.20	1.10	0.35	0.65
d, Delay for Lane Group [s/veh]	8.24	5.62	200.70	3.69	61.41
Lane Group LOS	A	A	F	A	E
Critical Lane Group	yes	no	yes	no	yes
50th-Percentile Queue Length [veh]	13.80	2.88	4.57	6.11	5.73
50th-Percentile Queue Length [ft]	345.04	72.02	114.29	152.75	143.37
95th-Percentile Queue Length [veh]	22.96	6.23	9.15	11.58	11.00
95th-Percentile Queue Length [ft]	573.89	155.71	228.67	289.41	274.93

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	8.24	5.62	200.70	3.69	61.41	61.41
Movement LOS	A	A	F	A	E	E
d_A, Approach Delay [s/veh]	7.88		17.32		61.41	
Approach LOS	A		B		E	
d_I, Intersection Delay [s/veh]	16.14					
Intersection LOS	B					
Intersection V/C	0.568					

Sequence

Ring 1	1	2	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#20: Willow Rd (SR 114)/Newbridge St**

Control Type:	Signalized	Delay (sec / veh):	42.2
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.771

Intersection Setup

Name	Willow Road (SR 114)			Willow Road (SR 114)			Newbridge Street			Newbridge Street		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	1	0	1	0	0	0
Pocket Length [ft]	390.00	100.00	100.00	185.00	100.00	100.00	100.00	100.00	175.00	100.00	100.00	100.00
Speed [mph]	40.00			25.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road (SR 114)			Willow Road (SR 114)			Newbridge Street			Newbridge Street		
Base Volume Input [veh/h]	301	1484	263	74	1016	22	28	137	210	210	249	76
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.30	4.40	5.30	0.00	3.40	0.00	0.00	4.40	0.50	3.80	4.40	1.30
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	175	0	0	45
Total Hourly Volume [veh/h]	301	1484	263	74	1016	22	28	137	35	210	249	31
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	78	382	68	19	262	6	7	35	9	54	64	8
Total Analysis Volume [veh/h]	310	1530	271	76	1047	23	29	141	36	216	257	32
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	28			47			11			1		
Bicycle Volume [bicycles/h]	0			4			1			2		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	12.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Split	Split	Split	Split	Split	Split
Signal Group	5	2	2	1	6	6	4	4	4	8	8	8
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	4	16	16	4	16	16	16	16	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	32	79	79	13	60	60	18	18	18	20	20	20
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	0	1	1	0	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	no		no	no			no			no	
Pedestrian Recall	no	no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	6.0	6.0	20.0	6.0	6.0	20.0	20.0	20.0	20.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	L	C	R	L	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	28	75	9	56	14	14	14	16	16	16
g / C, Green / Cycle	0.22	0.58	0.07	0.43	0.11	0.11	0.11	0.12	0.12	0.12
(v / s)_i Volume / Saturation Flow Rate	0.17	0.37	0.04	0.31	0.02	0.08	0.02	0.06	0.14	0.02
Total Saturation Flow Adjustment	0.94	0.85	0.95	0.92	0.95	0.96	0.85	0.89	0.96	0.84
s, saturation flow rate [veh/h]	1782	4846	1805	3487	1805	1820	1607	3377	1820	1594
c, Capacity [veh/h]	384	2796	125	1502	194	196	173	416	224	196
d1, Uniform Delay [s]	48.44	18.52	58.79	30.38	52.60	56.10	52.94	53.40	57.00	51.01
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	16.54	1.16	20.07	2.90	1.62	20.29	2.71	4.59	105.61	1.78
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.81	0.64	0.61	0.71	0.15	0.72	0.21	0.52	1.15	0.16
d, Delay for Lane Group [s/veh]	64.99	19.67	78.86	33.29	54.22	76.39	55.65	57.99	162.61	52.79
Lane Group LOS	E	B	E	C	D	E	E	E	F	D
Critical Lane Group	yes	no	no	yes	no	yes	no	no	yes	no
50th-Percentile Queue Length [veh]	13.13	18.66	3.16	19.48	1.03	5.98	1.30	4.28	15.92	1.13
50th-Percentile Queue Length [ft]	328.30	466.38	79.04	487.08	25.79	149.38	32.49	107.12	397.91	28.14
95th-Percentile Queue Length [veh]	21.96	30.30	6.74	31.57	2.49	11.37	3.08	8.67	26.13	2.70
95th-Percentile Queue Length [ft]	549.02	757.38	168.46	789.22	62.25	284.22	77.03	216.87	653.15	67.50

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	64.99	19.67	19.67	78.86	33.29	33.29	54.22	76.39	55.65	57.99	162.61	52.79
Movement LOS	E	B	B	E	C	C	D	E	E	E	F	D
d_A, Approach Delay [s/veh]	26.33			36.31			69.64			110.90		
Approach LOS	C			D			E			F		
d_I, Intersection Delay [s/veh]	42.22											
Intersection LOS	D											
Intersection V/C	0.771											

Sequence

Ring 1	1	2	-	4	-	8	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#21: Willow Rd/Bay Rd**

Control Type:	Signalized	Delay (sec / veh):	27.0
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.813

Intersection Setup

Name	Willow Road		Willow Road		Bay Road	
Approach	Northeastbound		Southwestbound		Southeastbound	
Lane Configuration	⇐		⇐		⇐⇐	
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	0	1	0
Pocket Length [ft]	80.00	100.00	100.00	100.00	175.00	100.00
Speed [mph]	30.00		35.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	no		no		yes	

Volumes

Name	Willow Road		Willow Road		Bay Road	
Base Volume Input [veh/h]	35	1276	922	261	504	68
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	2.60	2.20	0.00	1.00	1.50
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	223	0	47
Total Hourly Volume [veh/h]	35	1276	922	38	504	21
Peak Hour Factor	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	10	354	256	11	140	6
Total Analysis Volume [veh/h]	39	1418	1024	42	560	23
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		0		7	
Bicycle Volume [bicycles/h]	8		7		3	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	100
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	12.00

Phasing & Timing

Control Type	Protected	Permissive	Permissive	Permissive	Split	Split
Signal Group	5	2	6	6	4	4
Auxiliary Signal Groups						
Lead / Lag	Lead	-	-	-	Lag	-
Minimum Green [s]	4	4	4	4	4	4
Maximum Green [s]	4	24	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	9	65	56	56	35	35
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	0	1	1	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no	no	no		no	
Maximum Recall	no	no	no		no	
Pedestrian Recall	no	no	no		no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	6.0	6.0	6.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	C	R	L	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	5	61	52	52	31	31
g / C, Green / Cycle	0.05	0.61	0.52	0.52	0.31	0.31
(v / s)_i Volume / Saturation Flow Rate	0.02	0.40	0.29	0.03	0.31	0.01
Total Saturation Flow Adjustment	0.95	0.93	0.93	0.85	0.94	0.84
s, saturation flow rate [veh/h]	1805	3526	3540	1615	1787	1591
c, Capacity [veh/h]	90	2151	1841	840	554	493
d1, Uniform Delay [s]	46.12	12.72	16.21	11.83	34.50	24.15
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	14.37	1.60	1.22	0.11	40.95	0.18
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.43	0.66	0.56	0.05	1.01	0.05
d, Delay for Lane Group [s/veh]	60.49	14.32	17.43	11.94	75.45	24.33
Lane Group LOS	E	B	B	B	E	C
Critical Lane Group	no	yes	no	no	yes	no
50th-Percentile Queue Length [veh]	1.22	15.96	11.56	0.63	23.48	0.48
50th-Percentile Queue Length [ft]	30.53	399.01	288.88	15.81	587.11	12.10
95th-Percentile Queue Length [veh]	2.91	26.19	19.63	1.57	37.79	1.21
95th-Percentile Queue Length [ft]	72.76	654.81	490.86	39.22	944.73	30.34

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	60.49	14.32	17.43	11.94	75.45	24.33
Movement LOS	E	B	B	B	E	C
d_A, Approach Delay [s/veh]	15.56		17.21		73.44	
Approach LOS	B		B		E	
d_I, Intersection Delay [s/veh]	26.99					
Intersection LOS	C					
Intersection V/C	0.813					

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Intersection Level Of Service Report
#22: Willow Rd/Durham St-VA Med Entrance

Control Type:	Signalized	Delay (sec / veh):	22.3
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.686

Intersection Setup

Name	Willow Road			Willow Road			VA Medical Center			Durham Street		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	0	0	0	1	0	0
Pocket Length [ft]	90.00	100.00	100.00	170.00	100.00	100.00	100.00	100.00	100.00	50.00	100.00	100.00
Speed [mph]	25.00			25.00			10.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road			Willow Road			VA Medical Center			Durham Street		
Base Volume Input [veh/h]	8	935	10	54	719	29	117	2	28	21	2	97
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	50.00	2.10	0.00	0.00	2.60	27.60	4.30	0.00	17.90	0.00	0.00	6.20
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	18	0	0	0
Total Hourly Volume [veh/h]	8	935	10	54	719	29	117	2	10	21	2	97
Peak Hour Factor	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	2	254	3	15	195	8	32	1	3	6	1	26
Total Analysis Volume [veh/h]	9	1016	11	59	782	32	127	2	11	23	2	105
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	15			17			10			19		
Bicycle Volume [bicycles/h]	5			3			1			1		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	100
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	5	2	2	1	6	6	4	4	4	8	8	8
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	5	4	4	5	4	4	4	4	4	4	4	4
Maximum Green [s]	30	16	16	30	16	16	16	16	16	16	16	16
Amber [s]	3.0	3.5	3.5	3.0	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	1.0	0.5	0.5	1.0	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	9	74	74	11	76	76	15	15	15	15	15	15
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	0	1	1	0	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	no		no	no			no			no	
Pedestrian Recall	no	no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	6.0	6.0	6.0	20.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	R	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	2.00	0.00	2.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	5	70	7	72	11	11	11	11
g / C, Green / Cycle	0.05	0.70	0.07	0.72	0.11	0.11	0.11	0.11
(v / s)_i Volume / Saturation Flow Rate	0.01	0.29	0.03	0.44	0.13	0.01	0.03	0.07
Total Saturation Flow Adjustment	0.63	0.93	0.95	0.97	0.54	0.72	0.47	0.85
s, saturation flow rate [veh/h]	1203	3538	1805	1841	1017	1370	898	1620
c, Capacity [veh/h]	60	2476	126	1325	112	151	99	178
d1, Uniform Delay [s]	45.47	6.34	44.71	7.03	44.50	39.93	40.65	42.41
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	5.19	0.51	11.89	2.14	132.25	0.94	5.45	14.07
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.15	0.41	0.47	0.61	1.15	0.07	0.23	0.60
d, Delay for Lane Group [s/veh]	50.66	6.85	56.60	9.16	176.75	40.86	46.09	56.47
Lane Group LOS	D	A	E	A	F	D	D	E
Critical Lane Group	yes	no	no	yes	yes	no	no	no
50th-Percentile Queue Length [veh]	0.27	7.37	1.82	13.65	6.99	0.30	0.66	3.35
50th-Percentile Queue Length [ft]	6.73	184.24	45.54	341.26	174.66	7.50	16.42	83.85
95th-Percentile Queue Length [veh]	0.69	13.48	4.18	22.73	12.91	0.76	1.63	7.08
95th-Percentile Queue Length [ft]	17.16	336.99	104.50	568.27	322.65	19.05	40.67	177.03

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	50.66	6.85	6.85	56.60	9.16	9.16	176.75	176.75	40.86	46.09	56.47	56.47
Movement LOS	D	A	A	E	A	A	F	F	D	D	E	E
d_A, Approach Delay [s/veh]	7.24			12.37			166.08			54.64		
Approach LOS	A			B			F			D		
d_I, Intersection Delay [s/veh]	22.33											
Intersection LOS	C											
Intersection V/C	0.686											

Sequence

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#23: Willow Rd/Coleman Ave**

Control Type:	Signalized	Delay (sec / veh):	14.1
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.644

Intersection Setup

Name	Willow Road			Willow Road			Coleman Avenue					
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	0	0	0	0	0	0
Pocket Length [ft]	85.00	100.00	100.00	115.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	25.00			30.00			30.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road			Willow Road			Coleman Avenue					
Base Volume Input [veh/h]	11	738	3	5	680	84	102	4	31	1	1	4
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	3.10	0.00	0.00	3.70	2.40	3.90	0.00	3.20	0.00	0.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	11	738	3	5	680	84	102	4	31	1	1	4
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	3	196	1	1	181	22	27	1	8	0	0	1
Total Analysis Volume [veh/h]	12	785	3	5	723	89	109	4	33	1	1	4
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	12			3			1			9		
Bicycle Volume [bicycles/h]	16			10			7			1		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	126
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	2	2	2	6	6	6	4	4	4	8	8	8
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	16	16	16	16	16	16	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	107	107	107	107	107	107	19	19	19	19	19	19
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	1	1	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no			no			no			no	
Maximum Recall		no			no			no			no	
Pedestrian Recall		no			no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	6.0	6.0	6.0	6.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	2.00	2.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	103	103	103	103	15	15
g / C, Green / Cycle	0.82	0.82	0.82	0.82	0.12	0.12
(v / s)_i Volume / Saturation Flow Rate	0.02	0.43	0.01	0.45	0.11	0.00
Total Saturation Flow Adjustment	0.31	0.97	0.32	0.95	0.69	0.88
s, saturation flow rate [veh/h]	584	1842	604	1802	1311	1680
c, Capacity [veh/h]	477	1506	494	1473	156	200
d1, Uniform Delay [s]	2.14	3.67	2.12	3.82	55.02	49.07
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.10	1.30	0.04	1.49	56.57	0.28
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.03	0.52	0.01	0.55	0.94	0.03
d, Delay for Lane Group [s/veh]	2.24	4.97	2.15	5.31	111.59	49.35
Lane Group LOS	A	A	A	A	F	D
Critical Lane Group	no	no	no	yes	yes	no
50th-Percentile Queue Length [veh]	0.10	10.87	0.04	11.71	7.19	0.20
50th-Percentile Queue Length [ft]	2.51	271.63	1.03	292.72	179.69	5.00
95th-Percentile Queue Length [veh]	0.26	18.62	0.11	19.86	13.21	0.51
95th-Percentile Queue Length [ft]	6.48	465.53	2.67	496.50	330.19	12.81

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	2.24	4.97	4.97	2.15	5.31	5.31	111.59	111.59	111.59	49.35	49.35	49.35
Movement LOS	A	A	A	A	A	A	F	F	F	D	D	D
d_A, Approach Delay [s/veh]	4.93			5.29			111.59			49.35		
Approach LOS	A			A			F			D		
d_I, Intersection Delay [s/veh]	14.05											
Intersection LOS	B											
Intersection V/C	0.644											

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#24: Willow Rd/Gilbert Ave**

Control Type:	Signalized	Delay (sec / veh):	25.6
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.574

Intersection Setup

Name	Willow Road			Willow Road			Gilbert Avenue			Gilbert Avenue		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	↵↵			↵↵			↵↵			↵↵		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	1	0	0	1	0	0
Pocket Length [ft]	75.00	100.00	100.00	90.00	100.00	100.00	55.00	100.00	100.00	90.00	100.00	100.00
Speed [mph]	25.00			25.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road			Willow Road			Gilbert Avenue			Gilbert Avenue		
Base Volume Input [veh/h]	30	671	13	26	597	6	61	51	69	56	43	28
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	3.00	0.00	0.00	2.70	0.00	3.30	2.00	10.10	0.00	2.30	3.60
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	30	671	13	26	597	6	61	51	69	56	43	28
Peak Hour Factor	0.8900	0.8900	0.8900	0.8900	0.8900	0.8900	0.8900	0.8900	0.8900	0.8900	0.8900	0.8900
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	8	188	4	7	168	2	17	14	19	16	12	8
Total Analysis Volume [veh/h]	34	754	15	29	671	7	69	57	78	63	48	31
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	2			4			3			3		
Bicycle Volume [bicycles/h]	15			12			5			4		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	126
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	2	2	2	6	6	6	4	4	4	8	8	8
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	16	16	16	16	16	16	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	112	112	112	112	112	112	14	14	14	14	14	14
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	1	1	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no			no			no			no	
Maximum Recall		no			no			no			no	
Pedestrian Recall		no			no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	L	C	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	2.00	0.00	2.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	108	108	108	108	10	10	10	10
g / C, Green / Cycle	0.86	0.86	0.86	0.86	0.08	0.08	0.08	0.08
(v / s)_i Volume / Saturation Flow Rate	0.05	0.42	0.04	0.37	0.06	0.08	0.08	0.05
Total Saturation Flow Adjustment	0.38	0.97	0.34	0.97	0.59	0.90	0.40	0.92
s, saturation flow rate [veh/h]	724	1839	649	1847	1126	1701	760	1748
c, Capacity [veh/h]	621	1577	556	1583	89	135	60	139
d1, Uniform Delay [s]	1.35	2.21	1.35	2.03	56.88	58.00	58.00	55.92
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.17	1.08	0.18	0.85	46.85	77.41	128.86	15.86
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.05	0.49	0.05	0.43	0.77	1.00	1.04	0.57
d, Delay for Lane Group [s/veh]	1.52	3.29	1.52	2.88	103.74	135.40	186.86	71.79
Lane Group LOS	A	A	A	A	F	F	F	E
Critical Lane Group	no	yes	no	no	no	no	yes	no
50th-Percentile Queue Length [veh]	0.24	8.47	0.20	6.83	3.08	7.17	3.65	3.12
50th-Percentile Queue Length [ft]	5.96	211.67	5.11	170.75	76.89	179.35	91.16	77.96
95th-Percentile Queue Length [veh]	0.61	15.10	0.52	12.67	6.58	13.19	7.59	6.66
95th-Percentile Queue Length [ft]	15.21	377.59	13.09	316.76	164.58	329.68	189.82	166.52

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	1.52	3.29	3.29	1.52	2.88	2.88	103.74	135.40	135.40	186.86	71.79	71.79
Movement LOS	A	A	A	A	A	A	F	F	F	F	E	E
d_A, Approach Delay [s/veh]	3.22			2.82			124.69			122.84		
Approach LOS	A			A			F			F		
d_I, Intersection Delay [s/veh]	25.57											
Intersection LOS	C											
Intersection V/C	0.574											

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#25: Middlefield Rd-Willow Rd**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 46.8
 Level Of Service: D
 Volume to Capacity (v/c): 0.561

Intersection Setup

Name	Willow Road			Willow Road			Middlefield Road			Middlefield Road		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	⇌⇌⇌			⇌⇌⇌			⇌⇌⇌			⇌⇌⇌		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	1	1	0	1	1	0	1	1	0	0
Pocket Length [ft]	75.00	100.00	215.00	155.00	100.00	160.00	125.00	100.00	70.00	270.00	100.00	100.00
Speed [mph]	30.00			25.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Willow Road			Willow Road			Middlefield Road			Middlefield Road		
Base Volume Input [veh/h]	46	94	256	356	93	332	92	429	216	236	453	29
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.20	1.10	0.00	1.70	0.00	2.40	1.10	0.50	2.30	6.40	0.00	3.40
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	120	0	0	1000	0	0	1000	0	0	0
Total Hourly Volume [veh/h]	46	94	136	356	93	0	92	429	0	236	453	29
Peak Hour Factor	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	12	24	35	93	24	0	24	112	0	61	118	8
Total Analysis Volume [veh/h]	48	98	142	371	97	0	96	447	0	246	472	30
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	5			0			0			6		
Bicycle Volume [bicycles/h]	51			1			5			16		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Split	Split	Split	Split	Split	Split	Split
Signal Group	8	8	8	4	7	4	6	6	6	2	5	2
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	-	-	-	Lag	-	-	-	-	-
Minimum Green [s]	4	4	4	0	5	0	4	4	4	0	5	0
Maximum Green [s]	16	16	16	0	30	0	16	16	16	0	30	0
Amber [s]	3.5	3.5	3.5	0.0	3.0	0.0	3.5	3.5	3.5	0.0	3.0	0.0
All red [s]	0.5	0.5	0.5	0.0	1.0	0.0	0.5	0.5	0.5	0.0	1.0	0.0
Split [s]	25	25	25	0	32	0	32	32	32	0	41	0
Vehicle Extension [s]	3.0	3.0	3.0	0.0	3.0	0.0	3.0	3.0	3.0	0.0	3.0	0.0
Walk [s]	1	1	1	0	1	0	1	1	1	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	0.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	0.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0	0.0
Minimum Recall		no			no			no			no	
Maximum Recall		no			no			no			no	
Pedestrian Recall		no			no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	0.0	20.0	0.0	0.0	0.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	R	C	R	L	C	R	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	21	21	21	28	28	28	28	28	37	37
g / C, Green / Cycle	0.16	0.16	0.16	0.22	0.22	0.22	0.22	0.22	0.28	0.28
(v / s)_i Volume / Saturation Flow Rate	0.03	0.05	0.09	0.13	0.00	0.05	0.12	0.00	0.15	0.14
Total Saturation Flow Adjustment	0.93	0.99	0.85	0.92	0.83	0.94	0.95	0.83	0.89	0.94
s, saturation flow rate [veh/h]	1766	1879	1615	3480	1577	1785	3600	1579	1696	3585
c, Capacity [veh/h]	285	304	261	749	340	385	775	340	483	1020
d1, Uniform Delay [s]	46.97	48.21	50.10	46.23	40.02	42.29	45.69	40.02	38.91	38.68
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	1.27	2.80	7.93	3.90	0.00	1.55	3.11	0.00	3.81	1.70
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.17	0.32	0.54	0.62	0.00	0.25	0.58	0.00	0.51	0.49
d, Delay for Lane Group [s/veh]	48.24	51.01	58.04	50.14	40.02	43.84	48.80	40.02	42.71	40.38
Lane Group LOS	D	D	E	D	D	D	D	D	D	D
Critical Lane Group	no	no	yes	yes	no	no	yes	no	yes	no
50th-Percentile Queue Length [veh]	1.62	3.43	5.37	9.25	0.00	3.12	8.62	0.00	8.33	8.82
50th-Percentile Queue Length [ft]	40.44	85.82	134.34	231.19	0.00	78.12	215.47	0.00	208.19	220.52
95th-Percentile Queue Length [veh]	3.76	7.22	10.43	16.25	0.00	6.67	15.33	0.00	14.90	15.62
95th-Percentile Queue Length [ft]	93.97	180.50	260.81	406.27	0.00	166.80	383.19	0.00	372.47	390.61

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	48.24	51.01	58.04	50.14	50.14	40.02	43.84	48.80	40.02	42.71	40.38	40.38
Movement LOS	D	D	E	D	D	D	D	D	D	D	D	D
d_A, Approach Delay [s/veh]	54.01			50.14			47.92			41.15		
Approach LOS	D			D			D			D		
d_I, Intersection Delay [s/veh]	46.81											
Intersection LOS	D											
Intersection V/C	0.561											

Sequence

Ring 1	-	5	-	6	-	7	-	8	-	-	-	-	-	-	-	-
Ring 2	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#26: Ravenswood Ave/Laurel St**

Control Type:	Signalized	Delay (sec / veh):	23.0
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.972

Intersection Setup

Name	Ravenswood Avenue			Ravenswood Avenue			Laurel Street			Laurel Street		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	1	0	0	0	0	0
Pocket Length [ft]	85.00	100.00	100.00	75.00	100.00	100.00	95.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Ravenswood Avenue			Ravenswood Avenue			Laurel Street			Laurel Street		
Base Volume Input [veh/h]	57	596	125	32	523	24	267	215	64	52	135	26
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	1.00	0.00	0.00	2.10	4.20	0.40	0.50	0.00	13.50	0.70	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	57	596	125	32	523	24	267	215	64	52	135	26
Peak Hour Factor	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	15	160	34	9	141	6	72	58	17	14	36	7
Total Analysis Volume [veh/h]	61	641	134	34	562	26	287	231	69	56	145	28
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	35			40			67			58		
Bicycle Volume [bicycles/h]	1			5			20			7		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	60
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	2	2	2	6	6	6	8	8	8	4	4	4
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	16	16	16	16	16	16	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	39	39	39	39	39	39	21	21	21	21	21	21
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	1	1	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no			no		no	no			no	
Maximum Recall		no			no		no	no			no	
Pedestrian Recall		no			no		no	no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	6.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	L	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	2.00	0.00	2.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	35	35	35	35	17	17	17
g / C, Green / Cycle	0.58	0.58	0.58	0.58	0.28	0.28	0.28
(v / s)_i Volume / Saturation Flow Rate	0.09	0.42	0.08	0.32	0.29	0.16	0.16
Total Saturation Flow Adjustment	0.34	0.96	0.21	0.97	0.52	0.96	0.76
s, saturation flow rate [veh/h]	646	1832	406	1849	991	1825	1438
c, Capacity [veh/h]	377	1069	237	1078	281	517	407
d1, Uniform Delay [s]	5.75	9.03	5.68	7.64	21.50	18.44	18.33
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.92	4.29	1.27	1.98	59.53	4.69	5.52
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

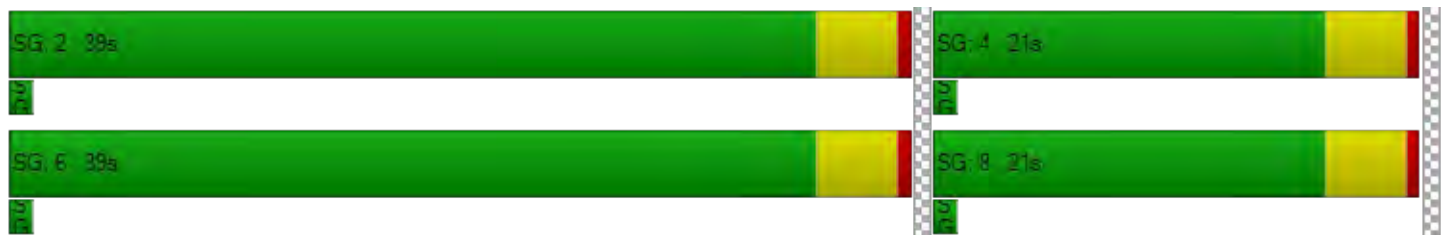
X, volume / capacity	0.16	0.73	0.14	0.55	1.02	0.58	0.56
d, Delay for Lane Group [s/veh]	6.67	13.32	6.96	9.62	81.03	23.13	23.84
Lane Group LOS	A	B	A	A	F	C	C
Critical Lane Group	no	yes	no	no	yes	no	no
50th-Percentile Queue Length [veh]	0.55	11.57	0.31	7.06	8.76	5.02	3.83
50th-Percentile Queue Length [ft]	13.79	289.15	7.75	176.41	218.88	125.43	95.68
95th-Percentile Queue Length [veh]	1.38	19.65	0.79	13.01	15.53	9.87	7.90
95th-Percentile Queue Length [ft]	34.41	491.25	19.70	325.28	388.20	246.68	197.60

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	6.67	13.32	13.32	6.96	9.62	9.62	81.03	23.13	23.13	23.84	23.84	23.84
Movement LOS	A	B	B	A	A	A	F	C	C	C	C	C
d_A, Approach Delay [s/veh]	12.83			9.47			51.44			23.84		
Approach LOS	B			A			D			C		
d_I, Intersection Delay [s/veh]	22.99											
Intersection LOS	C											
Intersection V/C	0.972											

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#28: Oak Grove Ave/Laurel St**

Control Type:	Signalized	Delay (sec / veh):	15.4
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.716

Intersection Setup

Name	Oak Grove Avenue			Oak Grove Avenue			Laurel Street			Laurel Street		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	+			+			+			+		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	25.00			25.00			25.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Oak Grove Avenue			Oak Grove Avenue			Laurel Street			Laurel Street		
Base Volume Input [veh/h]	13	335	94	30	262	48	78	232	38	28	116	29
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	3.30	7.40	0.00	0.80	0.00	0.00	0.40	2.60	0.00	0.00	6.90
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	13	335	94	30	262	48	78	232	38	28	116	29
Peak Hour Factor	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	4	95	27	9	74	14	22	66	11	8	33	8
Total Analysis Volume [veh/h]	15	381	107	34	298	55	89	264	43	32	132	33
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	13			1			18			5		
Bicycle Volume [bicycles/h]	16			12			20			5		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	60
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	2	2	2	6	6	6	8	8	8	4	4	4
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lag	-	-	Lag	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	16	16	16	16	16	16	16	16	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	36	36	36	36	36	36	24	24	24	24	24	24
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	1	1	1	1	1	1	1	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no			no			no			no	
Maximum Recall		no			no			no			no	
Pedestrian Recall		no			no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	2.00	2.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	32	32	20	20
g / C, Green / Cycle	0.53	0.53	0.33	0.33
(v / s)_i Volume / Saturation Flow Rate	0.29	0.22	0.24	0.12
Total Saturation Flow Adjustment	0.93	0.92	0.87	0.89
s, saturation flow rate [veh/h]	1763	1739	1653	1691
c, Capacity [veh/h]	940	927	551	564
d1, Uniform Delay [s]	9.14	8.40	17.54	15.09
k, delay calibration	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	2.18	1.38	7.87	1.70
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.53	0.42	0.72	0.35
d, Delay for Lane Group [s/veh]	11.32	9.79	25.40	16.80
Lane Group LOS	B	A	C	B
Critical Lane Group	yes	no	yes	no
50th-Percentile Queue Length [veh]	6.41	4.45	7.14	2.78
50th-Percentile Queue Length [ft]	160.13	111.27	178.52	69.62
95th-Percentile Queue Length [veh]	12.03	8.95	13.14	6.05
95th-Percentile Queue Length [ft]	300.68	223.72	328.43	151.27

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	11.32	11.32	11.32	9.79	9.79	9.79	25.40	25.40	25.40	16.80	16.80	16.80
Movement LOS	B	B	B	A	A	A	C	C	C	B	B	B
d_A, Approach Delay [s/veh]	11.32			9.79			25.40			16.80		
Approach LOS	B			A			C			B		
d_I, Intersection Delay [s/veh]	15.41											
Intersection LOS	B											
Intersection V/C	0.716											

Sequence

Ring 1	-	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Intersection Level Of Service Report

#29: El Camino Real (SR 82)/Encinal Ave-Menlo College Entrance

Control Type:	Signalized	Delay (sec / veh):	19.5
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.782

Intersection Setup

Name	Menlo College			Encinal Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	⇌			⇌			⇌⇌⇌			⇌⇌⇌		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	1	0	1	1	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	140.00	100.00	150.00	135.00	100.00	100.00
Speed [mph]	15.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			no			yes		

Volumes

Name	Menlo College			Encinal Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	19	15	31	107	9	238	24	1750	100	65	1215	13
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	0.00	1.30	0.00	1.40	0.00	1.50	1.20	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	101	0	0	0
Total Hourly Volume [veh/h]	19	15	31	107	9	238	24	1750	0	65	1215	13
Peak Hour Factor	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	5	4	8	27	2	61	6	446	0	17	310	3
Total Analysis Volume [veh/h]	19	15	32	109	9	243	24	1786	0	66	1240	13
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0			12			0			17		
Bicycle Volume [bicycles/h]	3			3			14			3		

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	8	0	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	0	5	0	0	5	0	5	5	0	5	5	0
Maximum Green [s]	0	30	0	0	30	0	30	30	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	0	40	0	0	40	0	10	85	0	11	86	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
Walk [s]	0	1	0	0	1	0	0	1	0	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	no		no	no	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	50.0	50.0	0.0	50.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	C	R	L	C	R	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	36	36	36	36	6	81	81	7	82
g / C, Green / Cycle	0.26	0.26	0.26	0.26	0.04	0.60	0.60	0.05	0.60
(v / s)_i Volume / Saturation Flow Rate	0.02	0.02	0.09	0.15	0.01	0.50	0.00	0.04	0.25
Total Saturation Flow Adjustment	0.85	0.85	0.72	0.84	0.95	0.94	0.85	0.94	0.90
s, saturation flow rate [veh/h]	1619	1615	1367	1594	1805	3568	1615	1778	5106
c, Capacity [veh/h]	429	427	362	422	80	2125	962	92	3079
d1, Uniform Delay [s]	37.55	37.51	40.24	43.38	62.97	22.27	11.12	63.54	14.21
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.36	0.34	2.39	5.62	9.47	4.22	0.00	38.83	0.40
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.33	1.33	1.33	1.33	1.33
PF, progression factor	1.00	1.00	1.00	1.00	1.00	0.59	0.59	1.00	0.57

Lane Group Results

X, volume / capacity	0.08	0.07	0.33	0.58	0.30	0.84	0.00	0.72	0.41
d, Delay for Lane Group [s/veh]	37.92	37.85	42.62	49.00	72.44	17.27	6.52	102.36	8.48
Lane Group LOS	D	D	D	D	E	B	A	F	A
Critical Lane Group	no	no	no	yes	no	yes	no	yes	no
50th-Percentile Queue Length [veh]	1.04	0.97	3.95	9.04	0.98	28.63	0.00	3.06	6.19
50th-Percentile Queue Length [ft]	25.93	24.37	98.63	226.08	24.47	715.67	0.00	76.56	154.85
95th-Percentile Queue Length [veh]	2.50	2.36	8.10	15.95	2.37	45.90	0.00	6.56	11.70
95th-Percentile Queue Length [ft]	62.56	59.05	202.62	398.78	59.28	1147.40	0.00	163.99	292.62

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	37.92	37.92	37.85	42.62	42.62	49.00	72.44	17.27	6.52	102.36	8.48	8.48
Movement LOS	D	D	D	D	D	D	E	B	A	F	A	A
d_A, Approach Delay [s/veh]	37.88			46.92			18.00			13.17		
Approach LOS	D			D			B			B		
d_I, Intersection Delay [s/veh]	19.51											
Intersection LOS	B											
Intersection V/C	0.782											

Sequence

Ring 1	1	2	4	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	8	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Intersection Level Of Service Report

#30: El Camino Real (SR 82)/Glenwood Ave-Valparaiso Ave

Control Type:	Signalized	Delay (sec / veh):	38.6
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.855

Intersection Setup

Name	Valparaiso Avenue			Glenwood Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	⇌⇌⇌			⇌			⇌⇌⇌			⇌⇌⇌		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	1	1	0	0	1	0	1	1	0	0
Pocket Length [ft]	205.00	100.00	130.00	120.00	100.00	100.00	190.00	100.00	105.00	180.00	100.00	100.00
Speed [mph]	30.00			25.00			35.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Valparaiso Avenue			Glenwood Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	363	144	79	59	171	41	98	1525	46	56	992	304
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	5.60	0.00	0.00	0.60	0.00	1.00	0.50	0.00	1.80	1.70	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	59	0	0	0	0	0	45	0	0	274
Total Hourly Volume [veh/h]	363	144	20	59	171	41	98	1525	1	56	992	30
Peak Hour Factor	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	95	38	5	15	45	11	26	397	0	15	258	8
Total Analysis Volume [veh/h]	378	150	21	61	178	43	102	1589	1	58	1033	31
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	2			9			17			3		
Bicycle Volume [bicycles/h]	6			5			4			9		

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	121.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Split	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	8	0	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	0	5	0	0	5	0	5	5	0	5	5	0
Maximum Green [s]	0	30	0	0	30	0	30	30	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	0	28	0	0	22	0	17	76	0	10	69	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
Walk [s]	0	1	0	0	1	0	0	1	0	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	no		no	no	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	50.0	0.0	0.0	50.0	0.0	50.0	50.0	0.0	50.0	50.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	L	C	L	C	R	L	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	24	24	18	18	13	72	72	6	65	65
g / C, Green / Cycle	0.18	0.18	0.13	0.13	0.10	0.53	0.53	0.04	0.48	0.48
(v / s)_i Volume / Saturation Flow Rate	0.16	0.01	0.03	0.12	0.06	0.44	0.00	0.03	0.29	0.02
Total Saturation Flow Adjustment	0.87	0.85	0.95	0.97	0.94	0.95	0.85	0.93	0.94	0.85
s, saturation flow rate [veh/h]	3307	1615	1805	1834	1787	3600	1615	1773	3557	1615
c, Capacity [veh/h]	584	285	239	243	171	1906	855	78	1700	772
d1, Uniform Delay [s]	54.88	46.73	52.98	58.21	58.99	26.96	15.07	64.23	26.12	18.90
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	19.97	0.50	2.56	38.59	14.46	4.47	0.00	47.00	1.62	0.10
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.33	1.33	1.33	1.33	1.33	1.33
PF, progression factor	1.00	1.00	1.00	1.00	1.00	0.72	0.72	1.00	0.80	0.80

Lane Group Results

X, volume / capacity	0.90	0.07	0.26	0.91	0.60	0.83	0.00	0.74	0.61	0.04
d, Delay for Lane Group [s/veh]	74.85	47.23	55.55	96.79	73.45	23.86	10.84	111.24	22.50	15.20
Lane Group LOS	E	D	E	F	E	C	B	F	C	B
Critical Lane Group	yes	no	no	yes	no	yes	no	yes	no	no
50th-Percentile Queue Length [veh]	13.58	0.71	2.26	11.06	4.25	28.50	0.01	2.75	14.22	0.49
50th-Percentile Queue Length [ft]	339.54	17.81	56.49	276.54	106.29	712.42	0.32	68.78	355.46	12.23
95th-Percentile Queue Length [veh]	22.63	1.76	5.05	18.91	8.62	45.69	0.03	5.99	23.58	1.23
95th-Percentile Queue Length [ft]	565.71	43.93	126.34	472.73	215.48	1142.25	0.83	149.72	589.43	30.67

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	74.85	74.85	47.23	55.55	96.79	96.79	73.45	23.86	10.84	111.24	22.50	15.20
Movement LOS	E	E	D	E	F	F	E	C	B	F	C	B
d_A, Approach Delay [s/veh]	73.79			87.87			26.84			26.89		
Approach LOS	E			F			C			C		
d_I, Intersection Delay [s/veh]	38.65											
Intersection LOS	D											
Intersection V/C	0.855											

Sequence

Ring 1	1	2	4	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#31: El Camino Real (SR 82)/Oak Grove Ave**

Control Type:	Signalized	Delay (sec / veh):	31.9
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.755

Intersection Setup

Name	Oak Grove Avenue			Oak Grove Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	⇌⇌⇌			⇌⇌⇌			⇌⇌⇌			⇌⇌⇌		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	1	1	0	1	1	0	1	1	0	1
Pocket Length [ft]	90.00	100.00	110.00	115.00	100.00	110.00	280.00	100.00	110.00	200.00	100.00	140.00
Speed [mph]	25.00			25.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Oak Grove Avenue			Oak Grove Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	118	202	133	129	215	77	87	1426	94	114	1047	97
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	1.50	0.00	1.60	1.40	1.30	0.00	1.70	0.00	2.60	1.20	1.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	77	0	0	82	0	0	77	0	0	116
Total Hourly Volume [veh/h]	118	202	56	129	215	0	87	1426	17	114	1047	0
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	30	52	14	33	55	0	22	368	4	29	270	0
Total Analysis Volume [veh/h]	122	208	58	133	222	0	90	1470	18	118	1079	0
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	11			7			37			17		
Bicycle Volume [bicycles/h]	10			10			4			6		

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	98.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	7	4	0	3	8	0	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	5	5	0	5	5	0	5	5	0	5	5	0
Maximum Green [s]	30	30	0	30	30	0	30	30	0	30	30	0
Amber [s]	3.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
All red [s]	1.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	19	30	0	13	24	0	16	77	0	16	77	0
Vehicle Extension [s]	3.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
Walk [s]	0	1	0	0	1	0	0	1	0	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	2.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
Minimum Recall	no	no		no	no		no	no		no	no	
Maximum Recall	no	no		no	no		no	no		no	no	
Pedestrian Recall	no	no		no	no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	50.0	50.0	0.0	50.0	50.0	0.0	50.0	50.0	0.0	50.0	50.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	R	L	C	R	L	C	R	L	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	15	26	26	9	20	20	12	73	73	12	73	73
g / C, Green / Cycle	0.11	0.19	0.19	0.07	0.15	0.15	0.09	0.54	0.54	0.09	0.54	0.54
(v / s)_i Volume / Saturation Flow Rate	0.07	0.11	0.04	0.07	0.12	0.00	0.05	0.41	0.01	0.07	0.30	0.00
Total Saturation Flow Adjustment	0.95	0.99	0.85	0.94	0.99	0.84	0.95	0.94	0.85	0.93	0.94	0.84
s, saturation flow rate [veh/h]	1805	1872	1615	1777	1874	1594	1805	3557	1615	1759	3575	1599
c, Capacity [veh/h]	199	358	309	118	276	234	159	1909	867	155	1919	858
d1, Uniform Delay [s]	57.73	50.05	46.14	63.50	56.12	49.47	59.50	24.87	14.76	60.59	20.90	14.59
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	13.30	6.74	1.34	122.63	21.70	0.00	13.72	3.06	0.04	28.97	1.20	0.00
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.67	1.67	1.67	1.33	1.33	1.33
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	0.94	0.23	0.23	1.00	0.71	0.71

Lane Group Results

X, volume / capacity	0.61	0.58	0.19	1.13	0.81	0.00	0.57	0.77	0.02	0.76	0.56	0.00
d, Delay for Lane Group [s/veh]	71.03	56.79	47.49	186.13	77.82	49.47	69.38	8.71	3.40	89.57	15.96	10.31
Lane Group LOS	E	E	D	F	E	D	E	A	A	F	B	B
Critical Lane Group	yes	no	no	no	yes	no	no	yes	no	yes	no	no
50th-Percentile Queue Length [veh]	5.12	8.13	1.99	8.56	10.09	0.00	3.68	14.30	0.10	5.39	12.10	0.00
50th-Percentile Queue Length [ft]	128.08	203.24	49.81	213.93	252.35	0.00	91.99	357.38	2.55	134.67	302.38	0.00
95th-Percentile Queue Length [veh]	10.04	14.61	4.53	15.24	17.49	0.00	7.65	23.69	0.26	10.45	20.43	0.00
95th-Percentile Queue Length [ft]	250.89	365.17	113.14	380.92	437.27	0.00	191.25	592.30	6.59	261.33	510.72	0.00

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	71.03	56.79	47.49	186.13	77.82	49.47	69.38	8.71	3.40	89.57	15.96	10.31
Movement LOS	E	E	D	F	E	D	E	A	A	F	B	B
d_A, Approach Delay [s/veh]	59.87			118.39			12.11			23.22		
Approach LOS	E			F			B			C		
d_I, Intersection Delay [s/veh]	31.88											
Intersection LOS	C											
Intersection V/C	0.755											

Sequence

Ring 1	1	2	3	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	7	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#32: El Camino Real (SR 82)/Santa Cruz Ave**

Control Type:	Signalized	Delay (sec / veh):	13.7
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.730

Intersection Setup

Name	Santa Cruz Avenue			Santa Cruz Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	↵↵↵			↵↵			↵↵			↵↵		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	1	1	0	0	0	0	1	0	0	1
Pocket Length [ft]	140.00	100.00	140.00	50.00	100.00	100.00	100.00	100.00	280.00	100.00	100.00	105.00
Speed [mph]	30.00			25.00			35.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Santa Cruz Avenue			Santa Cruz Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	172	50	137	50	58	43	0	1406	41	0	1451	87
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.70	10.00	0.70	2.00	5.20	7.00	0.00	1.40	2.40	0.00	1.20	1.70
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	73	0	0	99
Total Hourly Volume [veh/h]	172	50	137	50	58	43	0	1406	0	0	1451	0
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	1.0000	0.9700	0.9700	1.0000	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	44	13	35	13	15	11	0	362	0	0	374	0
Total Analysis Volume [veh/h]	177	52	141	52	60	44	0	1449	0	0	1496	0
Presence of On-Street Parking	no		no	no		no			no			no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	20			27			66			41		
Bicycle Volume [bicycles/h]	14			12			4			0		

Intersection Settings

Located in CBD	yes
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	92.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Split	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	0	4	0	0	8	0	0	6	0	0	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	-	-	-	-	-	-
Minimum Green [s]	0	5	0	0	5	0	0	5	0	0	5	0
Maximum Green [s]	0	30	0	0	30	0	0	30	0	0	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	0.0	1.0	0.0	0.0	1.0	0.0
Split [s]	0	25	0	0	17	0	0	94	0	0	94	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	1	0	0	1	0	0	1	0	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Minimum Recall		no			no			no			no	
Maximum Recall		no			no			no			no	
Pedestrian Recall		no			no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	R	L	C	C	R	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	21	21	21	13	13	90	90	90	90
g / C, Green / Cycle	0.15	0.15	0.15	0.10	0.10	0.66	0.66	0.66	0.66
(v / s)_i Volume / Saturation Flow Rate	0.11	0.03	0.10	0.03	0.07	0.45	0.00	0.46	0.00
Total Saturation Flow Adjustment	0.84	0.82	0.76	0.84	0.80	0.84	0.75	0.85	0.75
s, saturation flow rate [veh/h]	1597	1555	1443	1593	1522	3211	1419	3217	1429
c, Capacity [veh/h]	247	240	223	152	146	2125	939	2129	946
d1, Uniform Delay [s]	54.68	50.30	53.89	57.50	59.70	14.18	7.78	14.54	7.78
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	16.42	2.06	12.90	6.01	25.80	1.79	0.00	1.97	0.00
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.45	1.45	1.45	1.45
PF, progression factor	1.00	1.00	1.00	1.00	1.00	0.12	0.12	0.12	0.12

Lane Group Results

X, volume / capacity	0.72	0.22	0.63	0.34	0.71	0.68	0.00	0.70	0.00
d, Delay for Lane Group [s/veh]	71.10	52.37	66.79	63.51	85.50	3.46	0.91	3.68	0.91
Lane Group LOS	E	D	E	E	F	A	A	A	A
Critical Lane Group	yes	no	no	no	yes	no	no	yes	no
50th-Percentile Queue Length [veh]	7.63	1.87	5.84	2.04	4.67	6.59	0.00	7.21	0.00
50th-Percentile Queue Length [ft]	190.71	46.82	145.99	51.12	116.63	164.86	0.00	180.27	0.00
95th-Percentile Queue Length [veh]	13.86	4.28	11.16	4.63	9.30	12.31	0.00	13.24	0.00
95th-Percentile Queue Length [ft]	346.61	107.11	279.00	115.76	232.49	307.86	0.00	331.05	0.00

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	71.10	52.37	66.79	63.51	85.50	85.50	0.00	3.46	0.91	0.00	3.68	0.91
Movement LOS	E	D	E	E	F	F		A	A		A	A
d_A, Approach Delay [s/veh]	66.83			78.17				3.46			3.68	
Approach LOS	E			E				A			A	
d_I, Intersection Delay [s/veh]	13.66											
Intersection LOS	B											
Intersection V/C	0.730											

Sequence

Ring 1	-	2	-	-	4	-	8	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Intersection Level Of Service Report

#33: El Camino Real (SR 82)/Ravenswood Ave-Menlo Ave

Control Type:	Signalized	Delay (sec / veh):	44.6
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.872

Intersection Setup

Name	Menlo Avenue			Ravenswood Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	TT			TTT			TTT			TTT		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	1	0	1	1	0	0	1	0	1
Pocket Length [ft]	100.00	100.00	100.00	240.00	100.00	135.00	320.00	100.00	100.00	240.00	100.00	65.00
Speed [mph]	30.00			30.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			no			yes		

Volumes

Name	Menlo Avenue			Ravenswood Avenue			El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	44	348	108	451	249	80	113	1339	590	180	1154	39
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	4.50	1.10	0.00	0.40	0.40	1.30	0.90	1.80	1.70	0.60	1.40	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	80	0	0	454	0	0	33
Total Hourly Volume [veh/h]	44	348	108	451	249	0	113	1339	136	180	1154	6
Peak Hour Factor	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800	0.9800
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	11	89	28	115	64	0	29	342	35	46	294	2
Total Analysis Volume [veh/h]	45	355	110	460	254	0	115	1366	139	184	1178	6
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	31			12			0			37		
Bicycle Volume [bicycles/h]	13			14			5			0		

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	109.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Split	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	8	8	8	4	4	4	1	6	6	5	2	2
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	16	16	16	16	4	16	16	4	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	25	25	25	27	27	27	17	64	64	20	67	67
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	1	1	1	1	0	1	1	0	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	no		no	no	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	6.0	6.0	6.0	20.0	20.0	20.0	20.0	6.0	6.0	20.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	L	C	R	L	C	R	L	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	21	23	23	23	13	60	60	16	63	63
g / C, Green / Cycle	0.15	0.17	0.17	0.17	0.10	0.44	0.44	0.12	0.46	0.46
(v / s)_i Volume / Saturation Flow Rate	0.15	0.13	0.13	0.00	0.06	0.38	0.09	0.10	0.33	0.00
Total Saturation Flow Adjustment	0.91	0.92	1.00	0.84	0.94	0.94	0.84	0.94	0.94	0.85
s, saturation flow rate [veh/h]	3447	3491	1892	1594	1789	3554	1588	1794	3568	1615
c, Capacity [veh/h]	532	590	320	270	171	1568	701	211	1653	748
d1, Uniform Delay [s]	57.06	54.07	54.22	46.94	59.44	34.50	23.27	58.99	29.25	19.66
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	29.90	9.79	18.10	0.00	19.13	6.94	0.63	35.77	2.65	0.02
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.33	1.33	1.33	1.67	1.67	1.67
PF, progression factor	1.00	1.00	1.00	1.00	1.00	0.85	0.85	0.91	0.42	0.42

Lane Group Results

X, volume / capacity	0.96	0.78	0.79	0.00	0.67	0.87	0.20	0.87	0.71	0.01
d, Delay for Lane Group [s/veh]	86.97	63.86	72.32	46.94	78.58	36.18	20.36	89.51	15.06	8.36
Lane Group LOS	F	E	E	D	E	D	C	F	B	A
Critical Lane Group	yes	no	yes	no	no	yes	no	yes	no	no
50th-Percentile Queue Length [veh]	13.94	10.47	11.31	0.00	4.96	29.05	2.74	8.88	14.96	0.06
50th-Percentile Queue Length [ft]	348.56	261.70	282.84	0.00	124.02	726.22	68.54	221.94	373.96	1.55
95th-Percentile Queue Length [veh]	23.17	18.04	19.28	0.00	9.78	46.57	5.97	15.71	24.68	0.16
95th-Percentile Queue Length [ft]	579.14	450.97	481.99	0.00	244.42	1164.13	149.28	392.70	617.11	4.01

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	86.97	86.97	86.97	63.86	72.32	46.94	78.58	36.18	20.36	89.51	15.06	8.36
Movement LOS	F	F	F	E	E	D	E	D	C	F	B	A
d_A, Approach Delay [s/veh]	86.97			66.87			37.83			25.05		
Approach LOS	F			E			D			C		
d_I, Intersection Delay [s/veh]	44.55											
Intersection LOS	D											
Intersection V/C	0.872											

Sequence

Ring 1	1	2	-	4	-	8	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#34: El Camino Real (SR 82)/Roble Ave**

Control Type:	Signalized	Delay (sec / veh):	10.4
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.586

Intersection Setup

Name	Roble Avenue						El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	+			+			+			+		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	1	1	0	0	1	0	1
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	200.00	100.00	100.00	115.00	100.00	210.00
Speed [mph]	25.00			15.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			no		

Volumes

Name	Roble Avenue						El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	66	6	36	58	45	48	72	1845	20	68	1586	54
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.50	0.00	0.00	0.00	0.00	0.00	0.00	1.20	0.00	0.00	1.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	39	0	0	0	0	0	0
Total Hourly Volume [veh/h]	66	6	36	58	45	9	72	1845	20	68	1586	54
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	17	2	9	15	12	2	19	476	5	18	409	14
Total Analysis Volume [veh/h]	68	6	37	60	46	9	74	1902	21	70	1635	56
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	11			8			16			0		
Bicycle Volume [bicycles/h]	9			16			5			1		

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	65.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	8	8	8	4	4	4	1	6	6	5	2	2
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lag	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	4	4	4	4	4	4	4	4	4	4	4	4
Maximum Green [s]	16	16	16	16	16	16	4	16	16	4	16	16
Amber [s]	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All red [s]	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
Split [s]	23	23	23	23	23	23	19	98	98	15	94	94
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	1	1	1	1	0	1	1	0	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	no		no	no	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	6.0	6.0	6.0	6.0	6.0	6.0	20.0	6.0	6.0	20.0	6.0	6.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	R	L	C	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	19	19	19	15	94	11	90
g / C, Green / Cycle	0.14	0.14	0.14	0.11	0.69	0.08	0.66
(v / s)_i Volume / Saturation Flow Rate	0.10	0.07	0.01	0.04	0.38	0.04	0.33
Total Saturation Flow Adjustment	0.58	0.76	0.85	0.95	0.90	0.95	0.89
s, saturation flow rate [veh/h]	1093	1441	1615	1805	5106	1805	5099
c, Capacity [veh/h]	153	201	226	199	3529	146	3374
d1, Uniform Delay [s]	56.01	54.32	50.61	56.13	10.40	59.76	11.64
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	25.89	9.52	0.33	5.25	0.61	10.85	0.53
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.33	1.33	1.33	1.33
PF, progression factor	1.00	1.00	1.00	1.00	0.29	1.00	0.40

Lane Group Results

X, volume / capacity	0.73	0.53	0.04	0.37	0.54	0.48	0.50
d, Delay for Lane Group [s/veh]	81.91	63.85	50.94	61.38	3.66	70.62	5.20
Lane Group LOS	F	E	D	E	A	E	A
Critical Lane Group	yes	no	no	no	yes	yes	no
50th-Percentile Queue Length [veh]	4.94	4.25	0.32	2.81	6.34	2.84	6.69
50th-Percentile Queue Length [ft]	123.49	106.19	7.91	70.22	158.50	71.00	167.37
95th-Percentile Queue Length [veh]	9.74	8.61	0.80	6.10	11.93	6.15	12.47
95th-Percentile Queue Length [ft]	243.57	215.31	20.08	152.38	298.21	153.84	311.66

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	81.91	81.91	81.91	63.85	63.85	50.94	61.38	3.66	3.66	70.62	5.20	5.20
Movement LOS	F	F	F	E	E	D	E	A	A	E	A	A
d_A, Approach Delay [s/veh]	81.91			62.84			5.80			7.80		
Approach LOS	F			E			A			A		
d_I, Intersection Delay [s/veh]	10.45											
Intersection LOS	B											
Intersection V/C	0.586											

Sequence

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#35: El Camino Real (SR 82)/Middle Ave**

Control Type:	Signalized	Delay (sec / veh):	19.5
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.682

Intersection Setup

Name	Middle Avenue		El Camino Real (SR 82)		El Camino Real (SR 82)	
Approach	Northeastbound		Northwestbound		Southeastbound	
Lane Configuration						
Turning Movement	Left	Right	Left	Thru	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	1	1	0	0	0
Pocket Length [ft]	100.00	55.00	275.00	100.00	100.00	100.00
Speed [mph]	30.00		30.00		35.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		no		yes	

Volumes

Name	Middle Avenue		El Camino Real (SR 82)		El Camino Real (SR 82)	
Base Volume Input [veh/h]	249	182	323	1917	1334	86
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.40	1.60	0.90	1.70	1.60	1.20
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	211	0	0	0	0
Total Hourly Volume [veh/h]	249	0	323	1917	1334	86
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	64	0	83	494	344	22
Total Analysis Volume [veh/h]	257	0	333	1976	1375	89
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	27		0		17	
Bicycle Volume [bicycles/h]	2		7		4	

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	76.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	12.00

Phasing & Timing

Control Type	Protected	Permissive	Protected	Permissive	Permissive	Permissive
Signal Group	4	0	1	6	2	0
Auxiliary Signal Groups						
Lead / Lag	Lead	-	Lead	-	-	-
Minimum Green [s]	5	0	5	5	5	0
Maximum Green [s]	30	0	30	30	30	0
Amber [s]	3.0	0.0	3.0	3.0	3.0	0.0
All red [s]	1.0	0.0	1.0	1.0	1.0	0.0
Split [s]	32	0	43	104	61	0
Vehicle Extension [s]	3.0	0.0	3.0	3.0	3.0	0.0
Walk [s]	5	0	0	1	1	0
Pedestrian Clearance [s]	10	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	0.0	2.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	2.0	0.0	2.0	2.0	2.0	0.0
Minimum Recall	no		no	no	no	
Maximum Recall	no		no	no	no	
Pedestrian Recall	no		no	no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	50.0	50.0	50.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	R	L	C	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	28	28	39	100	57
g / C, Green / Cycle	0.21	0.21	0.29	0.74	0.42
(v / s)_i Volume / Saturation Flow Rate	0.15	0.00	0.19	0.39	0.29
Total Saturation Flow Adjustment	0.93	0.84	0.94	0.89	0.89
s, saturation flow rate [veh/h]	1763	1590	1789	5089	5048
c, Capacity [veh/h]	363	327	513	3742	2116
d1, Uniform Delay [s]	50.20	42.88	42.50	7.79	32.32
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	11.10	0.00	6.24	0.54	1.89
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.33	1.33	1.33
PF, progression factor	1.00	1.00	1.00	0.09	0.87

Lane Group Results

X, volume / capacity	0.71	0.00	0.65	0.53	0.69
d, Delay for Lane Group [s/veh]	61.30	42.88	48.58	1.21	30.12
Lane Group LOS	E	D	D	A	C
Critical Lane Group	yes	no	yes	no	yes
50th-Percentile Queue Length [veh]	10.65	0.00	11.98	3.21	17.25
50th-Percentile Queue Length [ft]	266.19	0.00	299.58	80.14	431.15
95th-Percentile Queue Length [veh]	18.30	0.00	20.26	6.82	28.14
95th-Percentile Queue Length [ft]	457.55	0.00	506.60	170.43	703.53

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	61.30	42.88	48.58	1.21	30.12	30.12
Movement LOS	E	D	D	A	C	C
d_A, Approach Delay [s/veh]	61.30		8.04		30.12	
Approach LOS	E		A		C	
d_I, Intersection Delay [s/veh]	19.46					
Intersection LOS	B					
Intersection V/C	0.682					

Sequence

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#36: El Camino Real (SR 82)/Cambridge Ave**

Control Type:	Signalized	Delay (sec / veh):	10.4
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.622

Intersection Setup

Name	Cambridge Avenue						El Camino Real (SR 82)			El Camino Real (SR 82)		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	↔↔			+			↔↔↔			↔↔↔		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	1	0	0	0	1	0	0	1	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	370.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	25.00			15.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			no			yes		

Volumes

Name	Cambridge Avenue						El Camino Real (SR 82)			El Camino Real (SR 82)		
Base Volume Input [veh/h]	26	0	39	4	3	0	310	2332	6	30	1660	4
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	2.85	0.00	0.00	0.00	0.00	1.20	0.00	0.00	0.90	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	35	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	26	0	4	4	3	0	310	2332	6	30	1660	4
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	7	0	1	1	1	0	82	620	2	8	441	1
Total Analysis Volume [veh/h]	28	0	4	4	3	0	330	2481	6	32	1766	4
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	1			2			0			3		
Bicycle Volume [bicycles/h]	0			0			17			1		

Intersection Settings

Located in CBD	no
Signal Coordination Group	1 - ECR
Cycle Length [s]	136
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	111.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	8	0	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	0	5	0	0	5	0	5	5	0	5	5	0
Maximum Green [s]	0	30	0	0	30	0	30	30	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	0	9	0	0	9	0	45	118	0	9	82	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	3.0	0.0	3.0	3.0	0.0
Walk [s]	0	1	0	0	1	0	0	1	0	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		no			no		no	no		no	no	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	50.0	0.0	50.0	50.0	0.0	50.0	50.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	R	C	L	C	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	5	5	5	41	114	5	78
g / C, Green / Cycle	0.04	0.04	0.04	0.30	0.84	0.04	0.57
(v / s)_i Volume / Saturation Flow Rate	0.02	0.00	0.00	0.18	0.49	0.02	0.35
Total Saturation Flow Adjustment	0.70	0.83	0.82	0.95	0.90	0.95	0.90
s, saturation flow rate [veh/h]	1336	1570	1564	1805	5112	1805	5128
c, Capacity [veh/h]	53	58	57	544	4285	66	2941
d1, Uniform Delay [s]	64.34	63.25	63.38	40.60	3.47	64.23	18.89
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	33.03	2.31	4.29	4.96	0.58	22.99	0.92
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.33	1.13	1.33	1.33
PF, progression factor	1.00	1.00	1.00	0.98	0.36	1.00	0.63

Lane Group Results

X, volume / capacity	0.53	0.07	0.12	0.61	0.58	0.48	0.60
d, Delay for Lane Group [s/veh]	97.37	65.56	67.67	44.94	1.82	87.22	12.91
Lane Group LOS	F	E	E	D	A	F	B
Critical Lane Group	yes	no	no	yes	no	no	yes
50th-Percentile Queue Length [veh]	1.23	0.16	0.28	11.32	6.67	1.38	13.04
50th-Percentile Queue Length [ft]	30.86	4.03	7.11	282.88	166.64	34.51	325.96
95th-Percentile Queue Length [veh]	2.94	0.41	0.72	19.28	12.42	3.26	21.82
95th-Percentile Queue Length [ft]	73.49	10.36	18.09	482.04	310.56	81.41	545.56

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	97.37	97.37	65.56	67.67	67.67	67.67	44.94	1.82	1.82	87.22	12.91	12.91
Movement LOS	F	F	E	E	E	E	D	A	A	F	B	B
d_A, Approach Delay [s/veh]	93.39			67.67			6.87			14.23		
Approach LOS	F			E			A			B		
d_I, Intersection Delay [s/veh]	10.40											
Intersection LOS	B											
Intersection V/C	0.622											

Sequence

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#38: Santa Cruz Ave/University Dr (S)**

Control Type:	Signalized	Delay (sec / veh):	19.2
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.619

Intersection Setup

Name	Santa Cruz Avenue		Santa Cruz Avenue		University Drive	
Approach	Northeastbound		Southwestbound		Northwestbound	
Lane Configuration	↑		↑		↑	
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	1	1	0	1	0
Pocket Length [ft]	100.00	100.00	60.00	100.00	100.00	100.00
Speed [mph]	30.00		25.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Santa Cruz Avenue		Santa Cruz Avenue		University Drive	
Base Volume Input [veh/h]	350	328	88	398	363	165
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.60	0.90	0.50	2.30	0.00	0.60
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	350	328	88	398	363	165
Peak Hour Factor	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	95	89	24	108	99	45
Total Analysis Volume [veh/h]	380	357	96	433	395	179
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		23		31	
Bicycle Volume [bicycles/h]	10		15		8	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	60
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	12.00

Phasing & Timing

Control Type	Permissive	Permissive	Protected	Permissive	Split	Split
Signal Group	2	2	1	6	8	8
Auxiliary Signal Groups						
Lead / Lag	-	-	Lead	-	Lead	-
Minimum Green [s]	4	4	5	4	4	4
Maximum Green [s]	16	16	30	16	16	16
Amber [s]	3.5	3.5	3.0	3.5	3.5	3.5
All red [s]	0.5	0.5	1.0	0.5	0.5	0.5
Split [s]	24	24	10	34	26	26
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	1	1	0	1	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no		no	no	no	
Maximum Recall	yes		no	yes	no	
Pedestrian Recall	yes		no	yes	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	6.0	6.0	20.0	20.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	L	C	L	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	20	20	6	30	22	22
g / C, Green / Cycle	0.33	0.33	0.10	0.50	0.37	0.37
(v / s)_i Volume / Saturation Flow Rate	0.21	0.22	0.05	0.23	0.22	0.11
Total Saturation Flow Adjustment	0.97	0.84	0.95	0.98	0.95	0.84
s, saturation flow rate [veh/h]	1852	1601	1796	1857	1805	1605
c, Capacity [veh/h]	617	534	180	929	662	589
d1, Uniform Delay [s]	16.78	17.16	25.67	9.78	15.40	13.54
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	4.55	6.54	10.94	1.68	3.94	1.33
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.62	0.67	0.53	0.47	0.60	0.30
d, Delay for Lane Group [s/veh]	21.33	23.70	36.61	11.46	19.34	14.87
Lane Group LOS	C	C	D	B	B	B
Critical Lane Group	no	yes	yes	no	yes	no
50th-Percentile Queue Length [veh]	6.26	6.17	1.81	5.41	6.27	2.38
50th-Percentile Queue Length [ft]	156.61	154.35	45.25	135.27	156.63	59.61
95th-Percentile Queue Length [veh]	11.81	11.67	4.16	10.49	11.81	5.30
95th-Percentile Queue Length [ft]	295.31	291.86	103.92	262.26	295.35	132.39

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	21.33	23.70	36.61	11.46	19.34	14.87
Movement LOS	C	C	D	B	B	B
d_A, Approach Delay [s/veh]	22.47		16.03		17.95	
Approach LOS	C		B		B	
d_I, Intersection Delay [s/veh]	19.21					
Intersection LOS	B					
Intersection V/C	0.619					

Sequence

Ring 1	1	2	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#39: Sand Hill Rd/Santa Cruz Ave**

Control Type:	Signalized	Delay (sec / veh):	49.9
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.709

Intersection Setup

Name	Sand Hill Road			Sand Hill Road			Santa Cruz Avenue			Santa Cruz Avenue		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	T T T			T T T			T T T			T T T		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	2	0	1	2	0	1	2	0	1	2	0	1
Pocket Length [ft]	210.00	100.00	240.00	135.00	100.00	140.00	100.00	100.00	100.00	150.00	100.00	180.00
Speed [mph]	35.00			35.00			35.00			35.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Sand Hill Road			Sand Hill Road			Santa Cruz Avenue			Santa Cruz Avenue		
Base Volume Input [veh/h]	361	611	215	424	1038	208	158	691	188	101	526	178
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.10	0.80	0.90	0.00	0.80	1.00	1.90	0.70	0.50	0.00	0.20	1.70
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	361	611	215	424	1038	208	158	691	188	101	526	178
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	93	157	55	109	268	54	41	178	48	26	136	46
Total Analysis Volume [veh/h]	372	630	222	437	1070	214	163	712	194	104	542	184
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	1			12			5			3		
Bicycle Volume [bicycles/h]	28			9			33			20		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	150
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	16.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Protecte
Signal Group	5	2	2	1	6	6	3	8	8	7	4	4
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	5	4	4	5	4	4	5	4	4	5	4	4
Maximum Green [s]	30	16	16	30	16	16	30	16	16	30	16	16
Amber [s]	3.0	3.5	3.5	3.0	3.5	3.5	3.0	3.5	3.5	3.0	3.5	3.5
All red [s]	1.0	0.5	0.5	1.0	0.5	0.5	1.0	0.5	0.5	1.0	0.5	0.5
Split [s]	24	54	54	42	72	72	16	42	42	12	38	38
Vehicle Extension [s]	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walk [s]	0	1	1	0	1	1	0	1	1	0	1	1
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Minimum Recall	no	no		no	no		no	no		no	no	no
Maximum Recall	no	yes		no	yes		no	no		no	no	no
Pedestrian Recall	no	yes		no	yes		no	no		no	no	no
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	20.0	20.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	R	L	C	R	L	C	R	L	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	20	50	50	38	68	68	12	38	38	8	34	34
g / C, Green / Cycle	0.13	0.33	0.33	0.25	0.45	0.45	0.08	0.25	0.25	0.05	0.23	0.23
(v / s)_i Volume / Saturation Flow Rate	0.11	0.18	0.14	0.12	0.30	0.13	0.05	0.20	0.12	0.03	0.15	0.12
Total Saturation Flow Adjustment	0.91	0.94	0.84	0.92	0.94	0.84	0.91	0.95	0.85	0.92	0.95	0.84
s, saturation flow rate [veh/h]	3467	3589	1601	3505	3589	1599	3440	3592	1607	3505	3610	1588
c, Capacity [veh/h]	462	1196	534	888	1627	725	275	910	407	187	818	360
d1, Uniform Delay [s]	63.10	40.43	38.70	47.77	31.93	25.88	66.64	52.15	47.55	69.27	52.78	50.73
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	13.86	1.66	2.38	1.95	2.10	1.04	9.06	6.66	3.96	11.42	4.20	5.11
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.80	0.53	0.42	0.49	0.66	0.30	0.59	0.78	0.48	0.56	0.66	0.51
d, Delay for Lane Group [s/veh]	76.96	42.09	41.08	49.72	34.03	26.91	75.69	58.81	51.51	80.69	56.97	55.84
Lane Group LOS	E	D	D	D	C	C	E	E	D	F	E	E
Critical Lane Group	yes	no	no	no	yes	no	no	yes	no	yes	no	no
50th-Percentile Queue Length [veh]	9.61	12.42	7.90	8.89	20.86	6.17	3.94	17.38	7.63	2.55	12.42	7.51
50th-Percentile Queue Length [ft]	240.35	310.43	197.39	222.14	521.51	154.23	98.46	434.46	190.86	63.72	310.51	187.79
95th-Percentile Queue Length [veh]	16.79	20.90	14.26	15.72	33.70	11.67	8.09	28.34	13.87	5.61	20.91	13.69
95th-Percentile Queue Length [ft]	419.69	522.59	356.51	393.00	842.46	291.67	202.32	708.58	346.84	140.23	522.71	342.27

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	76.96	42.09	41.08	49.72	34.03	26.91	75.69	58.81	51.51	80.69	56.97	55.84
Movement LOS	E	D	D	D	C	C	E	E	D	F	E	E
d_A, Approach Delay [s/veh]	52.51			37.13			60.06			59.69		
Approach LOS	D			D			E			E		
d_I, Intersection Delay [s/veh]	49.94											
Intersection LOS	D											
Intersection V/C	0.709											

Sequence

Ring 1	1	2	3	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	7	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#58: University Avenue and Adams Drive**

Control Type:	Two-way stop	Delay (sec / veh):	87.6
Analysis Method:	HCM2000	Level Of Service:	F
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.000

Intersection Setup

Name	University Avenue			University Avenue			Adams Drive			Adams Drive		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	270.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	University Avenue			University Avenue			Adams Drive			Adams Drive		
Base Volume Input [veh/h]	11	1786	0	0	360	15	25	0	26	0	0	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	2.40	2.00	2.00	2.50	2.00	4.00	0.00	0.00	2.00	2.00	2.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	11	1786	0	0	360	15	25	0	26	0	0	0
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	3	447	0	0	90	4	6	0	7	0	0	0
Total Analysis Volume [veh/h]	11	1786	0	0	360	15	25	0	26	0	0	0
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings

Priority Scheme	Free	Free	Stop	Stop
Flared Lane			no	
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance			no	
Number of Storage Spaces in Median	0	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.01	0.02	0.00	0.00	0.00	0.00	0.21	0.00	0.03	0.00	0.00	0.00
d_M, Delay for Movement [s/veh]	8.04	0.00	0.00	0.00	0.00	0.00	40.46	87.59	14.65	0.00	0.00	0.00
Movement LOS	A	A			A	A	E	F	B			
95th-Percentile Queue Length [veh]	0.03	0.00	0.00	0.00	0.00	0.00	0.91	0.91	0.91	0.00	0.00	0.00
95th-Percentile Queue Length [ft]	0.70	0.00	0.00	0.00	0.00	0.00	22.76	22.76	22.76	0.00	0.00	0.00
d_A, Approach Delay [s/veh]	0.05			0.00			27.30			0.00		
Approach LOS	A			A			D			A		
d_I, Intersection Delay [s/veh]	0.67											
Intersection LOS	F											

**Intersection Level Of Service Report
#74: University Ave/O'Brien Dr**

Control Type:	Signalized	Delay (sec / veh):	9.7
Analysis Method:	HCM2000	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.710

Intersection Setup

Name	University Avenue				O'Brien Drive	
Approach	Northbound		Southbound		Eastbound	
Lane Configuration						
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	0	1	0
Pocket Length [ft]	90.00	100.00	100.00	100.00	45.00	100.00
Speed [mph]	25.00		25.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	University Avenue				O'Brien Drive	
Base Volume Input [veh/h]	11	1724	356	10	171	77
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	9.10	2.40	2.50	0.00	4.70	1.30
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	11	1724	356	10	171	77
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	3	454	94	3	45	20
Total Analysis Volume [veh/h]	12	1815	375	11	180	81
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		0		0	
Bicycle Volume [bicycles/h]	6		0		1	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	85
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	11.00

Phasing & Timing

Control Type	Protected	Permissive	Permissive	Permissive	Split	Split
Signal Group	5	2	6	0	3	0
Auxiliary Signal Groups						
Lead / Lag	Lead	-	-	-	Lead	-
Minimum Green [s]	5	5	5	0	5	0
Maximum Green [s]	30	30	30	0	30	0
Amber [s]	3.0	3.0	3.0	0.0	3.0	0.0
All red [s]	1.0	1.0	1.0	0.0	1.0	0.0
Split [s]	9	66	57	0	19	0
Vehicle Extension [s]	3.0	3.0	3.0	0.0	3.0	0.0
Walk [s]	0	1	1	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	no	no	no		no	
Maximum Recall	yes	yes	yes		yes	
Pedestrian Recall	no	yes	yes		yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	C	L	R
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	7	64	55	17	17
g / C, Green / Cycle	0.08	0.75	0.65	0.20	0.20
(v / s)_i Volume / Saturation Flow Rate	0.01	0.51	0.11	0.10	0.05
Total Saturation Flow Adjustment	0.87	0.93	0.92	0.91	0.84
s, saturation flow rate [veh/h]	1654	3533	3514	1724	1594
c, Capacity [veh/h]	136	2660	2274	345	319
d1, Uniform Delay [s]	36.05	5.34	5.95	30.37	28.66
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	1.27	1.44	0.16	5.56	1.91
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.09	0.68	0.17	0.52	0.25
d, Delay for Lane Group [s/veh]	37.32	6.77	6.11	35.93	30.57
Lane Group LOS	D	A	A	D	C
Critical Lane Group	no	yes	no	yes	no
50th-Percentile Queue Length [veh]	0.29	14.27	2.15	4.35	1.78
50th-Percentile Queue Length [ft]	7.20	356.75	53.79	108.76	44.47
95th-Percentile Queue Length [veh]	0.73	23.65	4.84	8.78	4.09
95th-Percentile Queue Length [ft]	18.32	591.35	121.04	219.57	102.31

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	37.32	6.77	6.11	6.11	35.93	30.57
Movement LOS	D	A	A	A	D	C
d_A, Approach Delay [s/veh]	6.97		6.11		34.27	
Approach LOS	A		A		C	
d_I, Intersection Delay [s/veh]	9.72					
Intersection LOS	A					
Intersection V/C	0.710					

Sequence

Ring 1	-	2	3	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#88: Valparaiso Ave/ University Dr**

Control Type:	Signalized	Delay (sec / veh):	31.5
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.731

Intersection Setup

Name	Valparaiso Ave						University Drive (North)					
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	↵↵			↵↵			⊕			↵↵		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	0	0	0	1	0	0
Pocket Length [ft]	100.00	100.00	100.00	75.00	100.00	100.00	100.00	100.00	100.00	35.00	100.00	100.00
Speed [mph]	35.00			35.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Valparaiso Ave						University Drive (North)					
Base Volume Input [veh/h]	24	467	105	44	456	29	182	24	57	45	41	72
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	1.90	0.00	0.00	1.80	0.00	0.50	8.30	1.80	0.00	7.30	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	24	467	105	44	456	29	182	24	57	45	41	72
Peak Hour Factor	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	6	122	27	11	119	8	47	6	15	12	11	19
Total Analysis Volume [veh/h]	25	486	109	46	475	30	190	25	59	47	43	75
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	22			1			13			11		
Bicycle Volume [bicycles/h]	11			7			1			8		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	85
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	4.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	3	8	0	7	4	0	0	2	0	0	6	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	-	-	-	-	-	-
Minimum Green [s]	5	5	0	5	5	0	0	5	0	0	5	0
Maximum Green [s]	30	30	0	30	30	0	0	30	0	0	30	0
Amber [s]	3.0	3.0	0.0	3.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
All red [s]	1.0	1.0	0.0	1.0	1.0	0.0	0.0	1.0	0.0	0.0	1.0	0.0
Split [s]	9	37	0	9	37	0	0	39	0	0	39	0
Vehicle Extension [s]	3.0	3.0	0.0	3.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	1	0	0	1	0	0	1	0	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	0.0	2.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	2.0	2.0	0.0	2.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	yes	yes		yes	yes			yes			yes	
Pedestrian Recall	no	yes		no	yes			yes			yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	0.0	20.0	20.0	0.0	0.0	20.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	L	C
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	2.00	2.00	0.00
l2, Clearance Lost Time [s]	0.00	2.00	0.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	42	33	42	33	35	35	35
g / C, Green / Cycle	0.49	0.39	0.49	0.39	0.41	0.41	0.41
(v / s)_i Volume / Saturation Flow Rate	0.04	0.33	0.08	0.27	0.34	0.04	0.07
Total Saturation Flow Adjustment	0.37	0.95	0.30	0.97	0.42	0.61	0.84
s, saturation flow rate [veh/h]	706	1813	566	1850	799	1165	1602
c, Capacity [veh/h]	349	704	280	718	329	480	660
d1, Uniform Delay [s]	12.63	23.67	14.09	21.88	22.37	15.32	15.88
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.40	11.92	1.26	5.70	21.19	0.41	0.59
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.07	0.85	0.16	0.70	0.83	0.10	0.18
d, Delay for Lane Group [s/veh]	13.03	35.59	15.35	27.57	43.57	15.73	16.47
Lane Group LOS	B	D	B	C	D	B	B
Critical Lane Group	no	yes	no	no	yes	no	no
50th-Percentile Queue Length [veh]	0.35	16.48	0.69	11.96	7.74	0.75	1.95
50th-Percentile Queue Length [ft]	8.75	411.98	17.16	298.93	193.52	18.78	48.69
95th-Percentile Queue Length [veh]	0.89	26.98	1.70	20.23	14.03	1.85	4.44
95th-Percentile Queue Length [ft]	22.16	674.43	42.41	505.63	350.79	46.22	110.88

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	13.03	35.59	35.59	15.35	27.57	27.57	43.57	43.57	43.57	15.73	16.47	16.47
Movement LOS	B	D	D	B	C	C	D	D	D	B	B	B
d_A, Approach Delay [s/veh]	34.68			26.55			43.57			16.26		
Approach LOS	C			C			D			B		
d_I, Intersection Delay [s/veh]	31.52											
Intersection LOS	C											
Intersection V/C	0.731											

Sequence

Ring 1	2	3	4	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	7	8	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#103: Addison Wesley/Sand Hill Rd**

Control Type: Signalized
Analysis Method: HCM2000
Analysis Period: 15 minutes

Delay (sec / veh): 81.9
Level Of Service: F
Volume to Capacity (v/c): 0.650

Intersection Setup

Name	Addison Wesley			Addison Wesley			Sand Hill Rd			Sand Hill Rd		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	⇐⇐⇐			⇐⇐			⇐⇐⇐			⇐⇐		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	1	0	1	1	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	230.00	100.00	230.00	240.00	100.00	100.00
Speed [mph]	25.00			25.00			40.00			40.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			no			yes			yes		

Volumes

Name	Addison Wesley			Addison Wesley			Sand Hill Rd			Sand Hill Rd		
Base Volume Input [veh/h]	209	3	108	19	3	66	18	794	93	69	1750	16
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.50	33.30	3.70	5.30	0.00	0.00	11.10	1.50	3.20	7.20	1.40	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	209	3	108	19	3	66	18	794	93	69	1750	16
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	54	1	28	5	1	17	5	205	24	18	451	4
Total Analysis Volume [veh/h]	215	3	111	20	3	68	19	819	96	71	1804	16
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0			0			0			3		
Bicycle Volume [bicycles/h]	1			0			17			19		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	60.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Split	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	8	0	0	4	0	5	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lag	-	-	Lead	-	-
Minimum Green [s]	0	6	0	0	6	0	4	10	0	4	10	0
Maximum Green [s]	0	0	0	0	0	0	0	0	0	0	0	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	4.0	0.0	3.0	4.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	0	13	0	0	13	0	12	52	0	12	52	0
Vehicle Extension [s]	0.0	2.9	0.0	0.0	3.0	0.0	3.0	6.0	0.0	2.0	6.0	0.0
Walk [s]	0	7	0	0	0	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	30	0	0	0	0	0	24	0	0	19	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no			no		no	yes		no	yes	
Maximum Recall		no			no		no	no		no	no	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	0.0	0.0	20.0	0.0	20.0	20.0	0.0	20.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	L	C	L	C	R	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	11	11	11	11	10	50	50	10	50
g / C, Green / Cycle	0.10	0.10	0.10	0.10	0.09	0.44	0.44	0.09	0.44
(v / s)_i Volume / Saturation Flow Rate	0.08	0.07	0.01	0.04	0.01	0.23	0.06	0.04	0.51
Total Saturation Flow Adjustment	0.68	0.81	0.90	0.86	0.86	0.94	0.81	0.89	0.94
s, saturation flow rate [veh/h]	2586	1536	1714	1627	1625	3564	1534	1684	3563
c, Capacity [veh/h]	250	148	165	157	143	1563	673	148	1563
d1, Uniform Delay [s]	50.81	50.16	47.08	48.65	48.00	23.32	19.16	49.53	32.00
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	31.92	28.88	1.49	9.13	1.93	1.26	0.44	10.78	81.50
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.87	0.75	0.12	0.45	0.13	0.52	0.14	0.48	1.16
d, Delay for Lane Group [s/veh]	82.73	79.03	48.57	57.78	49.93	24.58	19.61	60.31	113.50
Lane Group LOS	F	E	D	E	D	C	B	E	F
Critical Lane Group	yes	no	no	yes	yes	no	no	no	yes
50th-Percentile Queue Length [veh]	4.93	4.31	0.63	2.42	0.61	11.19	1.99	2.46	53.26
50th-Percentile Queue Length [ft]	123.35	107.77	15.79	60.51	15.21	279.70	49.76	61.45	1331.53
95th-Percentile Queue Length [veh]	9.73	8.72	1.57	5.36	1.51	19.09	4.52	5.44	85.22
95th-Percentile Queue Length [ft]	243.34	217.94	39.17	134.10	37.79	477.37	113.03	135.91	2130.49

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	82.73	82.73	79.03	48.57	57.78	57.78	49.93	24.58	19.61	60.31	113.50	113.50
Movement LOS	F	F	E	D	E	E	D	C	B	E	F	F
d_A, Approach Delay [s/veh]	81.48			55.76			24.59			111.50		
Approach LOS	F			E			C			F		
d_I, Intersection Delay [s/veh]	81.88											
Intersection LOS	F											
Intersection V/C	0.650											

Sequence

Ring 1	1	2	4	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	5	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Intersection Level Of Service Report

#107: Alpine Rd/Santa Cruz Ave&Junipero Serra Blvd

Control Type:	Signalized	Delay (sec / veh):	51.3
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.732

Intersection Setup

Name	Santa Cruz Avenue		Junipero Serra Blvd		Alpine Road	
Approach	Southbound		Westbound		Northeastbound	
Lane Configuration	T T T		T T		T T	
Turning Movement	Left	Thru	Left	Right	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	1	0	0	0
Pocket Length [ft]	210.00	100.00	300.00	100.00	100.00	100.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Santa Cruz Avenue		Junipero Serra Blvd		Alpine Road	
Base Volume Input [veh/h]	373	638	423	398	535	100
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.30	1.10	0.00	0.80
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	373	638	423	398	535	100
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	96	164	109	103	138	26
Total Analysis Volume [veh/h]	385	658	436	410	552	103
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		0		0	
Bicycle Volume [bicycles/h]	29		41		20	

Intersection Settings

Located in CBD	yes
Signal Coordination Group	-
Cycle Length [s]	150
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	10.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Split
Signal Group	0	6	7	0	2	0
Auxiliary Signal Groups						
Lead / Lag	-	-	Lead	-	-	-
Minimum Green [s]	0	5	5	0	5	0
Maximum Green [s]	0	30	30	0	30	0
Amber [s]	0.0	3.0	3.0	0.0	3.0	0.0
All red [s]	0.0	1.0	1.0	0.0	1.0	0.0
Split [s]	0	50	62	0	38	0
Vehicle Extension [s]	0.0	3.0	3.0	0.0	3.0	0.0
Walk [s]	0	1	1	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	2.0	0.0	2.0	0.0
Minimum Recall		no	no		no	
Maximum Recall		yes	yes		yes	
Pedestrian Recall		yes	yes		yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	20.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	L	R	C	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	46	58	58	34	34
g / C, Green / Cycle	0.31	0.39	0.39	0.23	0.23
(v / s)_i Volume / Saturation Flow Rate	0.23	0.27	0.29	0.17	0.07
Total Saturation Flow Adjustment	0.80	0.85	0.76	0.86	0.76
s, saturation flow rate [veh/h]	4574	1620	1438	3256	1442
c, Capacity [veh/h]	1403	626	556	738	327
d1, Uniform Delay [s]	46.70	38.61	39.47	54.01	48.30
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	3.61	6.30	8.49	6.83	2.51
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.74	0.70	0.74	0.75	0.32
d, Delay for Lane Group [s/veh]	50.31	44.90	47.96	60.84	50.82
Lane Group LOS	D	D	D	E	D
Critical Lane Group	yes	no	yes	yes	no
50th-Percentile Queue Length [veh]	16.90	17.69	17.31	13.38	3.91
50th-Percentile Queue Length [ft]	422.60	442.30	432.85	334.38	97.84
95th-Percentile Queue Length [veh]	27.62	28.82	28.24	22.32	8.05
95th-Percentile Queue Length [ft]	690.53	720.54	706.12	558.05	201.27

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	50.31	50.31	44.90	47.96	60.84	50.82
Movement LOS	D	D	D	D	E	D
d_A, Approach Delay [s/veh]	50.31		46.38		59.26	
Approach LOS	D		D		E	
d_I, Intersection Delay [s/veh]	51.31					
Intersection LOS	D					
Intersection V/C	0.732					

Sequence

Ring 1	2	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	-	7	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#110: Marsh Road/101 NB Ramps**

Control Type:	Signalized	Delay (sec / veh):	49.8
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.851

Intersection Setup

Name	Marsh Road		Marsh Road		101 NB Ramps	
Approach	Northbound		Southbound		Northwestbound	
Lane Configuration	↑↑		↑↑		↑↑↑	
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	1	1
Pocket Length [ft]	100.00	100.00	100.00	100.00	500.00	360.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		no		yes	

Volumes

Name	Marsh Road		Marsh Road		101 NB Ramps	
Base Volume Input [veh/h]	2255	0	0	567	544	239
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	4.40	0.00	0.00	3.00	5.10	12.10
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	2255	0	0	567	544	239
Peak Hour Factor	0.9600	1.0000	1.0000	0.9600	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	587	0	0	148	142	62
Total Analysis Volume [veh/h]	2349	0	0	591	567	249
Presence of On-Street Parking	no			no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		0		0	
Bicycle Volume [bicycles/h]	1		2		0	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	100
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	25.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permissive	Permissive	Permissive	Permissive	Protected	Overlap
Signal Group	2	0	0	6	8	1
Auxiliary Signal Groups						1,6,8
Lead / Lag	-	-	-	-	Lag	-
Minimum Green [s]	8	0	0	8	6	4
Maximum Green [s]	0	0	0	0	0	0
Amber [s]	3.6	0.0	0.0	3.6	3.2	3.2
All red [s]	0.5	0.0	0.0	0.5	1.0	0.0
Split [s]	64	0	0	72	28	8
Vehicle Extension [s]	3.0	0.0	0.0	3.0	3.0	2.0
Walk [s]	7	0	0	0	7	0
Pedestrian Clearance [s]	18	0	0	0	20	0
I1, Start-Up Lost Time [s]	2.0	0.0	0.0	2.0	2.0	2.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	yes			yes	no	no
Maximum Recall	no			no	no	no
Pedestrian Recall	no			no	no	no
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	0.0	0.0	20.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	L	R
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	62	70	26	98
g / C, Green / Cycle	0.62	0.70	0.26	0.98
(v / s)_i Volume / Saturation Flow Rate	0.68	0.17	0.17	0.17
Total Saturation Flow Adjustment	0.91	0.92	0.88	0.76
s, saturation flow rate [veh/h]	3465	3512	3335	1441
c, Capacity [veh/h]	2148	2459	867	1412
d1, Uniform Delay [s]	19.00	5.41	32.99	0.02
k, delay calibration	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	50.23	0.23	3.83	0.27
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	1.09	0.24	0.65	0.18
d, Delay for Lane Group [s/veh]	69.23	5.64	36.82	0.30
Lane Group LOS	E	A	D	A
Critical Lane Group	yes	no	no	yes
50th-Percentile Queue Length [veh]	56.66	3.57	8.47	0.50
50th-Percentile Queue Length [ft]	1416.45	89.34	211.80	12.54
95th-Percentile Queue Length [veh]	90.65	7.47	15.11	1.26
95th-Percentile Queue Length [ft]	2266.33	186.66	377.79	31.41

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	69.23	0.00	0.00	5.64	36.82	0.30
Movement LOS	E			A	D	A
d_A, Approach Delay [s/veh]	69.23		5.64		25.67	
Approach LOS	E		A		C	
d_I, Intersection Delay [s/veh]	49.76					
Intersection LOS	D					
Intersection V/C	0.851					

Sequence

Ring 1	-	2	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#132: Oak Ave/Sand Hill Rd**

Control Type:	Signalized	Delay (sec / veh):	6.2
Analysis Method:	HCM2000	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.556

Intersection Setup

Name	Sand Hill Road		Sand Hill Road		Oak Avenue	
Approach	Northeastbound		Southwestbound		Southeastbound	
Lane Configuration			H		TH	
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	1	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	95.00	100.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Sand Hill Road		Sand Hill Road		Oak Avenue	
Base Volume Input [veh/h]	0	912	1585	100	29	88
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	1.10	0.80	0.00	0.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	912	1585	100	29	88
Peak Hour Factor	1.0000	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	235	409	26	7	23
Total Analysis Volume [veh/h]	0	940	1634	103	30	91
Presence of On-Street Parking		no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	23		0		2	
Bicycle Volume [bicycles/h]	16		34		1	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	150
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	3.00

Phasing & Timing

Control Type	Permissive	Permissive	Permissive	Permissive	Split	Split
Signal Group	0	2	6	0	3	0
Auxiliary Signal Groups						
Lead / Lag	-	-	-	-	Lead	-
Minimum Green [s]	0	5	5	0	5	0
Maximum Green [s]	0	30	30	0	30	0
Amber [s]	0.0	3.0	3.0	0.0	3.0	0.0
All red [s]	0.0	1.0	1.0	0.0	1.0	0.0
Split [s]	0	130	130	0	20	0
Vehicle Extension [s]	0.0	3.0	3.0	0.0	3.0	0.0
Walk [s]	0	1	1	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no	no		no	
Maximum Recall		yes	yes		yes	
Pedestrian Recall		yes	yes		yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	20.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	L	R
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	128	128	18	18
g / C, Green / Cycle	0.85	0.85	0.12	0.12
(v / s)_i Volume / Saturation Flow Rate	0.26	0.49	0.02	0.06
Total Saturation Flow Adjustment	0.94	0.94	0.95	0.85
s, saturation flow rate [veh/h]	3578	3557	1805	1615
c, Capacity [veh/h]	3053	3035	217	194
d1, Uniform Delay [s]	2.19	3.15	59.06	61.55
k, delay calibration	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.26	0.79	1.33	7.96
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.31	0.57	0.14	0.47
d, Delay for Lane Group [s/veh]	2.45	3.94	60.39	69.50
Lane Group LOS	A	A	E	E
Critical Lane Group	no	yes	no	yes
50th-Percentile Queue Length [veh]	5.10	13.82	1.21	3.98
50th-Percentile Queue Length [ft]	127.39	345.60	30.21	99.48
95th-Percentile Queue Length [veh]	9.99	22.99	2.88	8.16
95th-Percentile Queue Length [ft]	249.81	574.73	72.05	204.05

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	0.00	2.45	3.94	3.94	60.39	69.50
Movement LOS		A	A	A	E	E
d_A, Approach Delay [s/veh]	2.45		3.94		67.25	
Approach LOS	A		A		E	
d_I, Intersection Delay [s/veh]	6.18					
Intersection LOS	A					
Intersection V/C	0.556					

Sequence

Ring 1	2	3	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#156: Saga Ln/Sand Hill Rd**

Control Type:	Signalized	Delay (sec / veh):	35.2
Analysis Method:	HCM2000	Level Of Service:	D
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.516

Intersection Setup

Name	Saga Ln			Saga Ln			Sand Hill Road			Sand Hill Road		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	↔↔			↔↔			↔↔			↔↔		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	1	0	0	0	1	0	0	1	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	200.00	100.00	100.00	230.00	100.00	100.00
Speed [mph]	25.00			25.00			40.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Saga Ln			Saga Ln			Sand Hill Road			Sand Hill Road		
Base Volume Input [veh/h]	108	0	113	90	0	103	65	945	6	28	1307	20
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	4.60	0.00	2.70	2.20	0.00	1.90	3.10	0.50	0.00	14.30	0.50	5.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	108	0	113	90	0	103	65	945	6	28	1307	20
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	29	0	30	24	0	27	17	251	2	7	348	5
Total Analysis Volume [veh/h]	115	0	120	96	0	110	69	1005	6	30	1390	21
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	3			16			3			0		
Bicycle Volume [bicycles/h]	12			0			22			22		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	20.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	4	0	5	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	0	8	0	0	8	0	4	10	0	6	10	0
Maximum Green [s]	0	0	0	0	0	0	0	0	0	0	0	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	4.0	0.0	3.0	4.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	0	29	0	0	29	0	14	47	0	14	40	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	5.0	0.0	3.0	5.0	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	27	0	0	27	0	0	20	0	0	15	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		yes			yes		no	yes		yes	yes	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	0.0	0.0	20.0	0.0	20.0	20.0	0.0	20.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	R	L	R	L	C	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	27	27	27	27	12	45	12	38
g / C, Green / Cycle	0.28	0.28	0.28	0.28	0.13	0.47	0.13	0.40
(v / s)_i Volume / Saturation Flow Rate	0.08	0.08	0.07	0.07	0.04	0.28	0.02	0.39
Total Saturation Flow Adjustment	0.72	0.81	0.74	0.83	0.92	0.95	0.83	0.94
s, saturation flow rate [veh/h]	1372	1541	1408	1581	1751	3596	1579	3589
c, Capacity [veh/h]	390	438	400	449	221	1703	199	1436
d1, Uniform Delay [s]	26.56	26.39	26.12	26.16	37.75	18.30	36.96	28.18
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	1.92	1.54	1.41	1.29	3.65	1.53	1.59	19.98
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

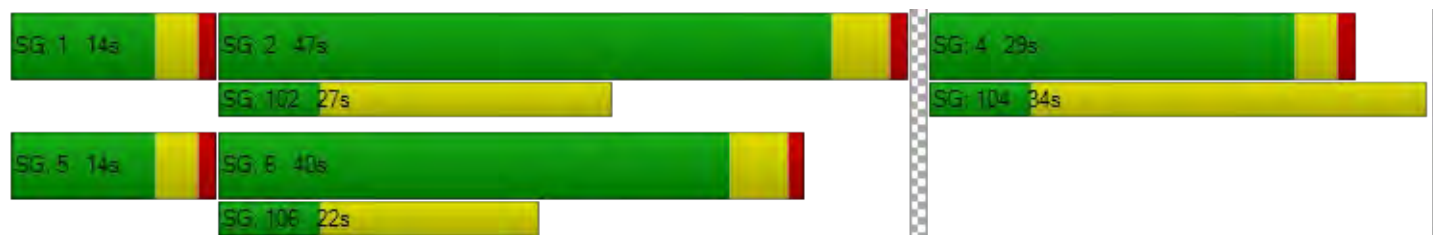
X, volume / capacity	0.29	0.27	0.24	0.24	0.31	0.59	0.15	0.98
d, Delay for Lane Group [s/veh]	28.48	27.93	27.53	27.45	41.39	19.83	38.55	48.15
Lane Group LOS	C	C	C	C	D	B	D	D
Critical Lane Group	yes	no	no	no	yes	no	no	yes
50th-Percentile Queue Length [veh]	2.63	2.71	2.14	2.45	1.84	11.81	0.77	28.05
50th-Percentile Queue Length [ft]	65.64	67.69	53.56	61.29	46.03	295.22	19.32	701.27
95th-Percentile Queue Length [veh]	5.75	5.91	4.82	5.42	4.22	20.01	1.90	44.98
95th-Percentile Queue Length [ft]	143.85	147.69	120.58	135.60	105.51	500.18	47.46	1124.60

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	28.48	28.48	27.93	27.53	27.53	27.45	41.39	19.83	19.83	38.55	48.15	48.15
Movement LOS	C	C	C	C	C	C	D	B	B	D	D	D
d_A, Approach Delay [s/veh]	28.20			27.49			21.21			47.95		
Approach LOS	C			C			C			D		
d_I, Intersection Delay [s/veh]	35.21											
Intersection LOS	D											
Intersection V/C	0.516											

Sequence

Ring 1	1	2	4	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#157: Branner Dr/Sand Hill Rd**

Control Type:	Signalized	Delay (sec / veh):	21.0
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.451

Intersection Setup

Name	Branner Drive						Sand Hill Road					
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	+			+			T T T			T T T		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	1	0	0	1	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	160.00	100.00	100.00	155.00	100.00	100.00
Speed [mph]	25.00			25.00			40.00			40.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Branner Drive						Sand Hill Road					
Base Volume Input [veh/h]	2	0	17	20	0	43	22	1076	7	17	1321	24
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	5.00	0.00	2.30	4.50	1.10	0.00	5.90	2.00	4.20
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	2	0	17	20	0	43	22	1076	7	17	1321	24
Peak Hour Factor	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	1	0	4	5	0	11	6	280	2	4	344	6
Total Analysis Volume [veh/h]	2	0	18	21	0	45	23	1121	7	18	1376	25
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	4			2			0			5		
Bicycle Volume [bicycles/h]	2			0			35			15		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	58.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	4	0	5	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lag	-	-
Minimum Green [s]	0	6	0	0	6	0	4	10	0	4	10	0
Maximum Green [s]	0	3	0	0	3	0	0	0	0	0	0	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	3.0	4.0	0.0	3.0	4.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	1.0	1.0	0.0	1.0	1.0	0.0
Split [s]	0	20	0	0	20	0	12	58	0	12	58	0
Vehicle Extension [s]	0.0	2.9	0.0	0.0	2.9	0.0	2.0	5.0	0.0	2.0	5.0	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	28	0	0	28	0	0	20	0	0	20	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no			no		no	no		no	no	
Maximum Recall		yes			yes		yes	yes		yes	yes	
Pedestrian Recall		no			no		no	no		no	no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	0.0	0.0	20.0	0.0	20.0	20.0	0.0	20.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	L	C	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	18	18	10	56	10	56
g / C, Green / Cycle	0.17	0.17	0.10	0.53	0.10	0.53
(v / s)_i Volume / Saturation Flow Rate	0.01	0.04	0.01	0.32	0.01	0.40
Total Saturation Flow Adjustment	0.86	0.85	0.91	0.94	0.90	0.93
s, saturation flow rate [veh/h]	1625	1609	1727	3574	1704	3536
c, Capacity [veh/h]	279	276	165	1906	162	1886
d1, Uniform Delay [s]	36.49	37.58	43.56	16.71	43.43	18.94
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.50	2.04	1.77	1.36	1.38	2.69
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.07	0.24	0.14	0.59	0.11	0.74
d, Delay for Lane Group [s/veh]	36.99	39.63	45.33	18.06	44.81	21.63
Lane Group LOS	D	D	D	B	D	C
Critical Lane Group	no	yes	yes	no	no	yes
50th-Percentile Queue Length [veh]	0.53	1.82	0.67	13.57	0.52	19.89
50th-Percentile Queue Length [ft]	13.24	45.62	16.84	339.13	13.11	497.36
95th-Percentile Queue Length [veh]	1.32	4.19	1.67	22.60	1.31	32.20
95th-Percentile Queue Length [ft]	33.09	104.66	41.65	565.10	32.78	805.08

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	36.99	36.99	36.99	39.63	39.63	39.63	45.33	18.06	18.06	44.81	21.63	21.63
Movement LOS	D	D	D	D	D	D	D	B	B	D	C	C
d_A, Approach Delay [s/veh]	36.99			39.63			18.61			21.92		
Approach LOS	D			D			B			C		
d_I, Intersection Delay [s/veh]	21.04											
Intersection LOS	C											
Intersection V/C	0.451											

Sequence

Ring 1	2	1	4	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#162: Sharon Park Dr/ Sand Hill Rd**

Control Type: Signalized
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 51.3
 Level Of Service: D
 Volume to Capacity (v/c): 0.684

Intersection Setup

Name	Northeastbound			Sand Hill Road			Northwestbound			Sharon Park Drive		
Approach	Northeastbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	↵↵↵			↵↵↵			⊕			↵↵		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	0	0	0	0	0	0	0	1
Pocket Length [ft]	250.00	100.00	100.00	150.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	130.00
Speed [mph]	40.00			35.00			25.00			25.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Northeastbound			Sand Hill Road			Northwestbound			Sharon Park Drive		
Base Volume Input [veh/h]	164	916	1	16	1220	252	12	6	17	178	3	172
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.60	1.30	100.00	0.00	1.10	0.80	0.00	0.00	17.60	0.00	0.00	0.60
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	164	916	1	16	1220	252	12	6	17	178	3	172
Peak Hour Factor	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500	0.9500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	43	241	0	4	321	66	3	2	4	47	1	45
Total Analysis Volume [veh/h]	173	964	1	17	1284	265	13	6	18	187	3	181
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0			6			6			10		
Bicycle Volume [bicycles/h]	32			32			2			5		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	69.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss
Signal Group	5	2	0	1	6	0	0	4	0	0	4	0
Auxiliary Signal Groups												
Lead / Lag	Lag	-	-	Lead	-	-	-	-	-	-	-	-
Minimum Green [s]	4	10	0	5	10	0	0	6	0	0	6	0
Maximum Green [s]	0	0	0	0	0	0	0	0	0	0	0	0
Amber [s]	3.0	4.0	0.0	3.0	4.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
All red [s]	1.0	1.0	0.0	1.0	0.5	0.0	0.0	1.0	0.0	0.0	1.0	0.0
Split [s]	16	41	0	14	39	0	0	35	0	0	35	0
Vehicle Extension [s]	3.0	7.0	0.0	3.0	7.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	7	0	0	7	0	0	4	0	0	4	0
Pedestrian Clearance [s]	0	19	0	0	19	0	0	27	0	0	27	0
I1, Start-Up Lost Time [s]	2.0	2.0	0.0	2.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	no	no		no	no			no			no	
Maximum Recall	no	yes		no	yes			no			no	
Pedestrian Recall	no	no		no	no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	0.0	20.0	20.0	0.0	0.0	20.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	L	C	C	C	R
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	2.00	2.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	14	39	12	37	33	33	33
g / C, Green / Cycle	0.16	0.43	0.13	0.41	0.37	0.37	0.37
(v / s)_i Volume / Saturation Flow Rate	0.10	0.27	0.01	0.45	0.02	0.14	0.11
Total Saturation Flow Adjustment	0.94	0.94	0.95	0.91	0.85	0.71	0.83
s, saturation flow rate [veh/h]	1794	3570	1805	3461	1620	1352	1584
c, Capacity [veh/h]	279	1547	241	1423	594	496	581
d1, Uniform Delay [s]	35.51	19.80	34.12	26.50	18.47	21.00	20.38
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	9.94	1.91	0.57	51.83	0.20	2.24	1.40
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

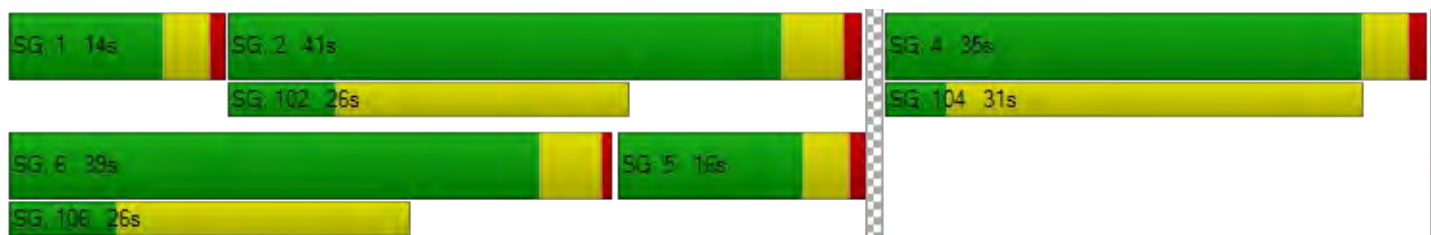
X, volume / capacity	0.62	0.62	0.07	1.09	0.06	0.38	0.31
d, Delay for Lane Group [s/veh]	45.45	21.71	34.69	78.33	18.67	23.24	21.77
Lane Group LOS	D	C	C	E	B	C	C
Critical Lane Group	yes	no	no	yes	no	yes	no
50th-Percentile Queue Length [veh]	4.76	11.41	0.40	35.05	0.65	3.93	3.59
50th-Percentile Queue Length [ft]	119.12	285.30	10.10	876.35	16.30	98.24	89.66
95th-Percentile Queue Length [veh]	9.46	19.42	1.02	56.12	1.62	8.08	7.49
95th-Percentile Queue Length [ft]	236.52	485.59	25.47	1402.94	40.40	201.96	187.22

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	45.45	21.71	21.71	34.69	78.33	78.33	18.67	18.67	18.67	23.24	23.24	21.77
Movement LOS	D	C	C	C	E	E	B	B	B	C	C	C
d_A, Approach Delay [s/veh]	25.32			77.86			18.67			22.53		
Approach LOS	C			E			B			C		
d_I, Intersection Delay [s/veh]	51.34											
Intersection LOS	D											
Intersection V/C	0.684											

Sequence

Ring 1	1	2	4	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#163: Bayfront Expy/Marsh Rd**

Control Type:	Signalized	Delay (sec / veh):	320.4
Analysis Method:	HCM2000	Level Of Service:	F
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.964

Intersection Setup

Name	Northbound			Eastbound			Westbound			Southwestbound		
Approach	Northbound			Eastbound			Westbound			Southwestbound		
Lane Configuration	T T T			T T			T T T T			T		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	no			yes			yes			yes		

Volumes

Name	Northbound			Eastbound			Westbound			Southwestbound		
Base Volume Input [veh/h]	125	31	2131	8	477	136	1287	63	13	16	25	3
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	19.20	0.00	2.90	0.00	0.40	2.20	2.90	14.30	0.00	0.00	0.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	125	31	2131	8	477	136	1287	63	13	16	25	3
Peak Hour Factor	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	33	8	555	2	124	35	335	16	3	4	7	1
Total Analysis Volume [veh/h]	130	32	2220	8	497	142	1341	66	14	17	26	3
Presence of On-Street Parking	no		no	no		no	no		no	no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0			3			0			2		
Bicycle Volume [bicycles/h]	0			0			2			2		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	40
Coordination Type	Free Running
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Permiss	Overlap	Split	Split	Split	Split	Split	Split	Permiss	Permiss	Permiss
Signal Group	2	3	2	4	1	4	8	2	8	6	4	6
Auxiliary Signal Groups			2,3									
Lead / Lag	Lag	-	-	Lag	-	-	-	-	-	-	-	-
Minimum Green [s]	10	6	10	4	12	4	0	10	0	0	4	0
Maximum Green [s]	50	26	50	10	30	10	0	50	0	0	10	0
Amber [s]	4.7	3.6	4.7	3.6	3.6	3.6	0.0	4.7	0.0	0.0	3.6	0.0
All red [s]	1.0	1.0	1.0	0.5	0.0	0.5	0.0	1.0	0.0	0.0	0.5	0.0
Split [s]	30	0	30	0	0	0	0	30	0	0	0	0
Vehicle Extension [s]	4.5	2.5	4.5	2.8	2.0	2.8	0.0	4.5	0.0	0.0	2.8	0.0
Walk [s]	7	0	7	7	5	7	0	7	0	0	7	0
Pedestrian Clearance [s]	16	0	16	22	26	22	0	16	0	0	22	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	2.0	2.0	2.0	0.0	2.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no	no		no			no			no	
Maximum Recall		no	no		no			no			no	
Pedestrian Recall		no	no		no			no			no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	20.0	20.0	20.0	0.0	20.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	R	C	R	L	C	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	29	54	32	32	54	54	12
g / C, Green / Cycle	0.19	0.36	0.21	0.21	0.36	0.36	0.08
(v / s)_i Volume / Saturation Flow Rate	0.09	0.80	0.14	0.09	0.26	0.05	0.02
Total Saturation Flow Adjustment	0.96	0.73	0.95	0.83	0.90	0.85	0.97
s, saturation flow rate [veh/h]	1827	2778	3600	1580	5110	1616	1845
c, Capacity [veh/h]	351	1001	764	335	1842	582	150
d1, Uniform Delay [s]	53.38	47.65	53.80	50.82	41.32	32.06	64.50
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	4.33	551.06	4.47	3.89	2.56	0.49	5.23
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.46	2.22	0.66	0.42	0.73	0.14	0.31
d, Delay for Lane Group [s/veh]	57.71	598.71	58.27	54.71	43.89	32.56	69.73
Lane Group LOS	E	F	E	D	D	C	E
Critical Lane Group	no	yes	yes	no	no	no	yes
50th-Percentile Queue Length [veh]	6.60	139.93	11.60	5.63	19.32	2.40	1.98
50th-Percentile Queue Length [ft]	164.94	3498.21	289.91	140.79	482.95	60.12	49.55
95th-Percentile Queue Length [veh]	12.32	223.89	19.69	10.84	31.31	5.33	4.50
95th-Percentile Queue Length [ft]	307.98	5597.14	492.37	270.92	782.86	133.36	112.61

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	57.71	57.71	598.71	58.27	58.27	54.71	43.89	32.56	32.56	69.73	69.73	69.73
Movement LOS	E	E	F	E	E	D	D	C	C	E	E	E
d_A, Approach Delay [s/veh]	561.92			57.49			43.25			69.73		
Approach LOS	F			E			D			E		
d_I, Intersection Delay [s/veh]	320.36											
Intersection LOS	F											
Intersection V/C	0.964											

Sequence

Ring 1	1	2	3	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#181: Santa Cruz Ave/Elder Ave**

Control Type:	Signalized	Delay (sec / veh):	10.0
Analysis Method:	HCM2000	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.514

Intersection Setup

Name	Santa Cruz Avenue				Elder Ave	
Approach	Northeastbound		Southwestbound		Southeastbound	
Lane Configuration	↵		↵		↵↶	
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	0	0	1
Pocket Length [ft]	60.00	100.00	100.00	100.00	100.00	105.00
Speed [mph]	25.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Santa Cruz Avenue				Elder Ave	
Base Volume Input [veh/h]	61	669	641	79	46	41
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	1.50	0.20	0.00	0.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	61	669	641	79	46	41
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	16	178	170	21	12	11
Total Analysis Volume [veh/h]	65	712	682	84	49	44
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		2		12	
Bicycle Volume [bicycles/h]	0		0		0	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	100
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	8.00

Phasing & Timing

Control Type	Protected	Permissive	Permissive	Permissive	Split	Split
Signal Group	5	2	6	0	3	0
Auxiliary Signal Groups						
Lead / Lag	Lead	-	-	-	Lead	-
Minimum Green [s]	5	5	5	0	5	0
Maximum Green [s]	30	30	30	0	30	0
Amber [s]	3.0	3.0	3.0	0.0	3.0	0.0
All red [s]	1.0	1.0	1.0	0.0	1.0	0.0
Split [s]	11	90	79	0	10	0
Vehicle Extension [s]	3.0	3.0	3.0	0.0	3.0	0.0
Walk [s]	0	1	1	0	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	2.0	2.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	2.0	2.0	2.0	0.0	2.0	0.0
Minimum Recall	no	no	no		no	
Maximum Recall	yes	yes	yes		yes	
Pedestrian Recall	no	yes	yes		yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	20.0	20.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	C	C	L	R
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	7	86	75	6	6
g / C, Green / Cycle	0.07	0.86	0.75	0.06	0.06
(v / s)_i Volume / Saturation Flow Rate	0.04	0.38	0.41	0.03	0.03
Total Saturation Flow Adjustment	0.95	0.99	0.98	0.95	0.85
s, saturation flow rate [veh/h]	1805	1872	1868	1805	1615
c, Capacity [veh/h]	126	1610	1401	108	97
d1, Uniform Delay [s]	44.86	1.58	5.30	45.41	45.42
k, delay calibration	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	14.17	0.88	1.54	13.04	14.58
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.51	0.44	0.55	0.45	0.45
d, Delay for Lane Group [s/veh]	59.03	2.47	6.83	58.46	60.00
Lane Group LOS	E	A	A	E	E
Critical Lane Group	yes	no	yes	no	yes
50th-Percentile Queue Length [veh]	2.04	5.81	10.85	1.52	1.37
50th-Percentile Queue Length [ft]	50.92	145.33	271.30	38.09	34.37
95th-Percentile Queue Length [veh]	4.61	11.12	18.60	3.56	3.24
95th-Percentile Queue Length [ft]	115.35	277.97	465.05	89.03	81.11

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	59.03	2.47	6.83	6.83	58.46	60.00
Movement LOS	E	A	A	A	E	E
d_A, Approach Delay [s/veh]	7.20		6.83		59.19	
Approach LOS	A		A		E	
d_I, Intersection Delay [s/veh]	9.98					
Intersection LOS	A					
Intersection V/C	0.514					

Sequence

Ring 1	-	2	3	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#195: Bayfront Expy/Chilco St**

Control Type:	Signalized	Delay (sec / veh):	12.7
Analysis Method:	HCM2000	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.701

Intersection Setup

Name	Chilco Street		Westbound		Bayfront Expy	
Approach	Northbound		Westbound		Southeastbound	
Lane Configuration	⇐⇐		⇐⇐⇐⇐		⇐⇐⇐	
Turning Movement	Left	Right	Left	Thru	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	1	0	0	1
Pocket Length [ft]	80.00	100.00	520.00	100.00	100.00	660.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Chilco Street		Westbound		Bayfront Expy	
Base Volume Input [veh/h]	201	196	19	767	2416	156
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	5.50	3.10	21.10	4.80	3.10	1.30
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	201	196	19	767	2416	156
Peak Hour Factor	0.9300	0.9300	0.9300	0.9300	0.9300	0.9300
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	54	53	5	206	649	42
Total Analysis Volume [veh/h]	216	211	20	825	2598	168
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		2		1	
Bicycle Volume [bicycles/h]	5		1		1	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	9.00

Phasing & Timing

Control Type	Protected	Permissive	Permissive	Permissive	Permissive	Permissive
Signal Group	5	0	0	4	8	0
Auxiliary Signal Groups						
Lead / Lag	Lead	-	-	-	-	-
Minimum Green [s]	5	0	0	5	5	0
Maximum Green [s]	30	0	0	30	30	0
Amber [s]	3.0	0.0	0.0	3.0	3.0	0.0
All red [s]	1.0	0.0	0.0	1.0	1.0	0.0
Split [s]	29	0	0	101	101	0
Vehicle Extension [s]	3.0	0.0	0.0	3.0	3.0	0.0
Walk [s]	1	0	0	1	1	0
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	0.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	no			no	no	
Maximum Recall	yes			yes	yes	
Pedestrian Recall	yes			yes	yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	0.0	0.0	20.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	R	L	C	C	R
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	2.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	27	27	99	99	99	99
g / C, Green / Cycle	0.21	0.21	0.76	0.76	0.76	0.76
(v / s)_i Volume / Saturation Flow Rate	0.13	0.13	0.32	0.17	0.52	0.11
Total Saturation Flow Adjustment	0.90	0.82	0.03	0.87	0.88	0.84
s, saturation flow rate [veh/h]	1711	1566	63	4939	5020	1594
c, Capacity [veh/h]	355	325	55	3761	3823	1214
d1, Uniform Delay [s]	46.70	47.16	5.10	4.44	7.66	4.13
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	7.53	9.62	17.33	0.13	0.99	0.24
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.61	0.65	0.36	0.22	0.68	0.14
d, Delay for Lane Group [s/veh]	54.23	56.78	22.42	4.57	8.65	4.37
Lane Group LOS	D	E	C	A	A	A
Critical Lane Group	no	yes	no	no	yes	no
50th-Percentile Queue Length [veh]	8.12	8.13	0.34	3.65	20.75	1.89
50th-Percentile Queue Length [ft]	203.00	203.16	8.39	91.24	518.71	47.21
95th-Percentile Queue Length [veh]	14.59	14.60	0.85	7.60	33.52	4.32
95th-Percentile Queue Length [ft]	364.82	365.06	21.26	189.95	838.11	107.89

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	54.23	56.78	22.42	4.57	8.65	4.37
Movement LOS	D	E	C	A	A	A
d_A, Approach Delay [s/veh]	55.49		4.99		8.39	
Approach LOS	E		A		A	
d_I, Intersection Delay [s/veh]	12.66					
Intersection LOS	B					
Intersection V/C	0.701					

Sequence




Ring 1	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#196: Bayfront Expy/Chrysler Drive**

Control Type:	Signalized	Delay (sec / veh):	21.6
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.795

Intersection Setup

Name	Chrysler Drive		Eastbound		Bayfront Expy	
Approach	Northbound		Eastbound		Westbound	
Lane Configuration						
Turning Movement	Left	Right	Thru	Right	Left	Thru
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	1	0	0	1	1	0
Pocket Length [ft]	140.00	100.00	100.00	290.00	345.00	100.00
Speed [mph]	25.00		45.00		45.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Chrysler Drive		Eastbound		Bayfront Expy	
Base Volume Input [veh/h]	382	43	2516	107	5	978
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.80	0.00	2.80	0.90	0.00	4.90
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	382	43	2516	107	5	978
Peak Hour Factor	0.9700	0.9700	0.9700	0.9700	0.9700	0.9700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	98	11	648	28	1	252
Total Analysis Volume [veh/h]	394	44	2594	110	5	1008
Presence of On-Street Parking	no	no	no	no	no	no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0		0		0	
Bicycle Volume [bicycles/h]	0		2		0	

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	130
Coordination Type	Time of Day Pattern Isolated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	9.00

Phasing & Timing

Control Type	Split	Split	Permissive	Permissive	Protected	Permissive
Signal Group	5	0	8	0	7	4
Auxiliary Signal Groups						
Lead / Lag	Lead	-	-	-	Lead	-
Minimum Green [s]	5	0	5	0	5	5
Maximum Green [s]	30	0	30	0	30	30
Amber [s]	3.0	0.0	3.0	0.0	3.0	3.0
All red [s]	1.0	0.0	1.0	0.0	1.0	1.0
Split [s]	38	0	83	0	9	92
Vehicle Extension [s]	3.0	0.0	3.0	0.0	3.0	3.0
Walk [s]	1	0	1	0	0	1
Pedestrian Clearance [s]	0	0	0	0	0	0
I1, Start-Up Lost Time [s]	2.0	0.0	2.0	0.0	2.0	2.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall	no		no		no	no
Maximum Recall	yes		yes		yes	yes
Pedestrian Recall	yes		yes		yes	yes
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	20.0	0.0	20.0	0.0	20.0	20.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	L	R	C	R	L	C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	2.00	2.00
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	0.00	0.00
g_i, Effective Green Time [s]	36	36	81	81	7	90
g / C, Green / Cycle	0.28	0.28	0.62	0.62	0.05	0.69
(v / s)_i Volume / Saturation Flow Rate	0.22	0.03	0.52	0.07	0.00	0.20
Total Saturation Flow Adjustment	0.93	0.85	0.88	0.84	0.95	0.87
s, saturation flow rate [veh/h]	1773	1615	5035	1601	1805	4934
c, Capacity [veh/h]	491	447	3137	997	97	3416
d1, Uniform Delay [s]	43.69	34.94	19.05	9.92	58.35	7.73
k, delay calibration	0.50	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	12.99	0.44	2.65	0.22	1.00	0.22
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.80	0.10	0.83	0.11	0.05	0.30
d, Delay for Lane Group [s/veh]	56.68	35.38	21.70	10.14	59.35	7.95
Lane Group LOS	E	D	C	B	E	A
Critical Lane Group	yes	no	yes	no	yes	no
50th-Percentile Queue Length [veh]	16.16	1.27	32.97	1.79	0.19	5.89
50th-Percentile Queue Length [ft]	404.08	31.82	824.26	44.76	4.67	147.18
95th-Percentile Queue Length [veh]	26.50	3.02	52.80	4.12	0.48	11.23
95th-Percentile Queue Length [ft]	662.47	75.57	1319.94	102.90	11.98	280.83

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	56.68	35.38	21.70	10.14	59.35	7.95
Movement LOS	E	D	C	B	E	A
d_A, Approach Delay [s/veh]	54.54		21.23		8.21	
Approach LOS	D		C		A	
d_I, Intersection Delay [s/veh]	21.57					
Intersection LOS	C					
Intersection V/C	0.795					

Sequence

Ring 1	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	7	8	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#207: Chilco St/Constitution Dr**

Control Type: All-way stop
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 23.6
 Level Of Service: C

Intersection Setup

Name	Northbound			Chilco Street			Constitution Drive			Northwestbound		
Approach	Northbound			Southbound			Eastbound			Northwestbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Northbound			Chilco Street			Constitution Drive			Northwestbound		
Base Volume Input [veh/h]	14	136	5	17	160	2	28	2	114	158	10	398
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	21.40	1.50	20.00	11.80	3.80	0.00	3.60	50.00	2.60	2.50	50.00	1.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	14	136	5	17	160	2	28	2	114	158	10	398
Peak Hour Factor	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100	0.9100
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	4	37	1	5	44	1	8	1	31	43	3	109
Total Analysis Volume [veh/h]	15	149	5	19	176	2	31	2	125	174	11	437
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings

Lanes

Movement, Approach, & Intersection Results

95th-Percentile Queue Length [veh]	1.30	1.60	1.01	10.83
95th-Percentile Queue Length [ft]	32.45	40.02	25.16	270.68
Approach Delay [s/veh]	12.44	13.06	10.76	33.32
Approach LOS	B	B	B	D
Intersection Delay [s/veh]	23.65			
Intersection LOS	C			

**Intersection Level Of Service Report
#209: Jefferson Dr/Constitution Dr**

Control Type: Two-way stop
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 15.5
 Level Of Service: C
 Volume to Capacity (v/c): 0.037

Intersection Setup

Name	Southbound			Eastbound			Constitution Drive			Jefferson Drive		
Approach	Southbound			Eastbound			Westbound			Northeastbound		
Lane Configuration	↑			↑			↑			Y		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Southbound			Eastbound			Constitution Drive			Jefferson Drive		
Base Volume Input [veh/h]	11	0	4	2	481	3	2	24	1	13	0	56
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	0.00	0.00	0.00	1.70	0.00	0.00	16.70	0.00	7.70	0.00	7.10
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	11	0	4	2	481	3	2	24	1	13	0	56
Peak Hour Factor	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800	0.8800
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	3	0	1	1	137	1	1	7	0	4	0	16
Total Analysis Volume [veh/h]	13	0	5	2	547	3	2	27	1	15	0	64
Pedestrian Volume [ped/h]	0			0			0			0		

Intersection Settings

Priority Scheme	Stop	Free	Free	Stop
Flared Lane	no			no
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance	no			no
Number of Storage Spaces in Median	0	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.04	0.00	0.00	0.00	0.01	0.00	0.00	0.00	0.00	0.04	0.00	0.12
d_M, Delay for Movement [s/veh]	15.49	13.85	8.77	7.25	0.00	0.00	8.50	0.00	0.00	15.13	14.84	13.19
Movement LOS	C	B	A	A	A	A	A	A	A	C	B	B
95th-Percentile Queue Length [veh]	0.13	0.13	0.13	1.56	1.56	1.56	0.09	0.09	0.09	0.56	0.56	0.56
95th-Percentile Queue Length [ft]	3.23	3.23	3.23	39.09	39.09	39.09	2.25	2.25	2.25	13.95	13.95	13.95
d_A, Approach Delay [s/veh]	13.62			0.03			0.57			13.56		
Approach LOS	B			A			A			B		
d_I, Intersection Delay [s/veh]	1.99											
Intersection LOS	C											

**Intersection Level Of Service Report
#213: Chrysler Dr/Independence Dr**

Control Type: Two-way stop
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 10.0
 Level Of Service: A
 Volume to Capacity (v/c): 0.004

Intersection Setup

Name	Chrysler Drive						Independence Drive					
Approach	Northbound			Southwestbound			Northwestbound			Southeastbound		
Lane Configuration	Y			T			T			T		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Chrysler Drive						Independence Drive					
Base Volume Input [veh/h]	0	101	0	1	2	3	0	0	15	35	3	6
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	3.00	0.00	100.00	0.00	0.00	0.00	0.00	0.00	5.70	0.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	101	0	1	2	3	0	0	15	35	3	6
Peak Hour Factor	0.8600	0.8600	0.8600	0.8600	0.8600	0.8600	0.8600	0.8600	0.8600	0.8600	0.8600	0.8600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	29	0	0	1	1	0	0	4	10	1	2
Total Analysis Volume [veh/h]	0	117	0	1	2	3	0	0	17	41	3	7
Pedestrian Volume [ped/h]	0			0			5			0		

Intersection Settings

Priority Scheme	Free	Free	Stop	Stop
Flared Lane			no	no
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance			no	no
Number of Storage Spaces in Median	0	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.02	0.05	0.00	0.01
d_M, Delay for Movement [s/veh]	7.21	0.00	0.00	8.52	0.00	0.00	9.40	9.80	8.94	9.68	9.99	8.59
Movement LOS	A	A	A	A	A	A	A	A	A	A	A	A
95th-Percentile Queue Length [veh]	0.00	0.00	0.00	0.02	0.02	0.02	0.06	0.06	0.06	0.19	0.19	0.19
95th-Percentile Queue Length [ft]	0.00	0.00	0.00	0.44	0.44	0.44	1.39	1.39	1.39	4.83	4.83	4.83
d_A, Approach Delay [s/veh]	0.00			1.42			8.94			9.55		
Approach LOS	A			A			A			A		
d_I, Intersection Delay [s/veh]	3.39											
Intersection LOS	A											

**Intersection Level Of Service Report
#214: Chrysler Dr/Jefferson Dr**

Control Type: Two-way stop
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 10.4
 Level Of Service: B
 Volume to Capacity (v/c): 0.002

Intersection Setup

Name	Chrysler Drive				Jefferson Drive	
Approach	Southbound		Northeastbound		Northwestbound	
Lane Configuration	Y		T		T	
Turning Movement	Left	Thru	Thru	Right	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	yes		yes		yes	

Volumes

Name	Chrysler Drive				Jefferson Drive	
Base Volume Input [veh/h]	9	7	132	7	2	134
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	22.20	42.90	0.80	71.40	0.00	2.20
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	9	7	132	7	2	134
Peak Hour Factor	0.8700	0.8700	0.8700	0.8700	0.8700	0.8700
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	3	2	38	2	1	39
Total Analysis Volume [veh/h]	10	8	152	8	2	154
Pedestrian Volume [ped/h]	1		0		1	

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			no
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			no
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.01	0.00	0.00	0.00	0.00	0.17
d_M, Delay for Movement [s/veh]	7.78	0.00	0.00	0.00	10.36	9.94
Movement LOS	A	A	A	A	B	A
95th-Percentile Queue Length [veh]	0.04	0.04	0.00	0.00	0.64	0.64
95th-Percentile Queue Length [ft]	1.05	1.05	0.00	0.00	15.96	15.96
d_A, Approach Delay [s/veh]	4.32		0.00		9.94	
Approach LOS	A		A		A	
d_I, Intersection Delay [s/veh]	4.88					
Intersection LOS	B					

**Intersection Level Of Service Report
#215: Chrysler Dr/Constitution Dr**

Control Type: All-way stop
 Analysis Method: HCM2000
 Analysis Period: 15 minutes

Delay (sec / veh): 14.4
 Level Of Service: B

Intersection Setup

Name	Chrysler Drive						Constitution Drive					
Approach	Southbound			Eastbound			Westbound			Northeastbound		
Lane Configuration	↕			↕			↕			↕		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Chrysler Drive						Constitution Drive					
Base Volume Input [veh/h]	190	6	1	66	285	9	2	2	78	1	234	14
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	1.60	0.00	100.00	1.50	1.80	11.10	50.00	50.00	5.10	0.00	0.00	0.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	190	6	1	66	285	9	2	2	78	1	234	14
Peak Hour Factor	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000	0.9000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	53	2	0	18	79	3	1	1	22	0	65	4
Total Analysis Volume [veh/h]	211	7	1	73	317	10	2	2	87	1	260	16
Pedestrian Volume [ped/h]	2			0			2			1		

Intersection Settings

Lanes

Movement, Approach, & Intersection Results

95th-Percentile Queue Length [veh]	1.70	4.32	0.50	2.25
95th-Percentile Queue Length [ft]	42.53	107.97	12.58	56.34
Approach Delay [s/veh]	12.62	17.35	9.68	13.19
Approach LOS	B	C	A	B
Intersection Delay [s/veh]	14.42			
Intersection LOS	B			

**Intersection Level Of Service Report
#233: Sand Hill Circle/Sand Hill Road**

Control Type:	Signalized	Delay (sec / veh):	361.8
Analysis Method:	HCM2000	Level Of Service:	F
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.712

Intersection Setup

Name	Sand Hill Road			Sand Hill Road			Sand Hill Circle			Sand Hill Circle		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	↵			↵↶						↵↶		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Sand Hill Road			Sand Hill Road			Sand Hill Circle			Sand Hill Circle		
Base Volume Input [veh/h]	9	54	0	0	87	180	0	0	0	5	1849	9
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	0.00	3.70	2.00	2.00	5.70	1.10	2.00	2.00	2.00	0.00	0.90	11.10
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	9	54	0	0	87	180	0	0	0	5	1849	9
Peak Hour Factor	0.8900	0.8900	1.0000	1.0000	0.8900	0.8900	1.0000	1.0000	1.0000	0.8900	0.8900	0.8900
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	3	15	0	0	24	51	0	0	0	1	519	3
Total Analysis Volume [veh/h]	10	61	0	0	98	202	0	0	0	6	2078	10
Presence of On-Street Parking	no					no				no		no
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	2			1			0			0		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fixed time
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Protecte	Permiss	Split	Split	Split
Signal Group	0	8	0	0	8	0	0	0	0	0	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	-	-	-	-	-	-
Minimum Green [s]	0	6	0	0	6	0	0	0	0	0	8	0
Maximum Green [s]	0	11	0	0	11	0	0	0	0	0	26	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	0.0	0.0	0.0	4.0	0.0
All red [s]	0.0	0.5	0.0	0.0	0.5	0.0	0.0	0.0	0.0	0.0	0.5	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	2.5	0.0	0.0	2.5	0.0	0.0	0.0	0.0	0.0	3.0	0.0
Walk [s]	0	10	0	0	10	0	0	0	0	0	7	0
Pedestrian Clearance [s]	0	10	0	0	10	0	0	0	0	0	14	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	0.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no			no						yes	
Maximum Recall		no			no						no	
Pedestrian Recall		no			no						no	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	0.0	0.0	20.0	0.0	0.0	0.0	0.0	0.0	20.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	C	R		C
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00		2.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	0.00		0.00
l2, Clearance Lost Time [s]	0.00	0.00	0.00		0.00
g_i, Effective Green Time [s]	13	13	13		29
g / C, Green / Cycle	0.14	0.14	0.14		0.32
(v / s)_i Volume / Saturation Flow Rate	0.04	0.05	0.13		0.58
Total Saturation Flow Adjustment	0.93	0.95	0.83		0.94
s, saturation flow rate [veh/h]	1773	1798	1579		3582
c, Capacity [veh/h]	246	250	219		1134
d1, Uniform Delay [s]	34.76	35.29	38.26		30.75
k, delay calibration	0.50	0.50	0.50		0.50
l, Upstream Filtering Factor	1.00	1.00	1.00		1.00
d2, Incremental Delay [s]	2.93	4.58	43.19		384.11
d3, Initial Queue Delay [s]	0.00	0.00	0.00		0.00
Rp, platoon ratio	1.00	1.00	1.00		1.00
PF, progression factor	1.00	1.00	1.00		1.00

Lane Group Results

X, volume / capacity	0.29	0.39	0.92		1.85
d, Delay for Lane Group [s/veh]	37.69	39.87	81.45		414.86
Lane Group LOS	D	D	F		F
Critical Lane Group	no	no	yes		yes
50th-Percentile Queue Length [veh]	1.76	2.51	7.24		92.18
50th-Percentile Queue Length [ft]	44.11	62.67	181.04		2304.61
95th-Percentile Queue Length [veh]	4.06	5.53	13.29		147.49
95th-Percentile Queue Length [ft]	101.57	138.23	332.21		3687.37

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	37.69	37.69	0.00	0.00	39.87	81.45	0.00	0.00	0.00	414.86	414.86	414.86
Movement LOS	D	D			D	F				F	F	F
d_A, Approach Delay [s/veh]	37.69			67.87			0.00			414.86		
Approach LOS	D			E			A			F		
d_I, Intersection Delay [s/veh]	361.76											
Intersection LOS	F											
Intersection V/C	0.712											

Sequence

Ring 1	2	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report
#234: Sand Hill Rd/Hwy 280 NB Off-Ramp**

Control Type:	Signalized	Delay (sec / veh):	31.3
Analysis Method:	HCM2000	Level Of Service:	C
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.360

Intersection Setup

Name	Hwy 280 NB Off-Ramp			Sand Hill Road			Sand Hill Road					
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	T T			L			T T T					
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Pocket	0	0	1	0	0	0	1	0	0	0	0	0
Pocket Length [ft]	100.00	100.00	300.00	100.00	100.00	100.00	270.00	100.00	100.00	100.00	100.00	100.00
Speed [mph]	40.00			40.00			40.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	yes			yes			yes			yes		

Volumes

Name	Hwy 280 NB Off-Ramp			Sand Hill Road			Sand Hill Road					
Base Volume Input [veh/h]	0	23	193	129	0	0	38	697	0	0	0	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	0.00	4.30	0.00	2.00	2.00	5.30	1.00	2.00	2.00	2.00	2.00
Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right-Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	23	193	129	0	0	38	697	0	0	0	0
Peak Hour Factor	1.0000	0.9300	0.9300	0.9300	1.0000	1.0000	0.9300	0.9300	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	6	52	35	0	0	10	187	0	0	0	0
Total Analysis Volume [veh/h]	0	25	208	139	0	0	41	749	0	0	0	0
Presence of On-Street Parking			no	no			no					
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			23			0		

Intersection Settings

Located in CBD	no
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	LeadGreen
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss	Split	Split	Split	Protecte	Permiss	Protecte	Permiss	Permiss	Protecte	Protecte	Permiss
Signal Group	0	8	0	7	0	0	1	6	0	0	0	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	Lag	-	-	Lead	-	-	-	-	-
Minimum Green [s]	0	6	0	6	0	0	4	8	0	0	0	0
Maximum Green [s]	0	11	0	22	0	0	13	26	0	0	0	0
Amber [s]	0.0	3.0	0.0	3.0	0.0	0.0	3.0	4.0	0.0	0.0	0.0	0.0
All red [s]	0.0	0.5	0.0	0.5	0.0	0.0	0.5	0.5	0.0	0.0	0.0	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	2.5	0.0	2.0	0.0	0.0	2.0	4.0	0.0	0.0	0.0	0.0
Walk [s]	0	10	0	0	0	0	5	10	0	0	0	0
Pedestrian Clearance [s]	0	10	0	0	0	0	10	10	0	0	0	0
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	0.0	0.0
I2, Clearance Lost Time [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Minimum Recall		no		no			no	yes				
Maximum Recall		no		no			no	no				
Pedestrian Recall		no		no			no	no				
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	20.0	0.0	20.0	0.0	0.0	20.0	20.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Calculations

Lane Group	C	L	L	C	
L, Total Lost Time per Cycle [s]	2.00	2.00	2.00	2.00	
l1_p, Permitted Start-Up Lost Time [s]	0.00	0.00	0.00	0.00	
l2, Clearance Lost Time [s]	0.00	0.00	0.00	0.00	
g_i, Effective Green Time [s]	13	24	29	29	
g / C, Green / Cycle	0.14	0.27	0.32	0.32	
(v / s)_i Volume / Saturation Flow Rate	0.07	0.08	0.02	0.21	
Total Saturation Flow Adjustment	0.82	0.95	0.90	0.94	
s, saturation flow rate [veh/h]	3133	1805	1714	3582	
c, Capacity [veh/h]	435	481	543	1134	
d1, Uniform Delay [s]	36.05	26.22	21.53	26.57	
k, delay calibration	0.50	0.50	0.50	0.50	
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	
d2, Incremental Delay [s]	4.66	1.51	0.27	3.03	
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	
Rp, platoon ratio	1.00	1.00	1.00	1.00	
PF, progression factor	1.00	1.00	1.00	1.00	

Lane Group Results

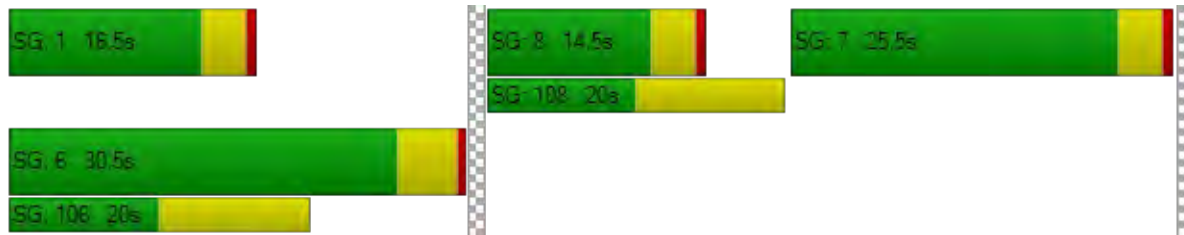
X, volume / capacity	0.54	0.29	0.08	0.66	
d, Delay for Lane Group [s/veh]	40.71	27.73	21.80	29.59	
Lane Group LOS	D	C	C	C	
Critical Lane Group	yes	yes	no	yes	
50th-Percentile Queue Length [veh]	3.30	3.04	0.78	9.96	
50th-Percentile Queue Length [ft]	82.48	75.93	19.46	248.95	
95th-Percentile Queue Length [veh]	6.98	6.51	1.91	17.29	
95th-Percentile Queue Length [ft]	174.60	162.85	47.79	432.30	

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	0.00	40.71	40.71	27.73	0.00	0.00	21.80	29.59	0.00	0.00	0.00	0.00
Movement LOS		D	D	C			C	C				
d_A, Approach Delay [s/veh]		40.71		27.73		29.19		0.00				
Approach LOS		D		C		C		A				
d_I, Intersection Delay [s/veh]	31.33											
Intersection LOS	C											
Intersection V/C	0.360											

Sequence

Ring 1	1	-	7	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Menlo Park GP Circulation Update

Vistro File: J:\...\Menlo Park PM_update.vistro
Report File: J:\...\Menlo Park_PM Results.pdf

Scenario 1: Existing PM
1/9/2015

Turning Movement Volume: Summary

ID	Intersection Name	Northeastbound		Southwestbound		Southeastbound		Total Volume
		Thru	Right	Left	Thru	Left	Right	
1	Marsh Rd (SR 84)/US 101 SB Offramp	980		893		1547	401	3821

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
2	Marsh Rd/Rolison Rd-Scott Dr	29	1083	4	66	1014	203	27	9	467	229	13	1	3145

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
3	Marsh Rd/Florence St-Bohannon Dr	185	720	84	30	697	377	475	15	130	98	41	72	2924

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4	Marsh Rd/Bay Rd	2	681	108	194	841	54	45	29	6	84	18	143	2205

ID	Intersection Name	Northeastbound		Northwestbound		Southeastbound		Total Volume
		Left	Right	Left	Thru	Thru	Right	
9	Middlefield Rd/Ravenswood Ave	199	610	457	673	357	60	2356

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
10	Middlefield Rd/Ringwood Ave	58	70	38	63	1	272	10	775	97	379	568	5	2336

ID	Intersection Name	Northeastbound		Southwestbound		Northwestbound		Total Volume
		Thru	Right	Left	Thru	Left	Right	
15	Bayfront Expy (SR 84)/University Ave (SR 109)	3800	62	304	813	78	1577	6634

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
16	Bayfront Expy (SR 84)/Willow Rd (SR 114)	24	46	1104	116	191	116	35	2362	75	359	675	10	5113

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
17	Willow Rd (SR 114)/Hamilton Ave	50	1299	3	13	623	30	163	15	39	139	17	61	2452

ID	Intersection Name	Northbound		Southbound		Eastbound		Total Volume
		Left	Thru	Thru	Right	Left	Right	
18	Willow Rd (SR 114)/Ivy Dr	68	1339	885	15	33	141	2481

ID	Intersection Name	Northbound		Southbound		Westbound		Total Volume
		Thru	Right	Left	Thru	Left	Right	
19	Willow Rd (SR 114)/O'Brien Dr	1368	217	72	966	218	44	2885

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
20	Willow Rd (SR 114)/Newbridge St	301	1484	263	74	1016	22	28	137	210	210	249	76	4070

ID	Intersection Name	Northeastbound		Southwestbound		Southeastbound		Total Volume
		Left	Thru	Thru	Right	Left	Right	
21	Willow Rd/Bay Rd	35	1276	922	261	504	68	3066

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
22	Willow Rd/Durham St-VA Med Entrance	8	935	10	54	719	29	117	2	28	21	2	97	2022

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
23	Willow Rd/Coleman Ave	11	738	3	5	680	84	102	4	31	1	1	4	1664

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
24	Willow Rd/Gilbert Ave	30	671	13	26	597	6	61	51	69	56	43	28	1651

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
25	Middlefield Rd-Willow Rd	46	94	256	356	93	332	92	429	216	236	453	29	2632

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
26	Ravenswood Ave/Laurel St	57	596	125	32	523	24	267	215	64	52	135	26	2116

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
28	Oak Grove Ave/Laurel St	13	335	94	30	262	48	78	232	38	28	116	29	1303

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
29	El Camino Real (SR 82)/Encinal Ave-Menlo College Entrance	19	15	31	107	9	238	24	1750	100	65	1215	13	3586

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
30	El Camino Real (SR 82)/Glenwood Ave-Valparaiso Ave	363	144	79	59	171	41	98	1525	46	56	992	304	3878

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
31	El Camino Real (SR 82)/Oak Grove Ave	118	202	133	129	215	77	87	1426	94	114	1047	97	3739

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound		Southeastbound		Total Volume
		Left	Thru	Right	Left	Thru	Right	Thru	Right	Thru	Right	
32	El Camino Real (SR 82)/Santa Cruz Ave	172	50	137	50	58	43	1406	41	1451	87	3495

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
33	El Camino Real (SR 82)/Ravenswood Ave-Menlo Ave	44	348	108	451	249	80	113	1339	590	180	1154	39	4695

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
34	El Camino Real (SR 82)/Roble Ave	66	6	36	58	45	48	72	1845	20	68	1586	54	3904

ID	Intersection Name	Northeastbound		Northwestbound		Southeastbound		Total Volume
		Left	Right	Left	Thru	Thru	Right	
35	El Camino Real (SR 82)/Middle Ave	249	182	323	1917	1334	86	4091

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
36	El Camino Real (SR 82)/Cambridge Ave	26	0	39	4	3	0	310	2332	6	30	1660	4	4414

ID	Intersection Name	Northeastbound		Southwestbound		Northwestbound		Total Volume
		Thru	Right	Left	Thru	Left	Right	
38	Santa Cruz Ave/University Dr (S)	350	328	88	398	363	165	1692

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
39	Sand Hill Rd/Santa Cruz Ave	361	611	215	424	1038	208	158	691	188	101	526	178	4699

ID	Intersection Name	Northbound		Southbound		Eastbound			Total Volume
		Left	Thru	Thru	Right	Left	Thru	Right	
58	University Avenue and Adams Drive	11	1786	360	15	25	0	26	2223

ID	Intersection Name	Northbound		Southbound		Eastbound		Total Volume
		Left	Thru	Thru	Right	Left	Right	
74	University Ave/O'Brien Dr	11	1724	356	10	171	77	2349

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
88	Valparaiso Ave/ University Dr	24	467	105	44	456	29	182	24	57	45	41	72	1546

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
103	Addison Wesley/Sand Hill Rd	209	3	108	19	3	66	18	794	93	69	1750	16	3148

ID	Intersection Name	Southbound		Westbound		Northeastbound		Total Volume
		Left	Thru	Left	Right	Thru	Right	
107	Alpine Rd/Santa Cruz Ave&Junipero Serra Blvd	373	638	423	398	535	100	2467

ID	Intersection Name	Northbound		Southbound		Northwestbound		Total Volume
		Thru		Thru		Left	Right	
110	Marsh Road/101 NB Ramps	2255		567		544	239	3605

ID	Intersection Name	Northeastbound		Southwestbound		Southeastbound		Total Volume
		Thru		Thru	Right	Left	Right	
132	Oak Ave/Sand Hill Rd	912		1585	100	29	88	2714

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
156	Saga Ln/Sand Hill Rd	108	0	113	90	0	103	65	945	6	28	1307	20	2785

ID	Intersection Name	Northbound			Southbound			Eastbound			Westbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
157	Branner Dr/Sand Hill Rd	2	0	17	20	0	43	22	1076	7	17	1321	24	2549

ID	Intersection Name	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
162	Sharon Park Dr/ Sand Hill Rd	164	916	1	16	1220	252	12	6	17	178	3	172	2957

ID	Intersection Name	Northbound			Eastbound			Westbound			Southwestbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
163	Bayfront Expy/Marsh Rd	125	31	2131	8	477	136	1287	63	13	16	25	3	4315

ID	Intersection Name	Northeastbound		Southwestbound		Southeastbound		Total Volume
		Left	Thru	Thru	Right	Left	Right	
181	Santa Cruz Ave/Elder Ave	61	669	641	79	46	41	1537

ID	Intersection Name	Northbound		Westbound		Southeastbound		Total Volume
		Left	Right	Left	Thru	Thru	Right	
195	Bayfront Expy/Chilco St	201	196	19	767	2416	156	3755

ID	Intersection Name	Northbound		Eastbound		Westbound		Total Volume
		Left	Right	Thru	Right	Left	Thru	
196	Bayfront Expy/Chrysler Drive	382	43	2516	107	5	978	4031

ID	Intersection Name	Northbound			Southbound			Eastbound			Northwestbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
207	Chilco St/Constitution Dr	14	136	5	17	160	2	28	2	114	158	10	398	1044

ID	Intersection Name	Southbound			Eastbound			Westbound			Northeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
209	Jefferson Dr/Constitution Dr	11	0	4	2	481	3	2	24	1	13	0	56	597

ID	Intersection Name	Northbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
213	Chrysler Dr/Independence Dr	0	101	0	1	2	3	0	0	15	35	3	6	166

ID	Intersection Name	Southbound		Northeastbound		Northwestbound		Total Volume
		Left	Thru	Thru	Right	Left	Right	
214	Chrysler Dr/Jefferson Dr	9	7	132	7	2	134	291

ID	Intersection Name	Southbound			Eastbound			Westbound			Northeastbound			Total Volume
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
215	Chrysler Dr/Constitution Dr	190	6	1	66	285	9	2	2	78	1	234	14	888

ID	Intersection Name	Northbound		Southbound		Westbound			Total Volume
		Left	Thru	Thru	Right	Left	Thru	Right	
233	Sand Hill Circle/Sand Hill Road	9	54	87	180	5	1849	9	2193

ID	Intersection Name	Northbound		Southbound	Eastbound		Total Volume
		Thru	Right	Left	Left	Thru	
234	Sand Hill Rd/Hwy 280 NB Off-Ramp	23	193	129	38	697	1080

Menlo Park GP Circulation Update

Vistro File: J:\...\Menlo Park PM_update.vistro

Scenario 1: Existing PM

Report File: J:\...\Menlo Park_PM Results.pdf

1/9/2015

Turning Movement Volume: Detail

ID	Intersection Name	Volume Type	Northeastbound		Southwestbound		Southeastbound		Total Volume
			Thru		Thru		Left	Right	
1	Marsh Rd (SR 84)/US 101 SB Offramp	Final Base	980		893		1547	401	3821
		Growth Rate	1.00		1.00		1.00	1.00	-
		In Process	0		0		0	0	0
		Net New Trips	0		0		0	0	0
		Other	0		0		0	0	0
		Future Total		980		893		1547	401

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
2	Marsh Rd/Rolison Rd-Scott Dr	Final Base	29	1083	4	66	1014	203	27	9	467	229	13	1	3145	
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total		29	1083	4	66	1014	203	27	9	467	229	13	1	3145

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
3	Marsh Rd/Florence St-Bohannon Dr	Final Base	185	720	84	30	697	377	475	15	130	98	41	72	2924	
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total		185	720	84	30	697	377	475	15	130	98	41	72	2924

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
4	Marsh Rd/Bay Rd	Final Base	2	681	108	194	841	54	45	29	6	84	18	143	2205	
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total		2	681	108	194	841	54	45	29	6	84	18	143	2205

ID	Intersection Name	Volume Type	Northeastbound		Northwestbound		Southeastbound		Total Volume
			Left	Right	Left	Thru	Thru	Right	
9	Middlefield Rd/Ravenswood Ave	Final Base	199	610	457	673	357	60	2356
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	199	610	457	673	357	60	2356

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
10	Middlefield Rd/Ringwood Ave	Final Base	58	70	38	63	1	272	10	775	97	379	568	5	2336
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	58	70	38	63	1	272	10	775	97	379	568	5	2336

ID	Intersection Name	Volume Type	Northeastbound		Southwestbound		Northwestbound		Total Volume
			Thru	Right	Left	Thru	Left	Right	
15	Bayfront Expy (SR 84)/University Ave (SR 109)	Final Base	3800	62	304	813	78	1577	6634
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	3800	62	304	813	78	1577	6634

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
16	Bayfront Expy (SR 84)/Willow Rd (SR 114)	Final Base	24	46	1404	116	191	116	35	2362	75	359	675	10	5413	
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-	
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	
		Net New Trips	0	0	-300	0	0	0	0	0	0	0	0	0	0	-300
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	
		Future Total	24	46	1104	116	191	116	35	2362	75	359	675	10	5113	

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
17	Willow Rd (SR 114)/Hamilton Ave	Final Base	50	1299	3	13	623	30	163	15	39	139	17	61	2452
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	50	1299	3	13	623	30	163	15	39	139	17	61	2452

ID	Intersection Name	Volume Type	Northbound		Southbound		Eastbound		Total Volume
			Left	Thru	Thru	Right	Left	Right	
18	Willow Rd (SR 114)/Ivy Dr	Final Base	68	1339	885	15	33	141	2481
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	68	1339	885	15	33	141	2481

ID	Intersection Name	Volume Type	Northbound		Southbound		Westbound		Total Volume
			Thru	Right	Left	Thru	Left	Right	
19	Willow Rd (SR 114)/O'Brien Dr	Final Base	1368	217	72	966	218	44	2885
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	1368	217	72	966	218	44	2885

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
20	Willow Rd (SR 114)/Newbridge St	Final Base	301	1484	263	74	1016	22	28	137	210	210	249	76	4070
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	301	1484	263	74	1016	22	28	137	210	210	249	76	4070

ID	Intersection Name	Volume Type	Northeastbound		Southwestbound		Southeastbound		Total Volume
			Left	Thru	Thru	Right	Left	Right	
21	Willow Rd/Bay Rd	Final Base	35	1276	922	261	504	68	3066
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	35	1276	922	261	504	68	3066

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
22	Willow Rd/Durham St-VA Med Entrance	Final Base	8	935	10	54	719	29	117	2	28	21	2	97	2022
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	8	935	10	54	719	29	117	2	28	21	2	97	2022

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
23	Willow Rd/Coleman Ave	Final Base	11	738	3	5	680	84	102	4	31	1	1	4	1664
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	11	738	3	5	680	84	102	4	31	1	1	4	1664

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
24	Willow Rd/Gilbert Ave	Final Base	30	671	13	26	597	6	61	51	69	56	43	28	1651
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	30	671	13	26	597	6	61	51	69	56	43	28	1651

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
25	Middlefield Rd-Willow Rd	Final Base	46	94	256	356	93	332	92	429	216	236	453	29	2632
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	46	94	256	356	93	332	92	429	216	236	453	29	2632

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
26	Ravenswood Ave/Laurel St	Final Base	57	596	125	32	523	24	267	215	64	52	135	26	2116
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	57	596	125	32	523	24	267	215	64	52	135	26	2116

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
28	Oak Grove Ave/Laurel St	Final Base	13	335	94	30	262	48	78	232	38	28	116	29	1303
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	13	335	94	30	262	48	78	232	38	28	116	29	1303

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
29	El Camino Real (SR 82)/Encinal Ave-Menlo College Entrance	Final Base	19	15	31	107	9	238	24	1750	100	65	1215	13	3586
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	19	15	31	107	9	238	24	1750	100	65	1215	13	3586

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
30	El Camino Real (SR 82)/Glenwood Ave-Valparaiso Ave	Final Base	363	144	79	59	171	41	98	1525	46	56	992	304	3878
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	363	144	79	59	171	41	98	1525	46	56	992	304	3878

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
31	El Camino Real (SR 82)/Oak Grove Ave	Final Base	118	202	133	129	215	77	87	1426	94	114	1047	97	3739
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	118	202	133	129	215	77	87	1426	94	114	1047	97	3739

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound		Southeastbound		Total Volume
			Left	Thru	Right	Left	Thru	Right	Thru	Right	Thru	Right	
32	El Camino Real (SR 82)/Santa Cruz Ave	Final Base	172	50	137	50	58	43	1406	41	1451	87	3495
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0
		Future Total	172	50	137	50	58	43	1406	41	1451	87	3495

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
33	El Camino Real (SR 82)/Ravenswood Ave-Menlo Ave	Final Base	44	348	108	451	249	80	113	1339	590	180	1154	39	4695
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	44	348	108	451	249	80	113	1339	590	180	1154	39	4695

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
34	El Camino Real (SR 82)/Roble Ave	Final Base	66	6	36	58	45	48	72	1845	20	68	1586	54	3904
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	66	6	36	58	45	48	72	1845	20	68	1586	54	3904

ID	Intersection Name	Volume Type	Northeastbound		Northwestbound		Southeastbound		Total Volume
			Left	Right	Left	Thru	Thru	Right	
35	El Camino Real (SR 82)/Middle Ave	Final Base	249	182	323	1917	1334	86	4091
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	249	182	323	1917	1334	86	4091

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
36	El Camino Real (SR 82)/Cambridge Ave	Final Base	26	0	39	4	3	0	310	2332	6	30	1660	4	4414
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	26	0	39	4	3	0	310	2332	6	30	1660	4	4414

ID	Intersection Name	Volume Type	Northeastbound		Southwestbound		Northwestbound		Total Volume
			Thru	Right	Left	Thru	Left	Right	
38	Santa Cruz Ave/University Dr (S)	Final Base	350	328	88	398	363	165	1692
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	350	328	88	398	363	165	1692

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
39	Sand Hill Rd/Santa Cruz Ave	Final Base	361	611	215	424	1038	208	158	691	188	101	526	178	4699
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	361	611	215	424	1038	208	158	691	188	101	526	178	4699

ID	Intersection Name	Volume Type	Northbound		Southbound		Eastbound			Total Volume
			Left	Thru	Thru	Right	Left	Thru	Right	
58	University Avenue and Adams Drive	Final Base	11	1786	360	15	25	0	26	2223
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0
		Future Total	11	1786	360	15	25	0	26	2223

ID	Intersection Name	Volume Type	Northbound		Southbound		Eastbound		Total Volume
			Left	Thru	Thru	Right	Left	Right	
74	University Ave/O'Brien Dr	Final Base	11	1724	356	10	171	77	2349
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	11	1724	356	10	171	77	2349

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
88	Valparaiso Ave/University Dr	Final Base	24	467	105	44	456	29	182	24	57	45	41	72	1546
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	24	467	105	44	456	29	182	24	57	45	41	72	1546

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
103	Addison Wesley/Sand Hill Rd	Final Base	209	3	108	19	3	66	18	794	93	69	1750	16	3148	
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	209	3	108	19	3	66	18	794	93	69	1750	16	3148	

ID	Intersection Name	Volume Type	Southbound		Westbound		Northeastbound		Total Volume
			Left	Thru	Left	Right	Thru	Right	
107	Alpine Rd/Santa Cruz Ave&Junipero Serra Blvd	Final Base	373	638	423	398	535	100	2467
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	373	638	423	398	535	100	2467

ID	Intersection Name	Volume Type	Northbound	Southbound	Northwestbound		Total Volume
			Thru	Thru	Left	Right	
110	Marsh Road/101 NB Ramps	Final Base	2255	567	544	239	3605
		Growth Rate	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0
		Net New Trips	0	0	0	0	0
		Other	0	0	0	0	0
		Future Total	2255	567	544	239	3605

ID	Intersection Name	Volume Type	Northeastbound	Southwestbound		Southeastbound		Total Volume
			Thru	Thru	Right	Left	Right	
132	Oak Ave/Sand Hill Rd	Final Base	912	1585	100	29	88	2714
		Growth Rate	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0
		Other	0	0	0	0	0	0
		Future Total	912	1585	100	29	88	2714

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
156	Saga Ln/Sand Hill Rd	Final Base	108	0	113	90	0	103	65	945	6	28	1307	20	2785
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	108	0	113	90	0	103	65	945	6	28	1307	20	2785

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
157	Branner Dr/Sand Hill Rd	Final Base	2	0	17	20	0	43	22	1076	7	17	1321	24	2549
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	2	0	17	20	0	43	22	1076	7	17	1321	24	2549

ID	Intersection Name	Volume Type	Northeastbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
162	Sharon Park Dr/ Sand Hill Rd	Final Base	164	916	1	16	1220	252	12	6	17	178	3	172	2957
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	164	916	1	16	1220	252	12	6	17	178	3	172	2957

ID	Intersection Name	Volume Type	Northbound			Eastbound			Westbound			Southwestbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
163	Bayfront Expy/Marsh Rd	Final Base	125	31	2131	8	477	136	1287	63	13	16	25	3	4315
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	125	31	2131	8	477	136	1287	63	13	16	25	3	4315

ID	Intersection Name	Volume Type	Northeastbound		Southwestbound		Southeastbound		Total Volume
			Left	Thru	Thru	Right	Left	Right	
181	Santa Cruz Ave/Elder Ave	Final Base	61	669	641	79	46	41	1537
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	61	669	641	79	46	41	1537

ID	Intersection Name	Volume Type	Northbound		Westbound		Southeastbound		Total Volume
			Left	Right	Left	Thru	Thru	Right	
195	Bayfront Expy/Chilco St	Final Base	201	196	19	767	2416	156	3755
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	201	196	19	767	2416	156	3755

ID	Intersection Name	Volume Type	Northbound		Eastbound		Westbound		Total Volume
			Left	Right	Thru	Right	Left	Thru	
196	Bayfront Expy/Chrysler Drive	Final Base	382	43	2516	107	5	978	4031
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	382	43	2516	107	5	978	4031

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Northwestbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
207	Chilco St/Constitution Dr	Final Base	14	136	5	17	160	2	28	2	114	158	10	398	1044
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	14	136	5	17	160	2	28	2	114	158	10	398	1044

ID	Intersection Name	Volume Type	Southbound			Eastbound			Westbound			Northeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
209	Jefferson Dr/Constitution Dr	Final Base	11	0	4	2	481	3	2	24	1	13	0	56	597
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	11	0	4	2	481	3	2	24	1	13	0	56	597

ID	Intersection Name	Volume Type	Northbound			Southwestbound			Northwestbound			Southeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
213	Chrysler Dr/Independence Dr	Final Base	0	101	0	1	2	3	0	0	15	35	3	6	166
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	0	101	0	1	2	3	0	0	15	35	3	6	166

ID	Intersection Name	Volume Type	Southbound		Northeastbound		Northwestbound		Total Volume
			Left	Thru	Thru	Right	Left	Right	
214	Chrysler Dr/Jefferson Dr	Final Base	9	7	132	7	2	134	291
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0
		Future Total	9	7	132	7	2	134	291

ID	Intersection Name	Volume Type	Southbound			Eastbound			Westbound			Northeastbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
215	Chrysler Dr/Constitution Dr	Final Base	190	6	1	66	285	9	2	2	78	1	234	14	888
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		Future Total	190	6	1	66	285	9	2	2	78	1	234	14	888

ID	Intersection Name	Volume Type	Northbound		Southbound		Westbound			Total Volume
			Left	Thru	Thru	Right	Left	Thru	Right	
233	Sand Hill Circle/Sand Hill Road	Final Base	9	54	87	180	5	1849	9	2193
		Growth Rate	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0
		Future Total	9	54	87	180	5	1849	9	2193

ID	Intersection Name	Volume Type	Northbound		Southbound	Eastbound		Total Volume
			Thru	Right	Left	Left	Thru	
234	Sand Hill Rd/Hwy 280 NB Off-Ramp	Final Base	23	193	129	38	697	1080
		Growth Rate	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0
		Other	0	0	0	0	0	0
		Future Total	23	193	129	38	697	1080

Signal Warrants Report For Intersection #58: University Avenue and Adams Drive

Warrants Summary

Warrant	Name	Met?
#1	Eight Hour Vehicular Volume	No
#2	Four Hour Vehicular Volume	No
#3	Peak Hour	No

Intersection Warrants Parameters

Major Approaches	S, N
Minor Approaches	W
Speed > 40mph	No
Population < 10,000	No
Warrant Factor	100%

Warrant Analysis Traffic Volumes

Hour	Major Streets		Minor Streets
	S	N	W
1	36	8	1
2	36	8	1
3	54	11	2
4	54	11	2
5	72	15	2
6	180	38	5
7	198	41	6
8	359	75	10
9	629	131	18
10	647	135	18
11	647	135	18
12	701	146	20
13	773	161	22
14	809	169	23
15	809	169	23
16	863	180	24
17	1078	225	31
18	1132	236	32
19	1222	255	35
20	1366	285	39
21	1438	300	41
22	1689	353	48
23	1725	360	49
24	1797	375	51

Warrant Analysis by Hour

Hour	Major Lanes		Minor Lanes		Warrant 1 Condition A				Warrant 1 Condition B				Warrant 2	Warrant 3 Condition B
	Number	Volume	Number	Volume	100%	80%	70%	56%	100%	80%	70%	56%		
1	5	44	1	1	No	No	No	No	No	No	No	No	No	No
2	5	44	1	1	No	No	No	No	No	No	No	No	No	No
3	5	65	1	2	No	No	No	No	No	No	No	No	No	No
4	5	65	1	2	No	No	No	No	No	No	No	No	No	No
5	5	87	1	2	No	No	No	No	No	No	No	No	No	No
6	5	218	1	5	No	No	No	No	No	No	No	No	No	No
7	5	239	1	6	No	No	No	No	No	No	No	No	No	No
8	5	434	1	10	No	No	No	No	No	No	No	No	No	No
9	5	760	1	18	No	No	No	No	No	No	No	No	No	No
10	5	782	1	18	No	No	No	No	No	No	No	No	No	No
11	5	782	1	18	No	No	No	No	No	No	No	No	No	No
12	5	847	1	20	No	No	No	No	No	No	No	No	No	No
13	5	934	1	22	No	No	No	No	No	No	No	No	No	No
14	5	978	1	23	No	No	No	No	No	No	No	No	No	No
15	5	978	1	23	No	No	No	No	No	No	No	No	No	No
16	5	1043	1	24	No	No	No	No	No	No	No	No	No	No
17	5	1303	1	31	No	No	No	No	No	No	No	No	No	No
18	5	1368	1	32	No	No	No	No	No	No	No	No	No	No
19	5	1477	1	35	No	No	No	No	No	No	No	No	No	No
20	5	1651	1	39	No	No	No	No	No	No	No	No	No	No
21	5	1738	1	41	No	No	No	No	No	No	No	No	No	No
22	5	2042	1	48	No	No	No	No	No	No	No	Yes	No	No
23	5	2085	1	49	No	No	No	No	No	No	No	Yes	No	No
24	5	2172	1	51	No	No	No	No	No	No	No	Yes	No	No
Hours Met					0	0	0	0	0	0	0	3	0	0

Warrant 3 Condition A

Orientation	W
Total Stopped Delay Per Vehicle on Minor Approach (s)	27.3
Number of Lanes on Minor Street Approach	1
VehicleHours of Stopped Delay on Minor Approach ([h]h:mm)	0:23
Delay Condition Met	No
Volume on Minor Street Approach During Same Hour	51
High Minor Volume Condition Met	No
Total Entering Volume on All Approaches During Same Hour	2223
Number of Approaches on Intersection	3
Total Volume Condition Met	Yes
Warrant Met for Approach	No
Warrant Met for Intersection	No

Signal Warrants Report For Intersection #207: Chilco St/Constitution Dr

Warrants Summary

Warrant	Name	Met?
#1	Eight Hour Vehicular Volume	No
#2	Four Hour Vehicular Volume	No
#3	Peak Hour	No

Intersection Warrants Parameters

Major Approaches	SE, W
Minor Approaches	N, S
Speed > 40mph	No
Population < 10,000	No
Warrant Factor	100%

Warrant Analysis Traffic Volumes

Hour	Major Streets		Minor Streets	
	SE	W	N	S
1	11	3	4	3
2	11	3	4	3
3	17	4	5	5
4	17	4	5	5
5	23	6	7	6
6	57	14	18	16
7	62	16	20	17
8	113	29	36	31
9	198	50	63	54
10	204	52	64	56
11	204	52	64	56
12	221	56	70	60
13	243	62	77	67
14	255	65	81	70
15	255	65	81	70
16	272	69	86	74
17	340	86	107	93
18	357	91	113	98
19	385	98	122	105
20	430	109	136	118
21	453	115	143	124
22	532	135	168	146
23	543	138	172	149
24	566	144	179	155

Warrant Analysis by Hour

Hour	Major Lanes		Minor Lanes		Warrant 1 Condition A				Warrant 1 Condition B				Warrant 2	Warrant 3 Condition B
	Number	Volume	Number	Volume	100%	80%	70%	56%	100%	80%	70%	56%		
1	2	14	2	7	No	No	No	No	No	No	No	No	No	No
2	2	14	2	7	No	No	No	No	No	No	No	No	No	No
3	2	21	2	10	No	No	No	No	No	No	No	No	No	No
4	2	21	2	10	No	No	No	No	No	No	No	No	No	No
5	2	29	2	13	No	No	No	No	No	No	No	No	No	No
6	2	71	2	34	No	No	No	No	No	No	No	No	No	No
7	2	78	2	37	No	No	No	No	No	No	No	No	No	No
8	2	142	2	67	No	No	No	No	No	No	No	No	No	No
9	2	248	2	117	No	No	No	No	No	No	No	No	No	No
10	2	256	2	120	No	No	No	No	No	No	No	No	No	No
11	2	256	2	120	No	No	No	No	No	No	No	No	No	No
12	2	277	2	130	No	No	No	No	No	No	No	No	No	No
13	2	305	2	144	No	No	No	No	No	No	No	No	No	No
14	2	320	2	151	No	No	No	No	No	No	No	No	No	No
15	2	320	2	151	No	No	No	No	No	No	No	No	No	No
16	2	341	2	160	No	No	No	Yes	No	No	No	No	No	No
17	2	426	2	200	No	No	Yes	Yes	No	No	No	No	No	No
18	2	448	2	211	No	No	Yes	Yes	No	No	No	No	No	No
19	2	483	2	227	No	Yes	Yes	Yes	No	No	No	No	No	No
20	2	539	2	254	No	Yes	Yes	Yes	No	No	No	Yes	No	No
21	2	568	2	267	No	Yes	Yes	Yes	No	No	No	Yes	No	No
22	2	667	2	314	Yes	Yes	Yes	Yes	No	No	Yes	Yes	Yes	No
23	2	681	2	321	Yes	Yes	Yes	Yes	No	No	Yes	Yes	Yes	No
24	2	710	2	334	Yes	Yes	Yes	Yes	No	No	Yes	Yes	Yes	No
Hours Met					3	6	8	9	0	0	3	5	3	0

Warrant 3 Condition A

Orientation	N	S
Total Stopped Delay Per Vehicle on Minor Approach (s)	13.1	12.4
Number of Lanes on Minor Street Approach	1	1
VehicleHours of Stopped Delay on Minor Approach ([h]h:mm)	0:38	0:32
Delay Condition Met	No	No
Volume on Minor Street Approach During Same Hour	179	155
High Minor Volume Condition Met	Yes	Yes
Total Entering Volume on All Approaches During Same Hour	1044	1044
Number of Approaches on Intersection	4	4
Total Volume Condition Met	Yes	Yes
Warrant Met for Approach	No	No
Warrant Met for Intersection	No	

Signal Warrants Report For Intersection #209: Jefferson Dr/Constitution Dr

Warrants Summary

Warrant	Name	Met?
#1	Eight Hour Vehicular Volume	No
#2	Four Hour Vehicular Volume	No
#3	Peak Hour	No

Intersection Warrants Parameters

Major Approaches	E, W
Minor Approaches	N, SW
Speed > 40mph	No
Population < 10,000	No
Warrant Factor	100%

Warrant Analysis Traffic Volumes

Hour	Major Streets		Minor Streets	
	E	W	N	SW
1	1	10	0	1
2	1	10	0	1
3	1	15	0	2
4	1	15	0	2
5	1	19	1	3
6	3	49	2	7
7	3	53	2	8
8	5	97	3	14
9	9	170	5	24
10	10	175	5	25
11	10	175	5	25
12	11	190	6	27
13	12	209	6	30
14	12	219	7	31
15	12	219	7	31
16	13	233	7	33
17	16	292	9	41
18	17	306	9	43
19	18	330	10	47
20	21	369	11	52
21	22	389	12	55
22	25	457	14	65
23	26	467	14	66
24	27	486	15	69

Warrant Analysis by Hour

Hour	Major Lanes		Minor Lanes		Warrant 1 Condition A				Warrant 1 Condition B				Warrant 2	Warrant 3 Condition B
	Number	Volume	Number	Volume	100%	80%	70%	56%	100%	80%	70%	56%		
1	2	11	2	1	No	No	No	No	No	No	No	No	No	No
2	2	11	2	1	No	No	No	No	No	No	No	No	No	No
3	2	16	2	2	No	No	No	No	No	No	No	No	No	No
4	2	16	2	2	No	No	No	No	No	No	No	No	No	No
5	2	20	2	4	No	No	No	No	No	No	No	No	No	No
6	2	52	2	9	No	No	No	No	No	No	No	No	No	No
7	2	56	2	10	No	No	No	No	No	No	No	No	No	No
8	2	102	2	17	No	No	No	No	No	No	No	No	No	No
9	2	179	2	29	No	No	No	No	No	No	No	No	No	No
10	2	185	2	30	No	No	No	No	No	No	No	No	No	No
11	2	185	2	30	No	No	No	No	No	No	No	No	No	No
12	2	201	2	33	No	No	No	No	No	No	No	No	No	No
13	2	221	2	36	No	No	No	No	No	No	No	No	No	No
14	2	231	2	38	No	No	No	No	No	No	No	No	No	No
15	2	231	2	38	No	No	No	No	No	No	No	No	No	No
16	2	246	2	40	No	No	No	No	No	No	No	No	No	No
17	2	308	2	50	No	No	No	No	No	No	No	No	No	No
18	2	323	2	52	No	No	No	No	No	No	No	No	No	No
19	2	348	2	57	No	No	No	No	No	No	No	No	No	No
20	2	390	2	63	No	No	No	No	No	No	No	No	No	No
21	2	411	2	67	No	No	No	No	No	No	No	No	No	No
22	2	482	2	79	No	No	No	No	No	No	No	No	No	No
23	2	493	2	80	No	No	No	No	No	No	No	No	No	No
24	2	513	2	84	No	No	No	No	No	No	No	Yes	No	No
Hours Met					0	0	0	0	0	0	0	1	0	0

Warrant 3 Condition A

Orientation	N	SW
Total Stopped Delay Per Vehicle on Minor Approach (s)	13.6	13.6
Number of Lanes on Minor Street Approach	1	1
VehicleHours of Stopped Delay on Minor Approach ([h]h:mm)	0:03	0:15
Delay Condition Met	No	No
Volume on Minor Street Approach During Same Hour	15	69
High Minor Volume Condition Met	No	No
Total Entering Volume on All Approaches During Same Hour	597	597
Number of Approaches on Intersection	4	4
Total Volume Condition Met	No	No
Warrant Met for Approach	No	No
Warrant Met for Intersection	No	

Signal Warrants Report For Intersection #213: Chrysler Dr/Independence Dr

Warrants Summary

Warrant	Name	Met?
#1	Eight Hour Vehicular Volume	No
#2	Four Hour Vehicular Volume	No
#3	Peak Hour	No

Intersection Warrants Parameters

Major Approaches	NE, S
Minor Approaches	SE, NW
Speed > 40mph	No
Population < 10,000	No
Warrant Factor	100%

Warrant Analysis Traffic Volumes

Hour	Major Streets		Minor Streets	
	NE	S	SE	NW
1	0	2	0	1
2	0	2	0	1
3	0	3	0	1
4	0	3	0	1
5	0	4	1	2
6	1	10	2	4
7	1	11	2	5
8	1	20	3	9
9	2	35	5	15
10	2	36	5	16
11	2	36	5	16
12	2	39	6	17
13	3	43	6	19
14	3	45	7	20
15	3	45	7	20
16	3	48	7	21
17	4	61	9	26
18	4	64	9	28
19	4	69	10	30
20	5	77	11	33
21	5	81	12	35
22	6	95	14	41
23	6	97	14	42
24	6	101	15	44

Warrant Analysis by Hour

Hour	Major Lanes		Minor Lanes		Warrant 1 Condition A				Warrant 1 Condition B				Warrant 2	Warrant 3 Condition B
	Number	Volume	Number	Volume	100%	80%	70%	56%	100%	80%	70%	56%		
1	2	2	2	1	No	No	No	No	No	No	No	No	No	No
2	2	2	2	1	No	No	No	No	No	No	No	No	No	No
3	2	3	2	1	No	No	No	No	No	No	No	No	No	No
4	2	3	2	1	No	No	No	No	No	No	No	No	No	No
5	2	4	2	3	No	No	No	No	No	No	No	No	No	No
6	2	11	2	6	No	No	No	No	No	No	No	No	No	No
7	2	12	2	7	No	No	No	No	No	No	No	No	No	No
8	2	21	2	12	No	No	No	No	No	No	No	No	No	No
9	2	37	2	20	No	No	No	No	No	No	No	No	No	No
10	2	38	2	21	No	No	No	No	No	No	No	No	No	No
11	2	38	2	21	No	No	No	No	No	No	No	No	No	No
12	2	41	2	23	No	No	No	No	No	No	No	No	No	No
13	2	46	2	25	No	No	No	No	No	No	No	No	No	No
14	2	48	2	27	No	No	No	No	No	No	No	No	No	No
15	2	48	2	27	No	No	No	No	No	No	No	No	No	No
16	2	51	2	28	No	No	No	No	No	No	No	No	No	No
17	2	65	2	35	No	No	No	No	No	No	No	No	No	No
18	2	68	2	37	No	No	No	No	No	No	No	No	No	No
19	2	73	2	40	No	No	No	No	No	No	No	No	No	No
20	2	82	2	44	No	No	No	No	No	No	No	No	No	No
21	2	86	2	47	No	No	No	No	No	No	No	No	No	No
22	2	101	2	55	No	No	No	No	No	No	No	No	No	No
23	2	103	2	56	No	No	No	No	No	No	No	No	No	No
24	2	107	2	59	No	No	No	No	No	No	No	No	No	No
Hours Met					0	0	0	0	0	0	0	0	0	0

Warrant 3 Condition A

Orientation	SE	NW
Total Stopped Delay Per Vehicle on Minor Approach (s)	8.9	9.6
Number of Lanes on Minor Street Approach	1	1
VehicleHours of Stopped Delay on Minor Approach ([h]h:mm)	0:02	0:07
Delay Condition Met	No	No
Volume on Minor Street Approach During Same Hour	15	44
High Minor Volume Condition Met	No	No
Total Entering Volume on All Approaches During Same Hour	166	166
Number of Approaches on Intersection	4	4
Total Volume Condition Met	No	No
Warrant Met for Approach	No	No
Warrant Met for Intersection	No	

Signal Warrants Report For Intersection #214: Chrysler Dr/Jefferson Dr

Warrants Summary

Warrant	Name	Met?
#1	Eight Hour Vehicular Volume	No
#2	Four Hour Vehicular Volume	No
#3	Peak Hour	No

Intersection Warrants Parameters

Major Approaches	N, SW
Minor Approaches	SE
Speed > 40mph	No
Population < 10,000	No
Warrant Factor	100%

Warrant Analysis Traffic Volumes

Hour	Major Streets		Minor Streets
	N	SW	SE
1	0	3	3
2	0	3	3
3	0	4	4
4	0	4	4
5	1	6	5
6	2	14	14
7	2	15	15
8	3	28	27
9	6	49	48
10	6	50	49
11	6	50	49
12	6	54	53
13	7	60	58
14	7	63	61
15	7	63	61
16	8	67	65
17	10	83	82
18	10	88	86
19	11	95	92
20	12	106	103
21	13	111	109
22	15	131	128
23	15	133	131
24	16	139	136

Warrant Analysis by Hour

Hour	Major Lanes		Minor Lanes		Warrant 1 Condition A				Warrant 1 Condition B				Warrant 2	Warrant 3 Condition B
	Number	Volume	Number	Volume	100%	80%	70%	56%	100%	80%	70%	56%		
1	2	3	1	3	No	No	No	No	No	No	No	No	No	No
2	2	3	1	3	No	No	No	No	No	No	No	No	No	No
3	2	4	1	4	No	No	No	No	No	No	No	No	No	No
4	2	4	1	4	No	No	No	No	No	No	No	No	No	No
5	2	7	1	5	No	No	No	No	No	No	No	No	No	No
6	2	16	1	14	No	No	No	No	No	No	No	No	No	No
7	2	17	1	15	No	No	No	No	No	No	No	No	No	No
8	2	31	1	27	No	No	No	No	No	No	No	No	No	No
9	2	55	1	48	No	No	No	No	No	No	No	No	No	No
10	2	56	1	49	No	No	No	No	No	No	No	No	No	No
11	2	56	1	49	No	No	No	No	No	No	No	No	No	No
12	2	60	1	53	No	No	No	No	No	No	No	No	No	No
13	2	67	1	58	No	No	No	No	No	No	No	No	No	No
14	2	70	1	61	No	No	No	No	No	No	No	No	No	No
15	2	70	1	61	No	No	No	No	No	No	No	No	No	No
16	2	75	1	65	No	No	No	No	No	No	No	No	No	No
17	2	93	1	82	No	No	No	No	No	No	No	No	No	No
18	2	98	1	86	No	No	No	No	No	No	No	No	No	No
19	2	106	1	92	No	No	No	No	No	No	No	No	No	No
20	2	118	1	103	No	No	No	No	No	No	No	No	No	No
21	2	124	1	109	No	No	No	No	No	No	No	No	No	No
22	2	146	1	128	No	No	No	No	No	No	No	No	No	No
23	2	148	1	131	No	No	No	No	No	No	No	No	No	No
24	2	155	1	136	No	No	No	No	No	No	No	No	No	No
Hours Met					0	0	0	0	0	0	0	0	0	0

Warrant 3 Condition A

Orientation	SE
Total Stopped Delay Per Vehicle on Minor Approach (s)	9.9
Number of Lanes on Minor Street Approach	1
VehicleHours of Stopped Delay on Minor Approach ([h]h:mm)	0:22
Delay Condition Met	No
Volume on Minor Street Approach During Same Hour	136
High Minor Volume Condition Met	Yes
Total Entering Volume on All Approaches During Same Hour	291
Number of Approaches on Intersection	3
Total Volume Condition Met	No
Warrant Met for Approach	No
Warrant Met for Intersection	No

Signal Warrants Report For Intersection #215: Chrysler Dr/Constitution Dr

Warrants Summary

Warrant	Name	Met?
#1	Eight Hour Vehicular Volume	No
#2	Four Hour Vehicular Volume	No
#3	Peak Hour	No

Intersection Warrants Parameters

Major Approaches	E, W
Minor Approaches	N, SW
Speed > 40mph	No
Population < 10,000	No
Warrant Factor	100%

Warrant Analysis Traffic Volumes

Hour	Major Streets		Minor Streets	
	E	W	N	SW
1	2	7	4	5
2	2	7	4	5
3	2	11	6	7
4	2	11	6	7
5	3	14	8	10
6	8	36	20	25
7	9	40	22	27
8	16	72	39	50
9	29	126	69	87
10	30	130	71	90
11	30	130	71	90
12	32	140	77	97
13	35	155	85	107
14	37	162	89	112
15	37	162	89	112
16	39	173	95	120
17	49	216	118	149
18	52	227	124	157
19	56	245	134	169
20	62	274	150	189
21	66	288	158	199
22	77	338	185	234
23	79	346	189	239
24	82	360	197	249

Warrant Analysis by Hour

Hour	Major Lanes		Minor Lanes		Warrant 1 Condition A				Warrant 1 Condition B				Warrant 2	Warrant 3 Condition B
	Number	Volume	Number	Volume	100%	80%	70%	56%	100%	80%	70%	56%		
1	2	9	2	9	No	No	No	No	No	No	No	No	No	No
2	2	9	2	9	No	No	No	No	No	No	No	No	No	No
3	2	13	2	13	No	No	No	No	No	No	No	No	No	No
4	2	13	2	13	No	No	No	No	No	No	No	No	No	No
5	2	17	2	18	No	No	No	No	No	No	No	No	No	No
6	2	44	2	45	No	No	No	No	No	No	No	No	No	No
7	2	49	2	49	No	No	No	No	No	No	No	No	No	No
8	2	88	2	89	No	No	No	No	No	No	No	No	No	No
9	2	155	2	156	No	No	No	No	No	No	No	No	No	No
10	2	160	2	161	No	No	No	No	No	No	No	No	No	No
11	2	160	2	161	No	No	No	No	No	No	No	No	No	No
12	2	172	2	174	No	No	No	No	No	No	No	No	No	No
13	2	190	2	192	No	No	No	No	No	No	No	No	No	No
14	2	199	2	201	No	No	No	No	No	No	No	No	No	No
15	2	199	2	201	No	No	No	No	No	No	No	No	No	No
16	2	212	2	215	No	No	No	No	No	No	No	No	No	No
17	2	265	2	267	No	No	No	No	No	No	No	No	No	No
18	2	279	2	281	No	No	No	No	No	No	No	No	No	No
19	2	301	2	303	No	No	No	No	No	No	No	No	No	No
20	2	336	2	339	No	No	No	Yes	No	No	No	No	No	No
21	2	354	2	357	No	No	No	Yes	No	No	No	No	No	No
22	2	415	2	419	No	No	No	Yes	No	No	No	No	No	No
23	2	425	2	428	No	No	Yes	Yes	No	No	No	No	No	No
24	2	442	2	446	No	No	Yes	Yes	No	No	No	No	No	No
Hours Met					0	0	2	5	0	0	0	0	0	0

Warrant 3 Condition A

Orientation	N	SW
Total Stopped Delay Per Vehicle on Minor Approach (s)	12.6	13.2
Number of Lanes on Minor Street Approach	1	1
VehicleHours of Stopped Delay on Minor Approach ([h]h:mm)	0:41	0:54
Delay Condition Met	No	No
Volume on Minor Street Approach During Same Hour	197	249
High Minor Volume Condition Met	Yes	Yes
Total Entering Volume on All Approaches During Same Hour	888	888
Number of Approaches on Intersection	4	4
Total Volume Condition Met	Yes	Yes
Warrant Met for Approach	No	No
Warrant Met for Intersection	No	

Study Intersections

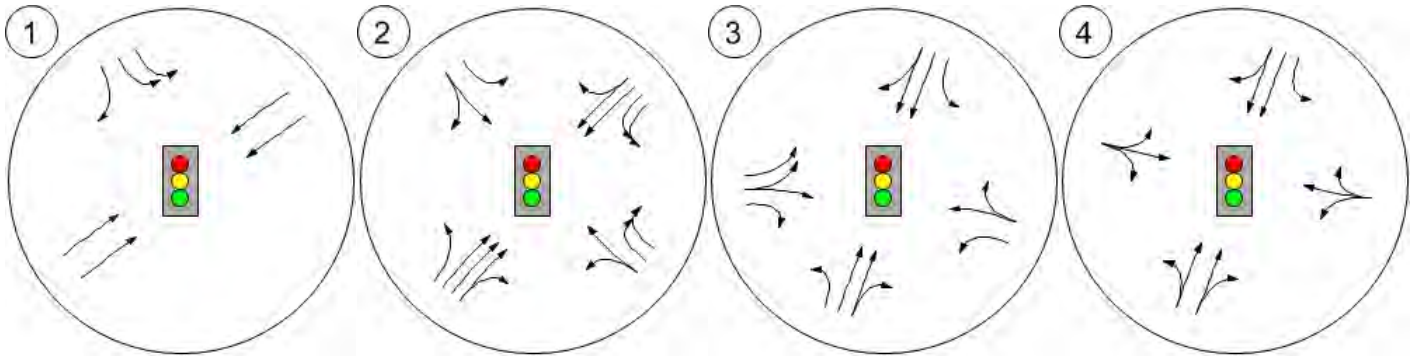


Lane Configuration and Traffic Control



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

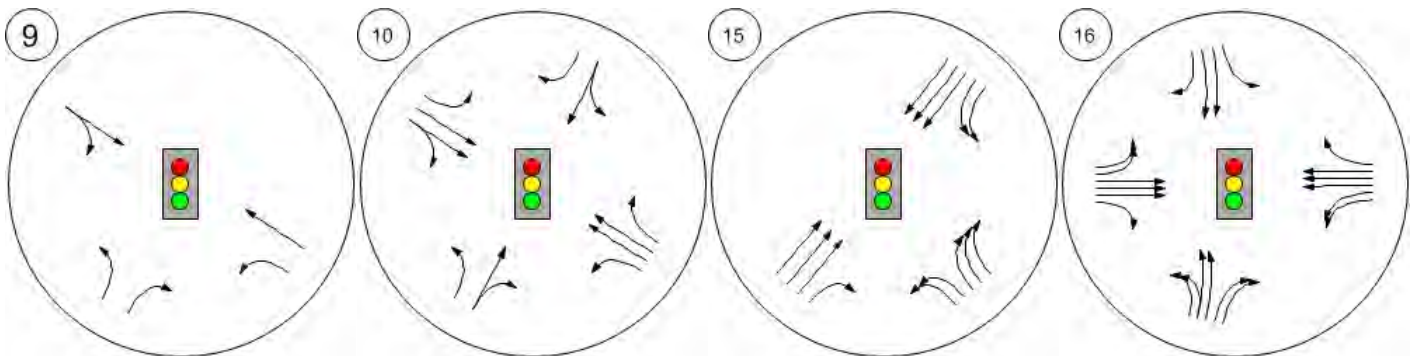


Middlefield Rd/Ravenswood

Middlefield Rd/Ringwood Ave

Bayfront Expy (SR 84)/Univer

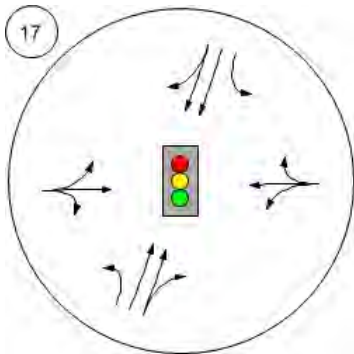
Bayfront Expy (SR 84)/Willow



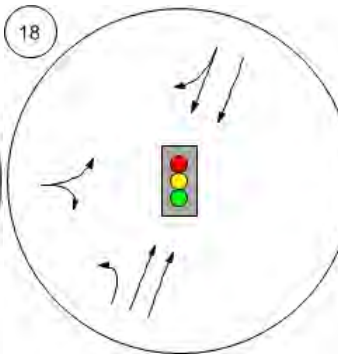
Lane Configuration and Traffic Control



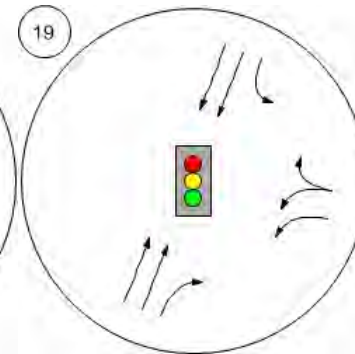
Willow Rd (SR 114)/Hamilton



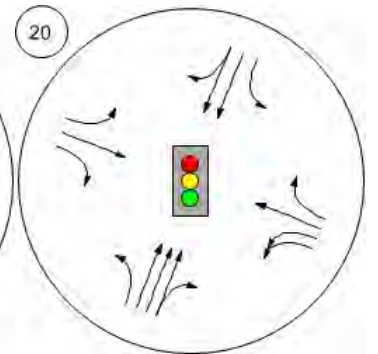
Willow Rd (SR 114)/Ivy Dr



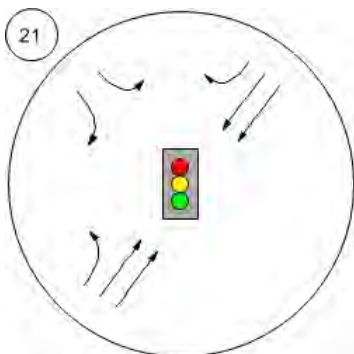
Willow Rd (SR 114)/O'Brien



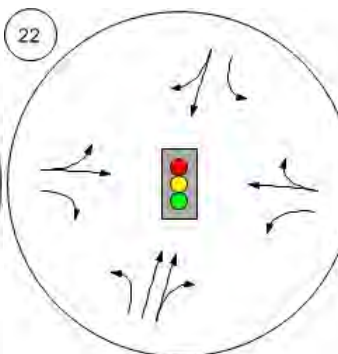
Willow Rd (SR 114)/Newbrid



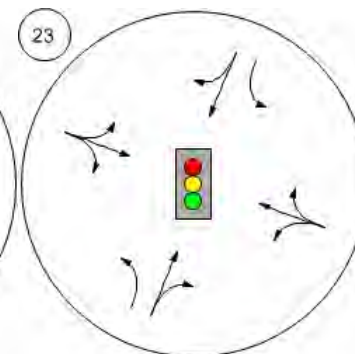
Willow Rd/Bay Rd



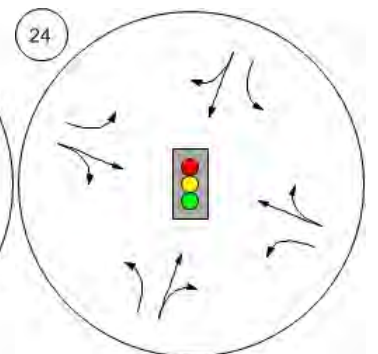
Willow Rd/Durham St-VA Me



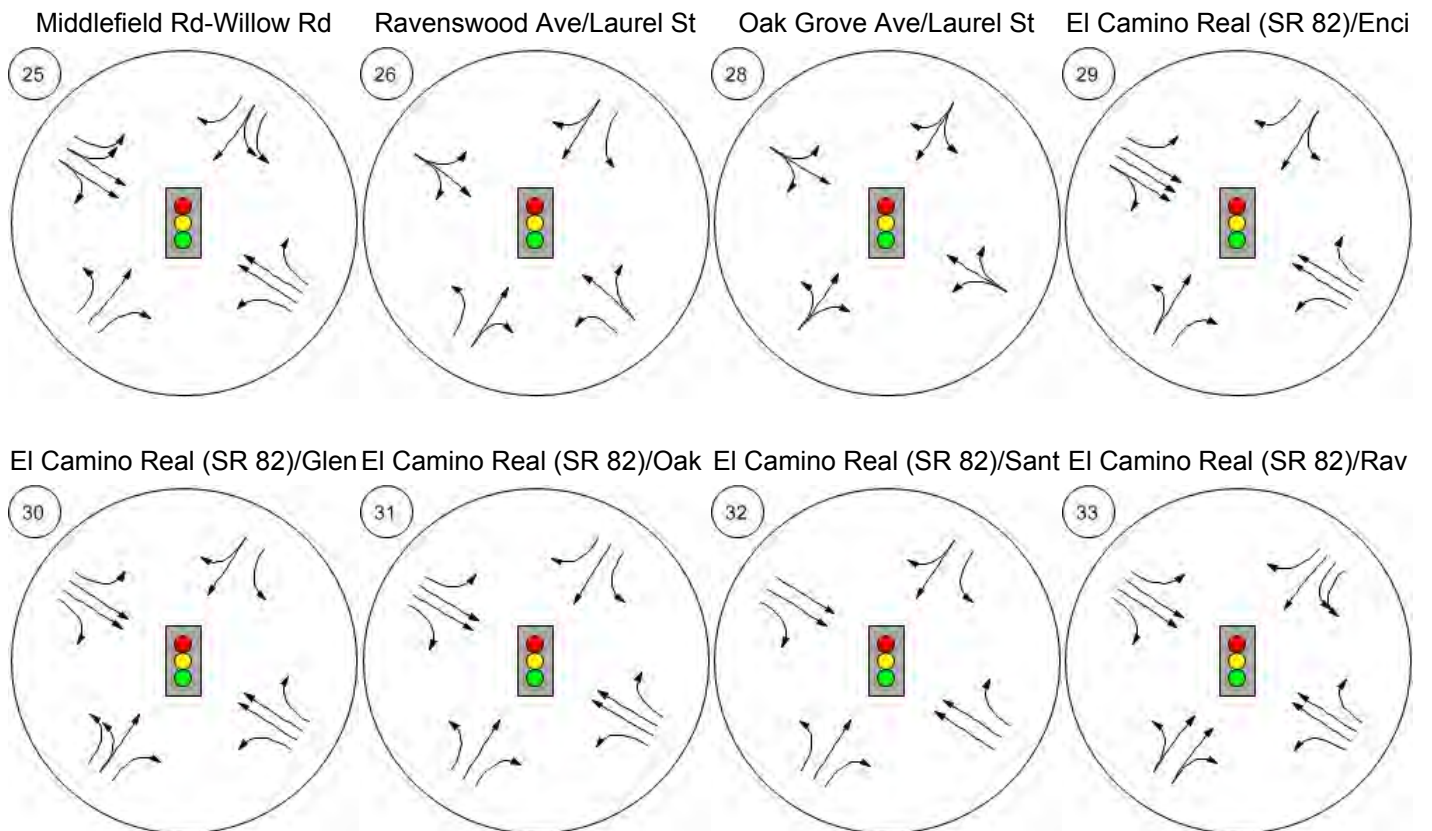
Willow Rd/Coleman Ave



Willow Rd/Gilbert Ave



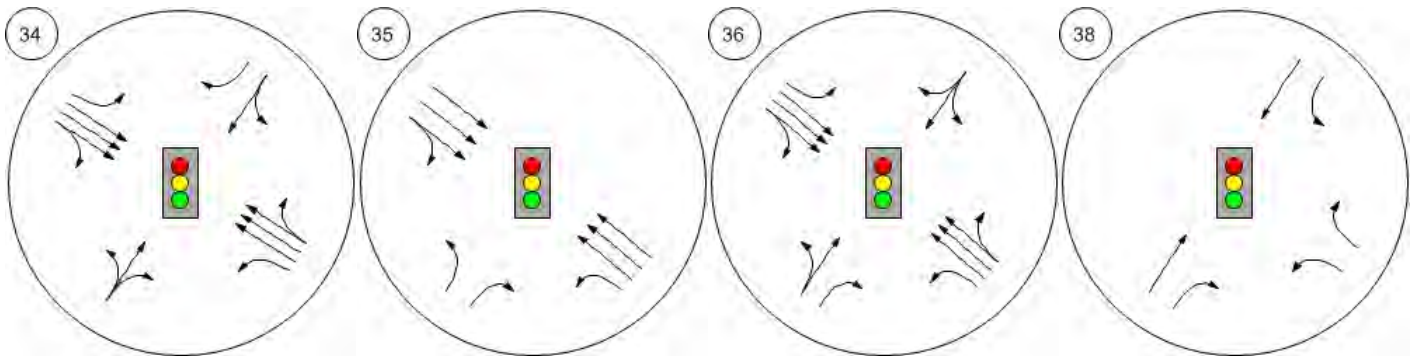
Lane Configuration and Traffic Control



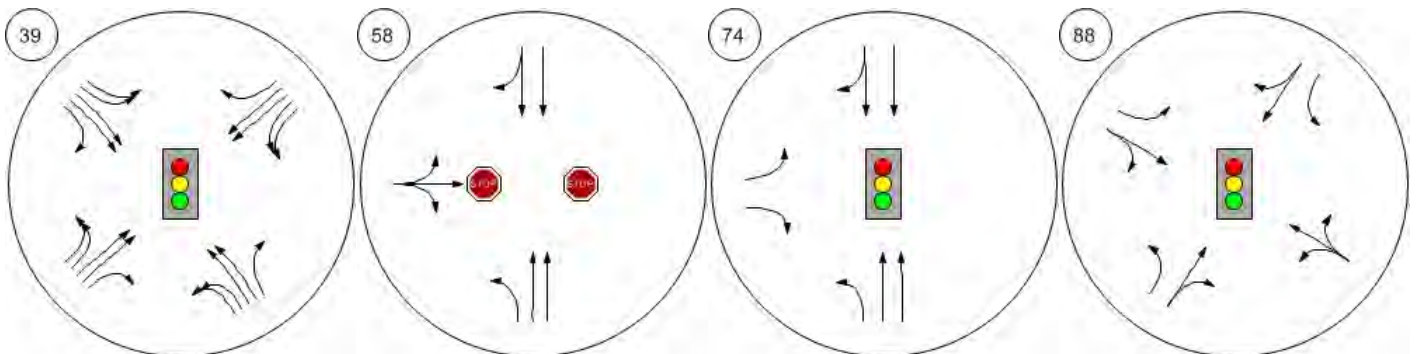
Lane Configuration and Traffic Control



El Camino Real (SR 82)/Robl El Camino Real (SR 82)/Middle El Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



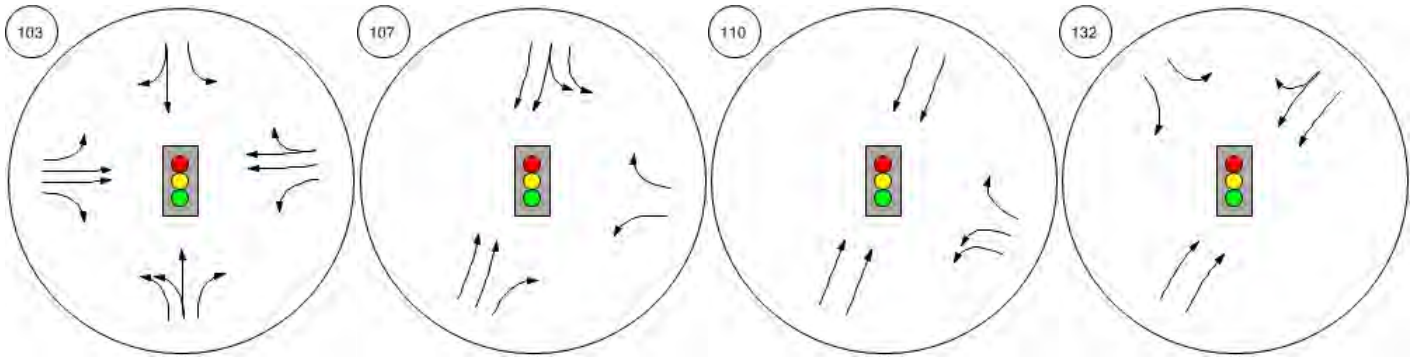
Sand Hill Rd/Santa Cruz Ave University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr



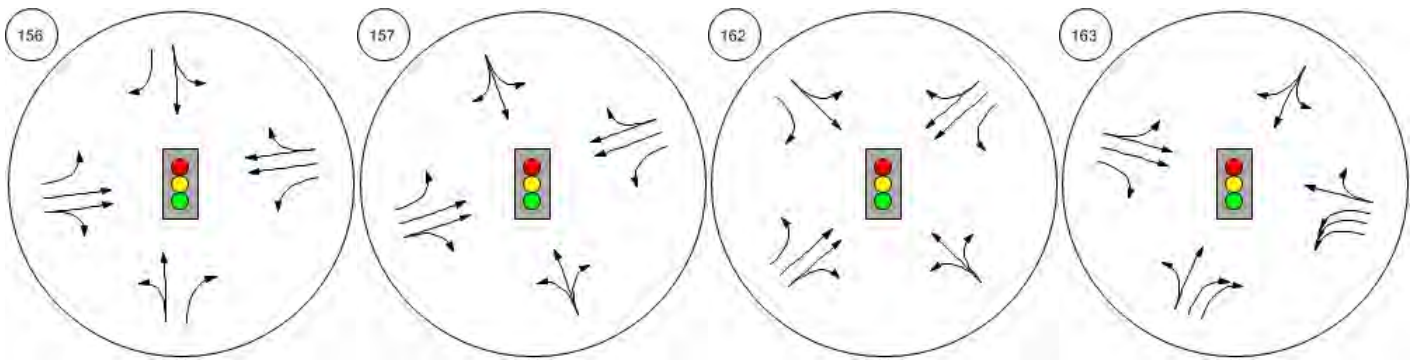
Lane Configuration and Traffic Control



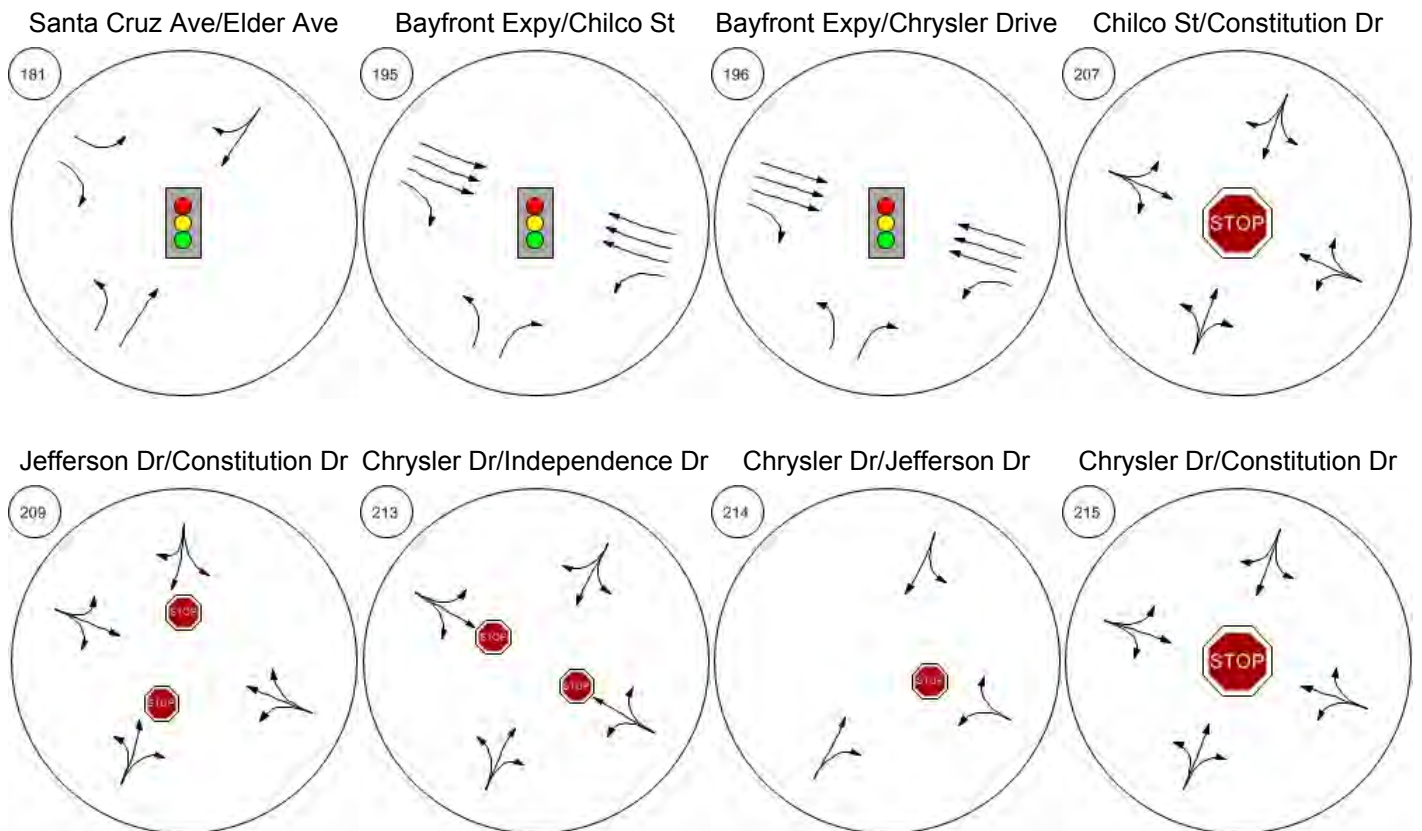
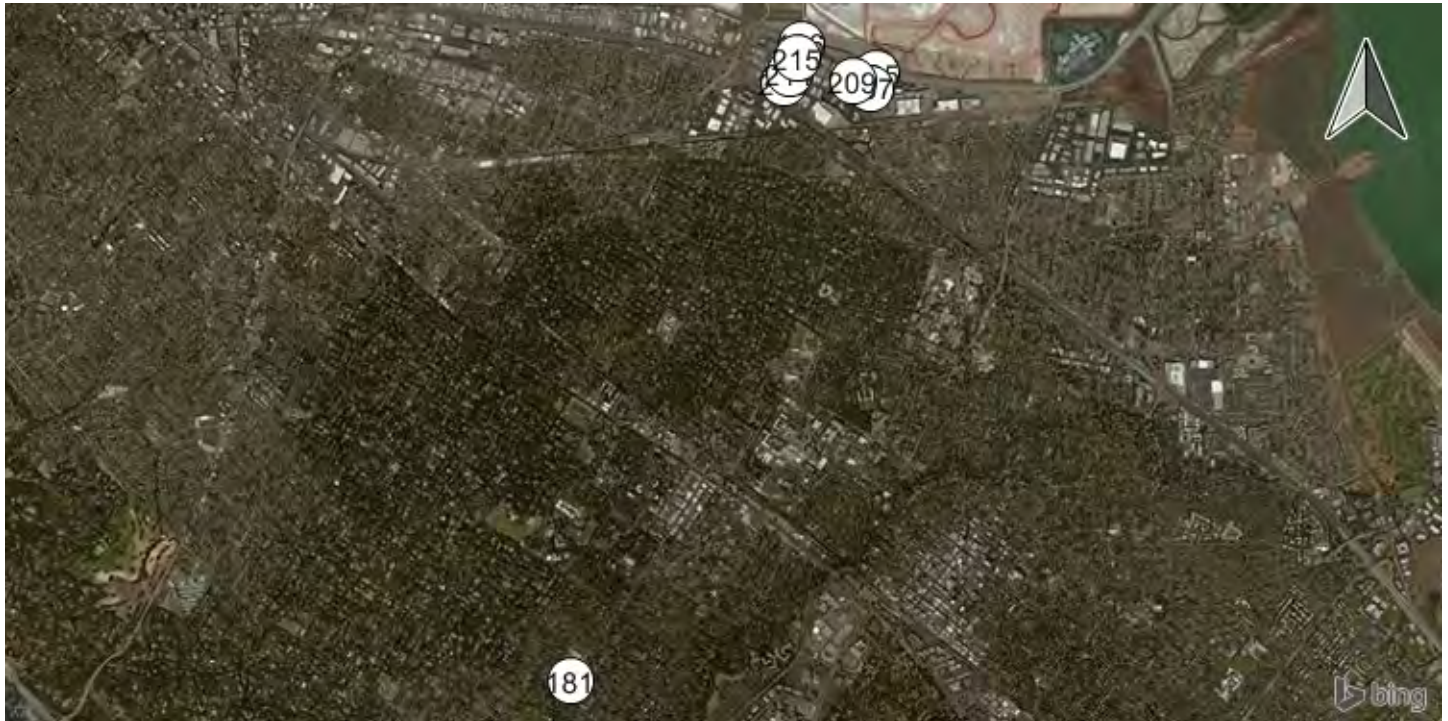
Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road/101 NB Ramps Oak Ave/Sand Hill Rd



Saga Ln/Sand Hill Rd Branner Dr/Sand Hill Rd Sharon Park Dr/ Sand Hill Rd Bayfront Expy/Marsh Rd



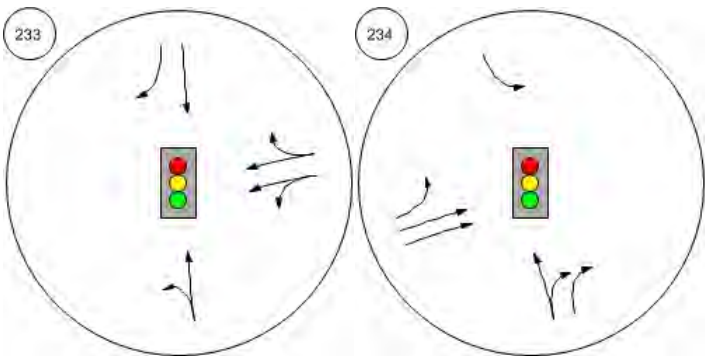
Lane Configuration and Traffic Control



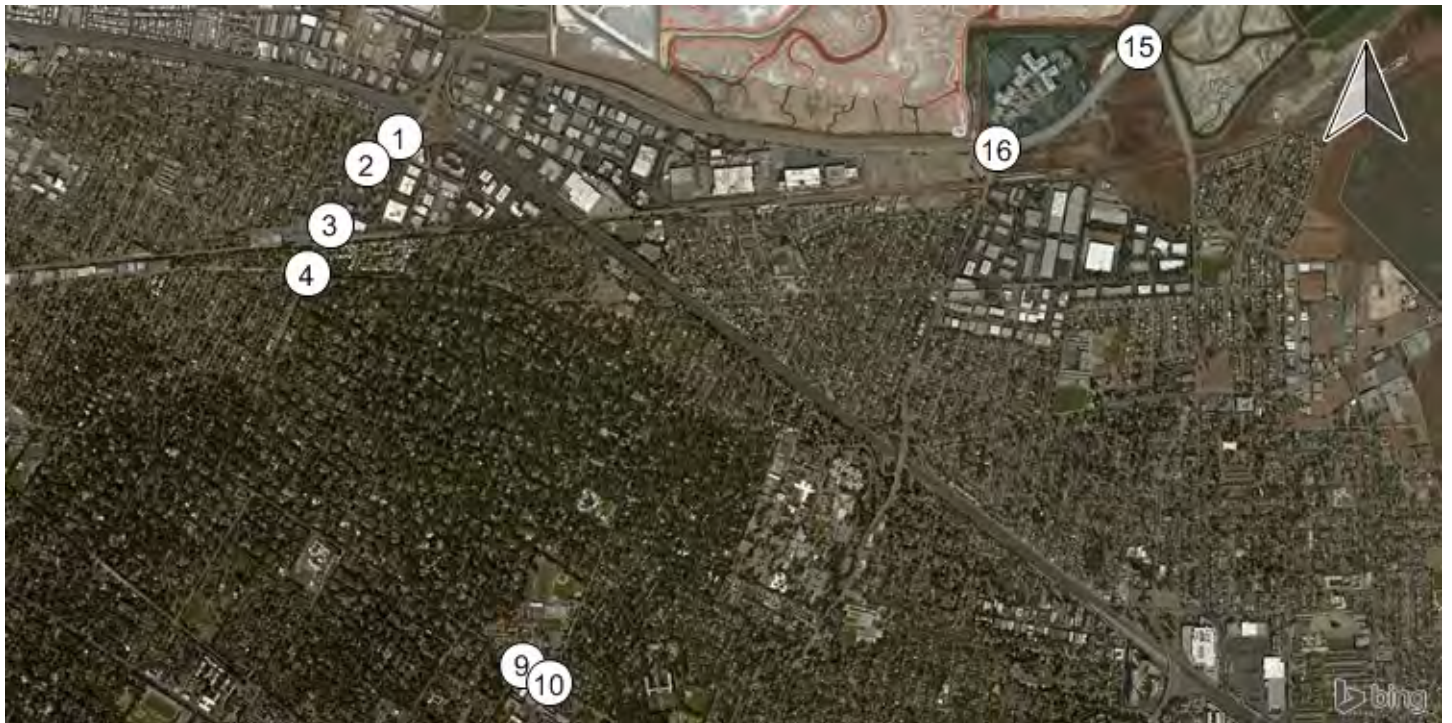
Lane Configuration and Traffic Control



Sand Hill Circle/Sand Hill Ro Sand Hill Rd/Hwy 280 NB Off

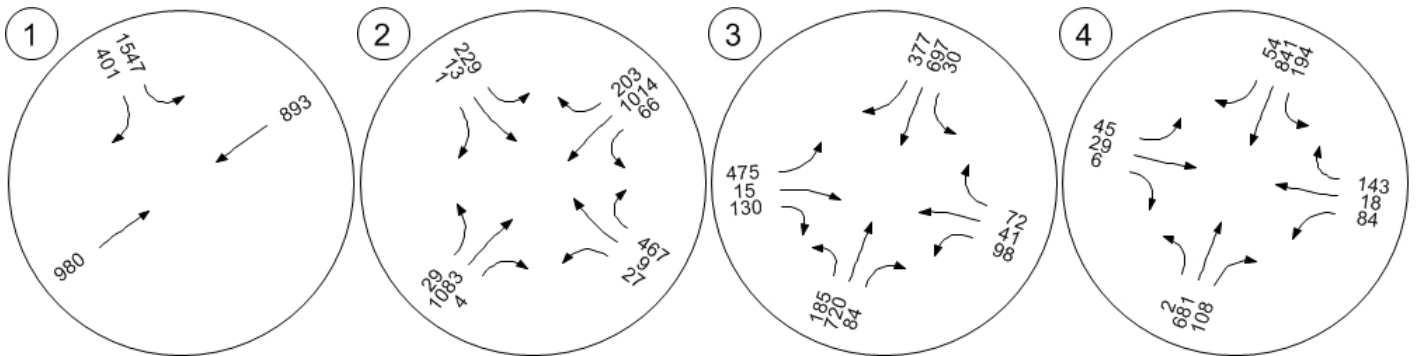


Traffic Volume - Base Volume

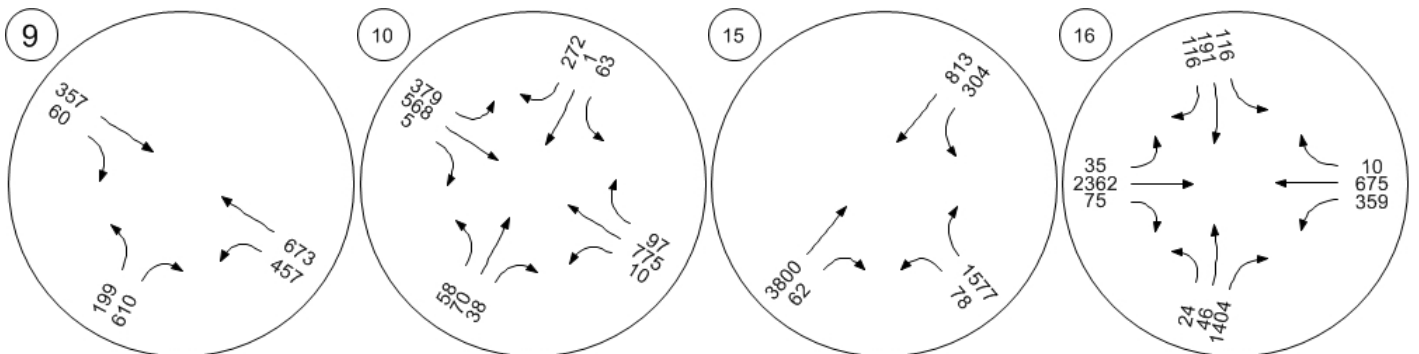


Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd



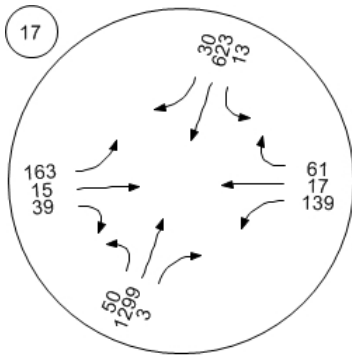
Middlefield Rd/Ravenswood Middlefield Rd/Ringwood Ave Bayfront Expy (SR 84)/Univer Bayfront Expy (SR 84)/Willow



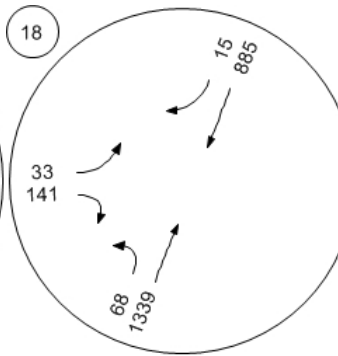
Traffic Volume - Base Volume



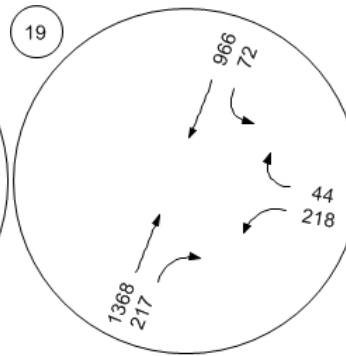
Willow Rd (SR 114)/Hamilton



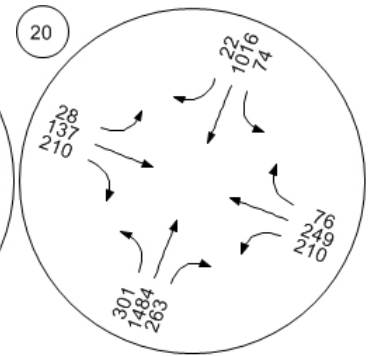
Willow Rd (SR 114)/Ivy Dr



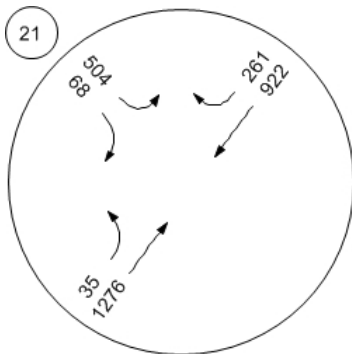
Willow Rd (SR 114)/O'Brien



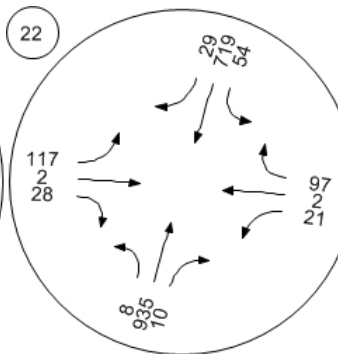
Willow Rd (SR 114)/Newbrid



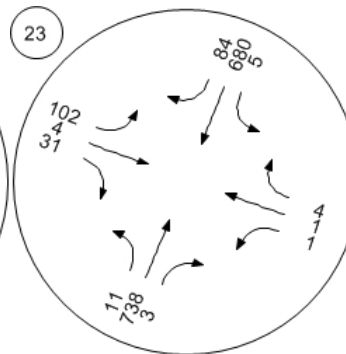
Willow Rd/Bay Rd



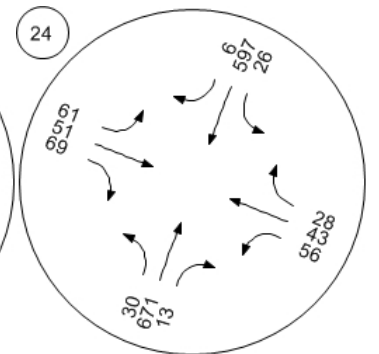
Willow Rd/Durham St-VA Me



Willow Rd/Coleman Ave



Willow Rd/Gilbert Ave



Traffic Volume - Base Volume

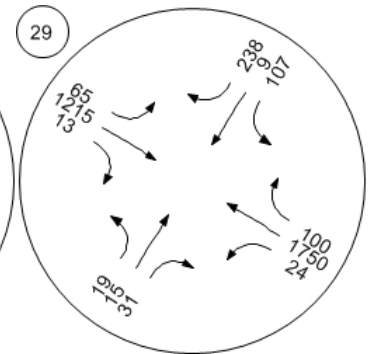
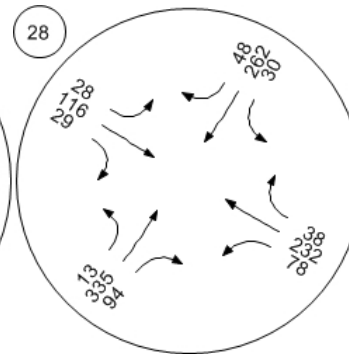
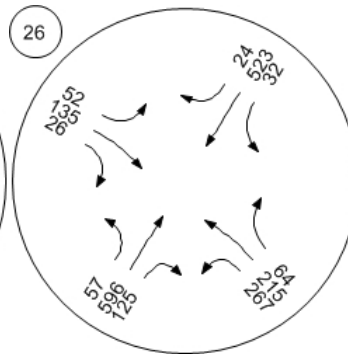
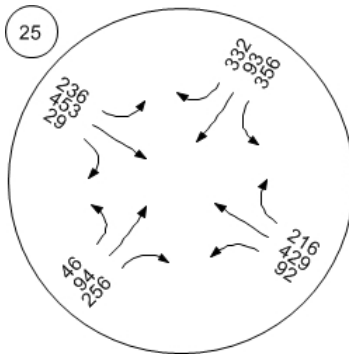


Middlefield Rd-Willow Rd

Ravenswood Ave/Laurel St

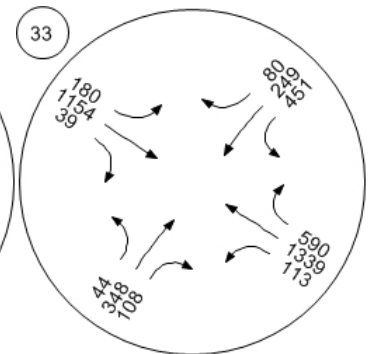
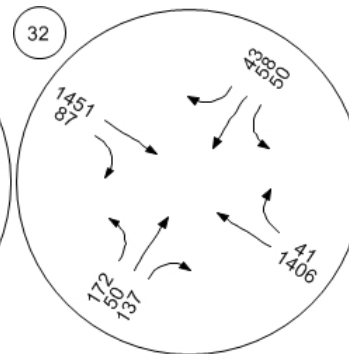
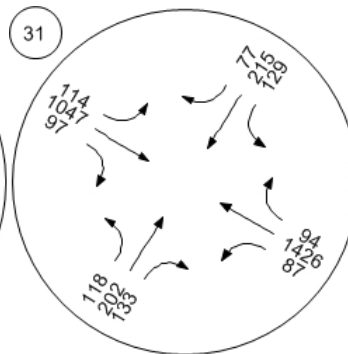
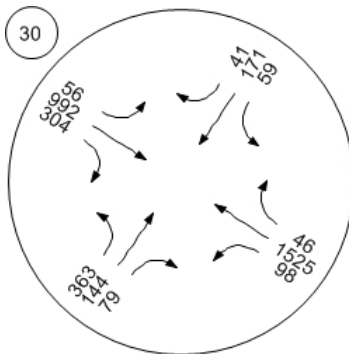
Oak Grove Ave/Laurel St

El Camino Real (SR 82)/Enci



El Camino Real (SR 82)/Glen

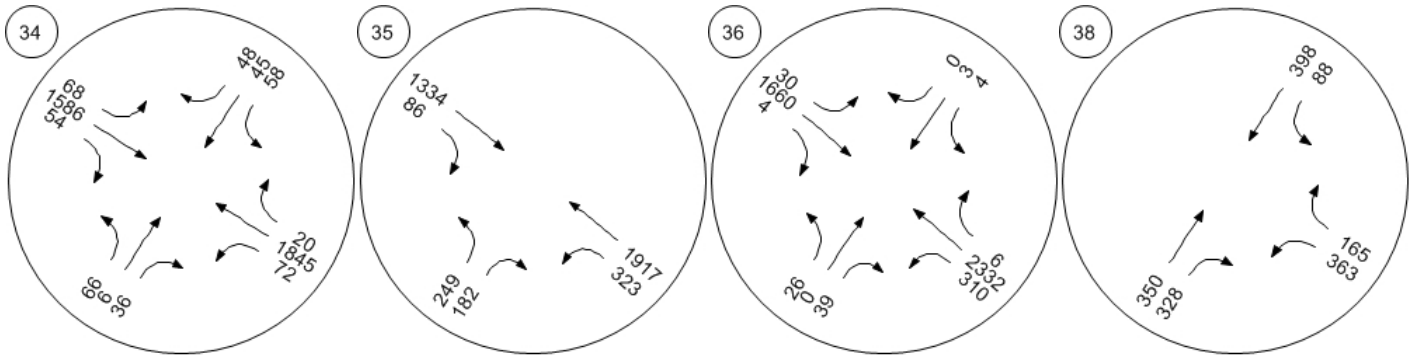
El Camino Real (SR 82)/Oak El Camino Real (SR 82)/Sant El Camino Real (SR 82)/Rav



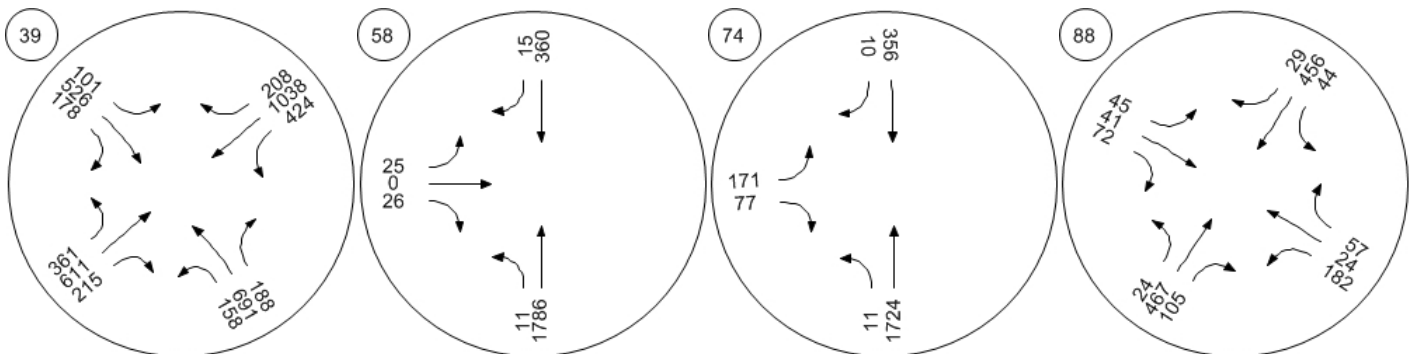
Traffic Volume - Base Volume



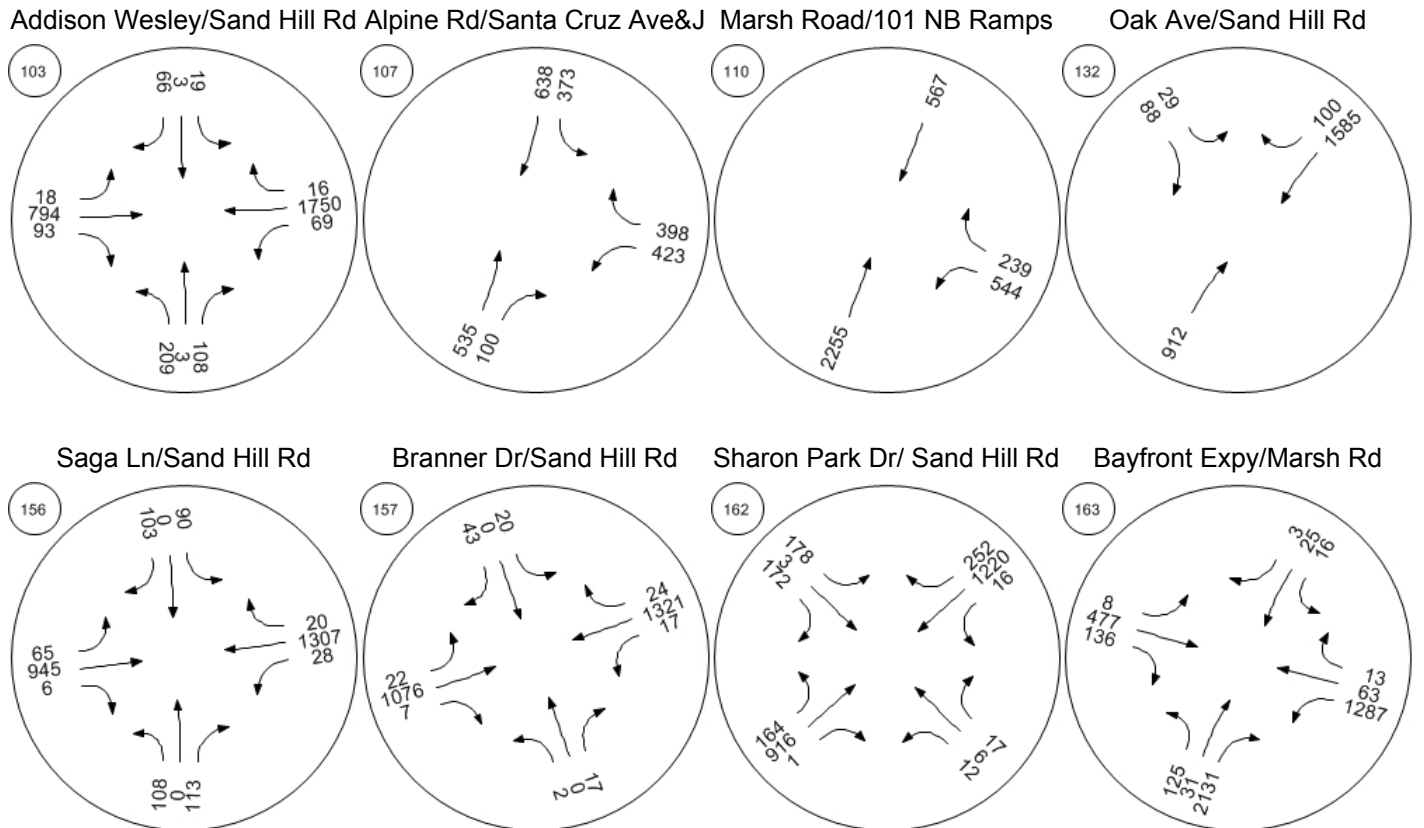
El Camino Real (SR 82)/Rob El Camino Real (SR 82)/Middle El Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



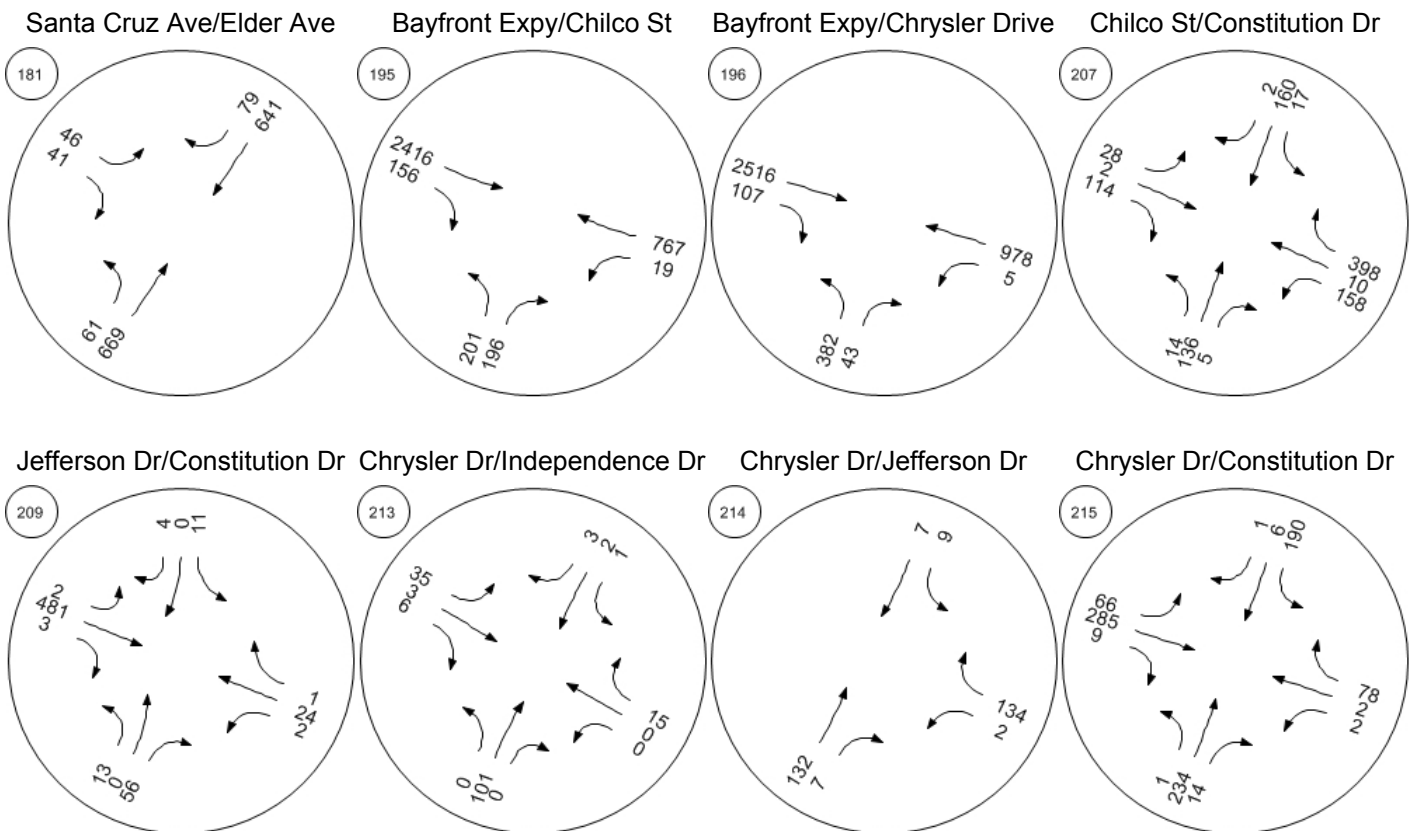
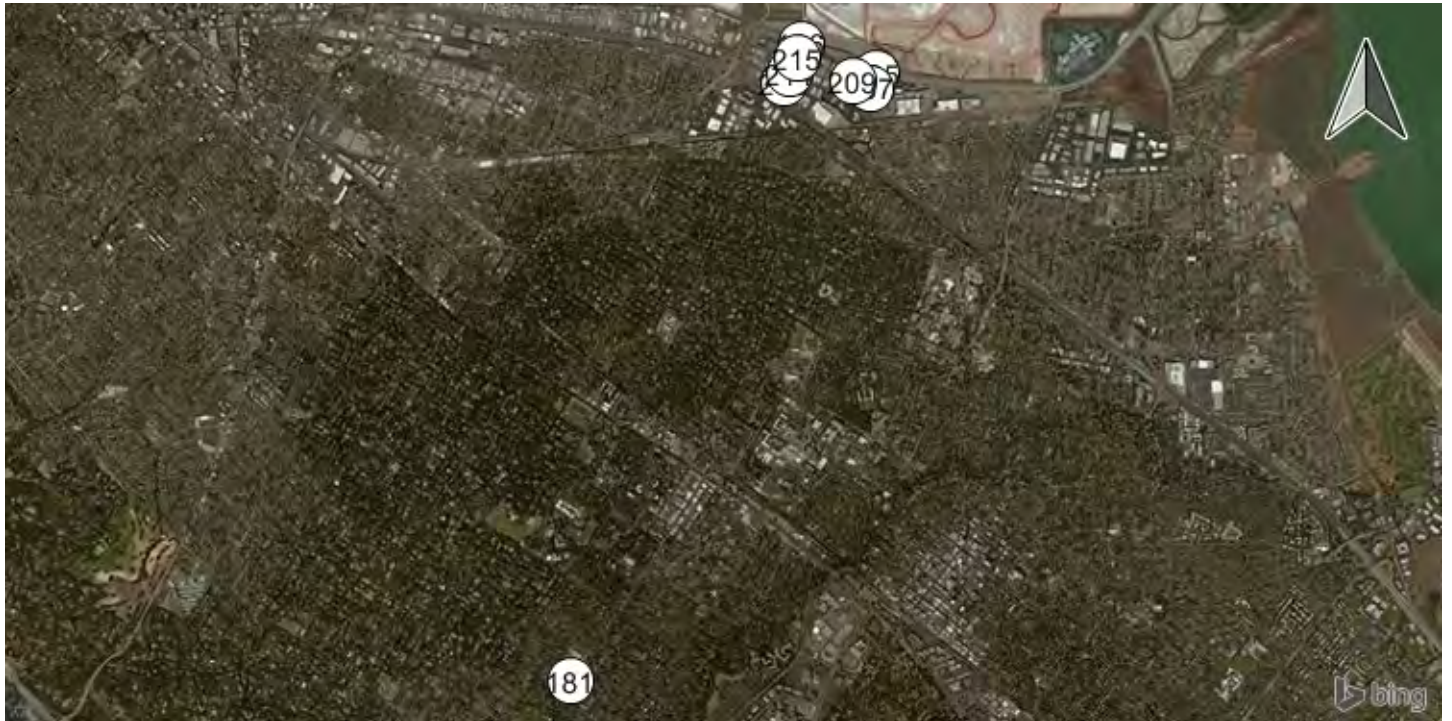
Sand Hill Rd/Santa Cruz Ave University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr



Traffic Volume - Base Volume



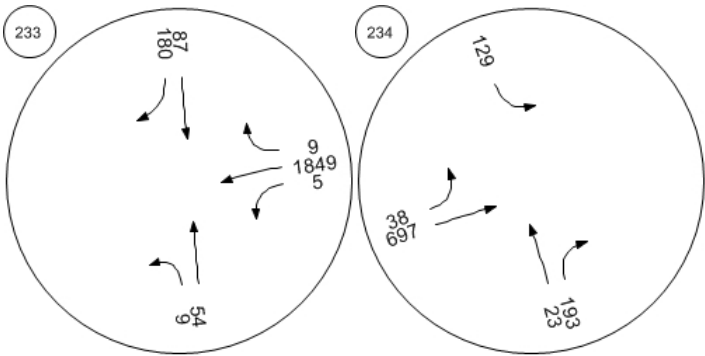
Traffic Volume - Base Volume



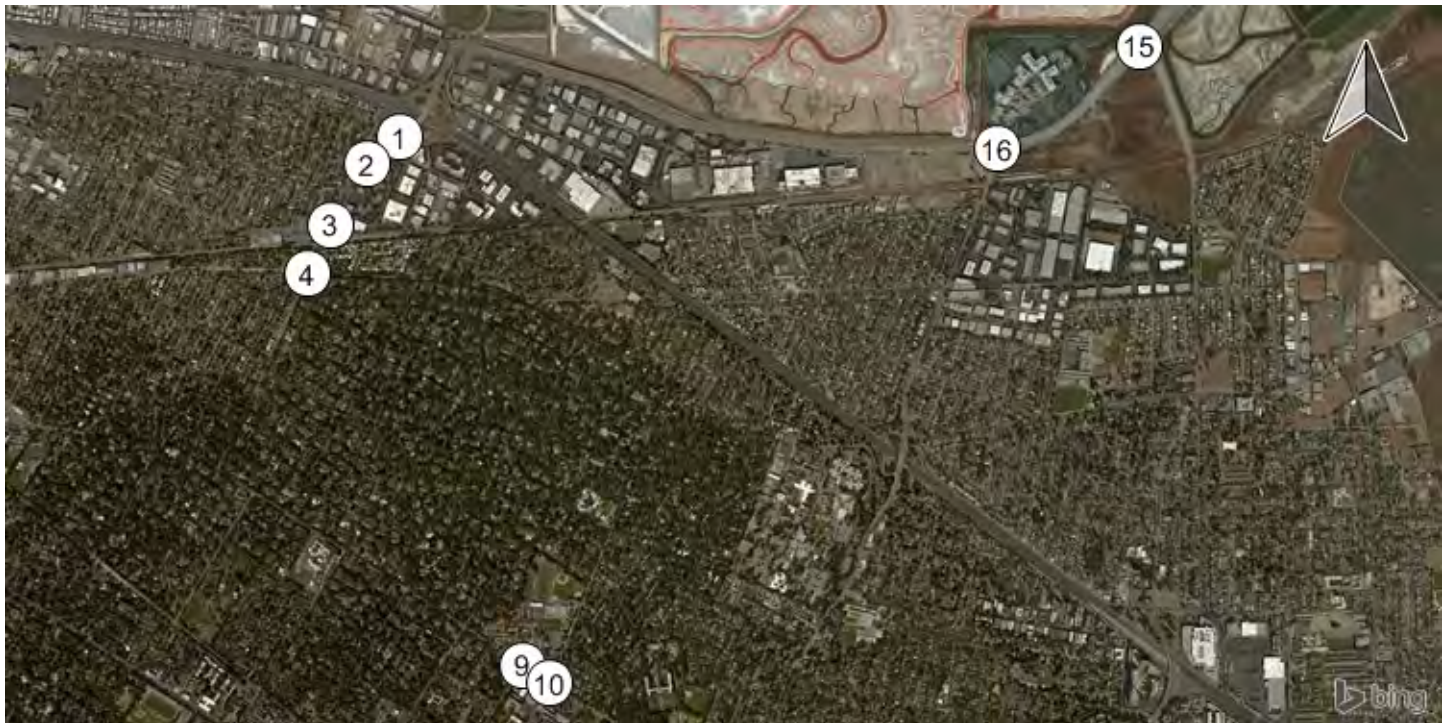
Traffic Volume - Base Volume



Sand Hill Circle/Sand Hill Ro Sand Hill Rd/Hwy 280 NB Off

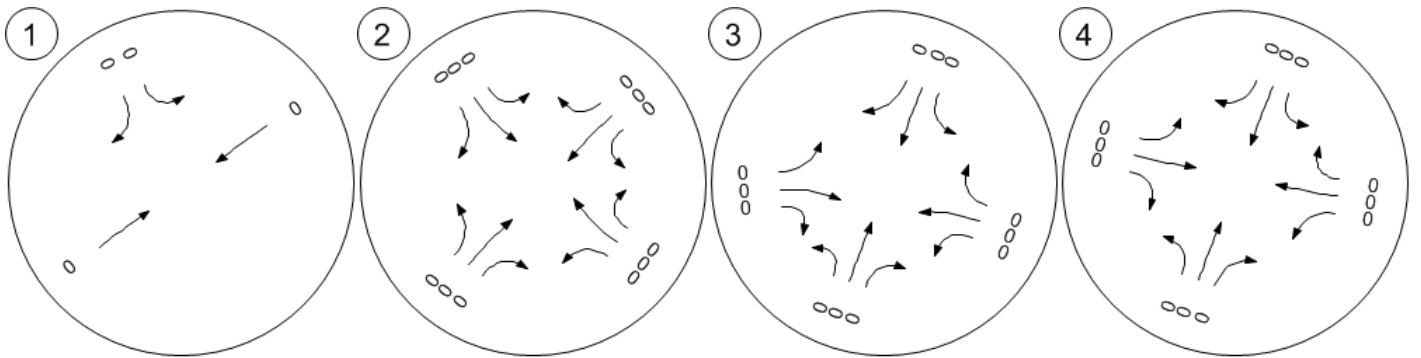


Traffic Volume - In-Process Volume



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

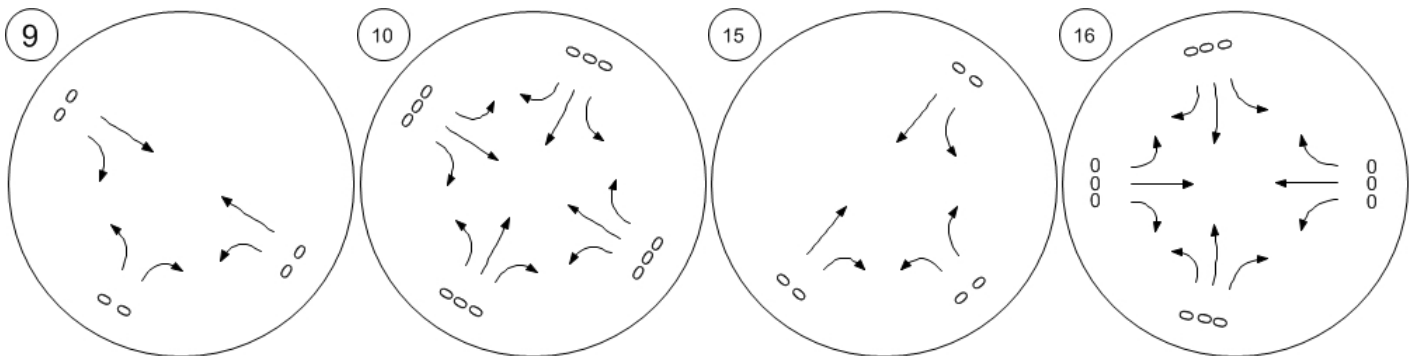


Middlefield Rd/Ravenswood

Middlefield Rd/Ringwood Ave

Bayfront Expy (SR 84)/Univer

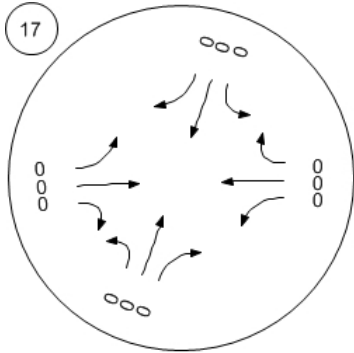
Bayfront Expy (SR 84)/Willow



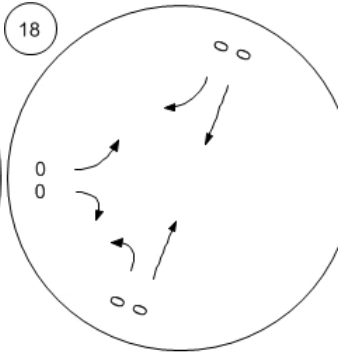
Traffic Volume - In-Process Volume



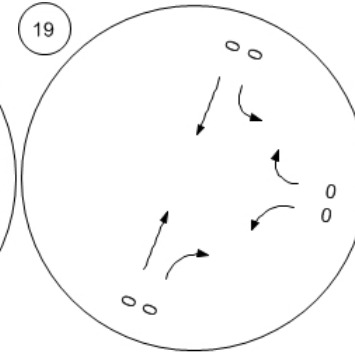
Willow Rd (SR 114)/Hamilton



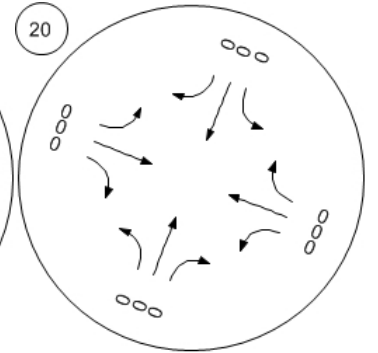
Willow Rd (SR 114)/Ivy Dr



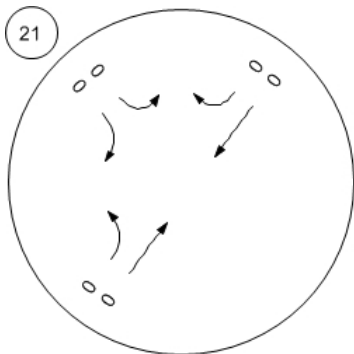
Willow Rd (SR 114)/O'Brien



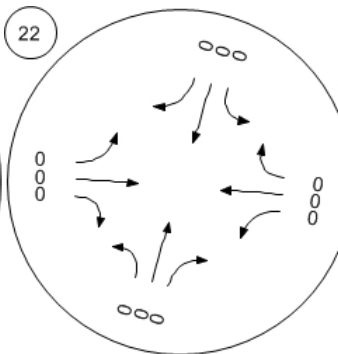
Willow Rd (SR 114)/Newbrid



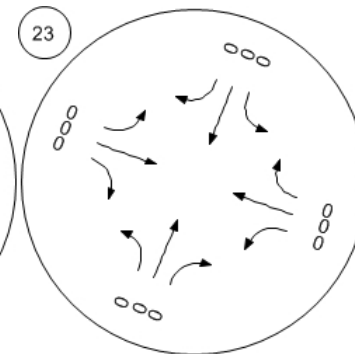
Willow Rd/Bay Rd



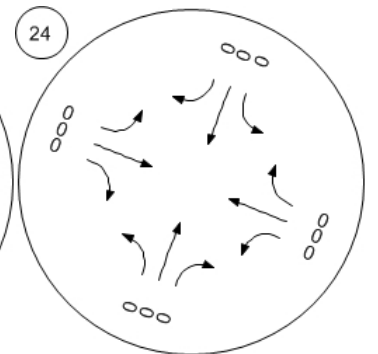
Willow Rd/Durham St-VA Me



Willow Rd/Coleman Ave



Willow Rd/Gilbert Ave



Traffic Volume - In-Process Volume

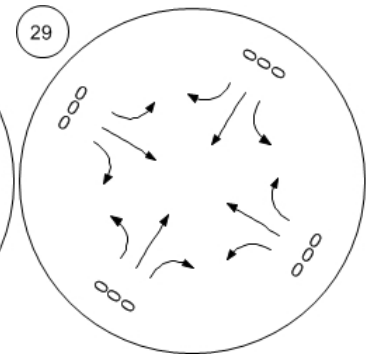
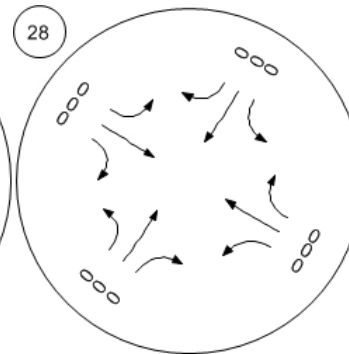
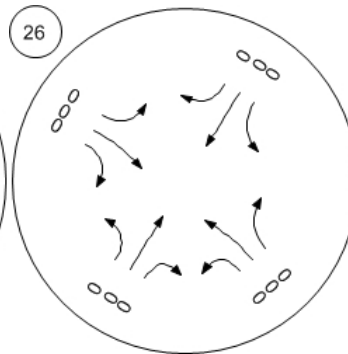
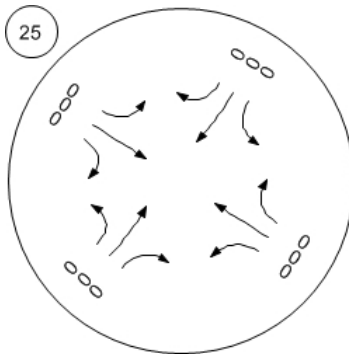


Middlefield Rd-Willow Rd

Ravenswood Ave/Laurel St

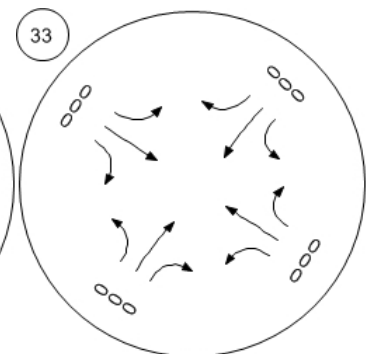
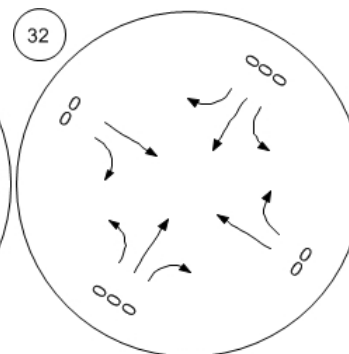
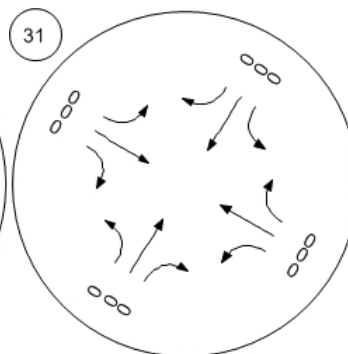
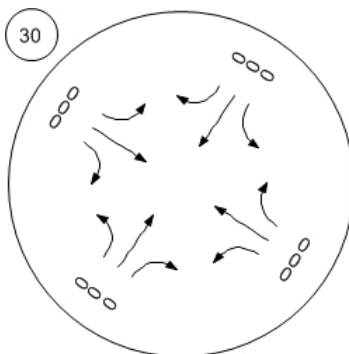
Oak Grove Ave/Laurel St

El Camino Real (SR 82)/Enci

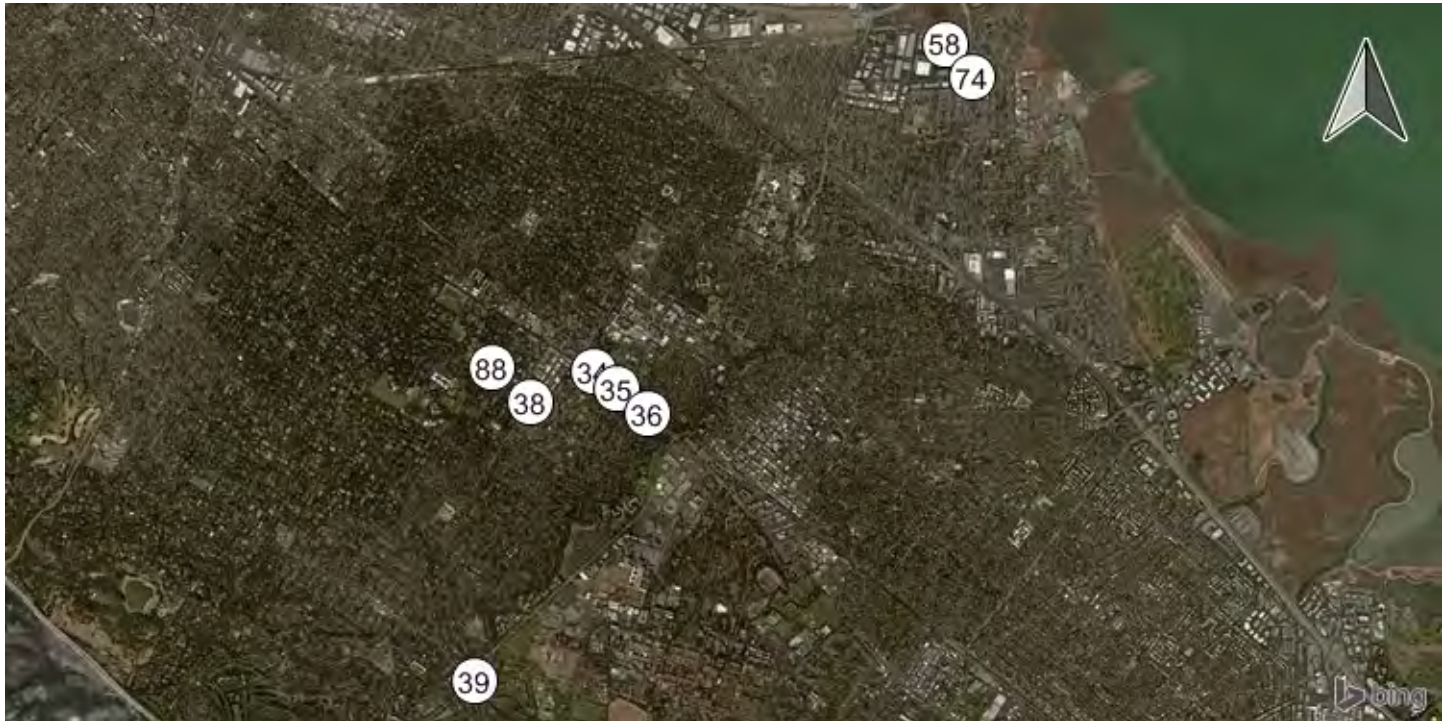


El Camino Real (SR 82)/Glen

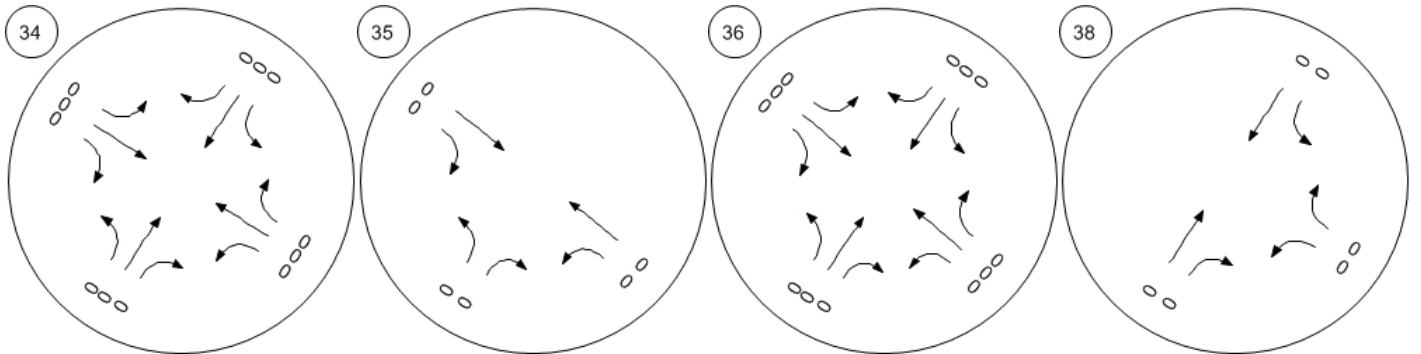
El Camino Real (SR 82)/Oak El Camino Real (SR 82)/Sant El Camino Real (SR 82)/Rav



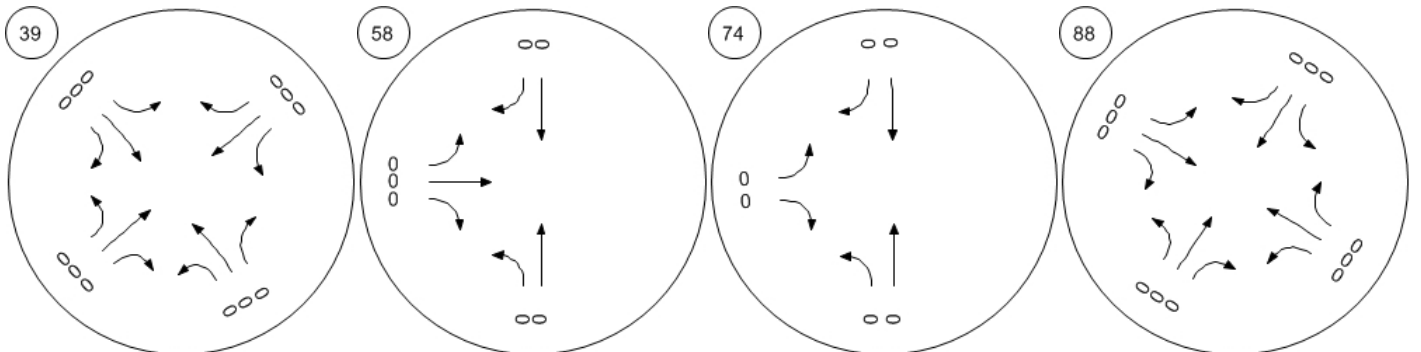
Traffic Volume - In-Process Volume



El Camino Real (SR 82)/Robl El Camino Real (SR 82)/Middle El Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



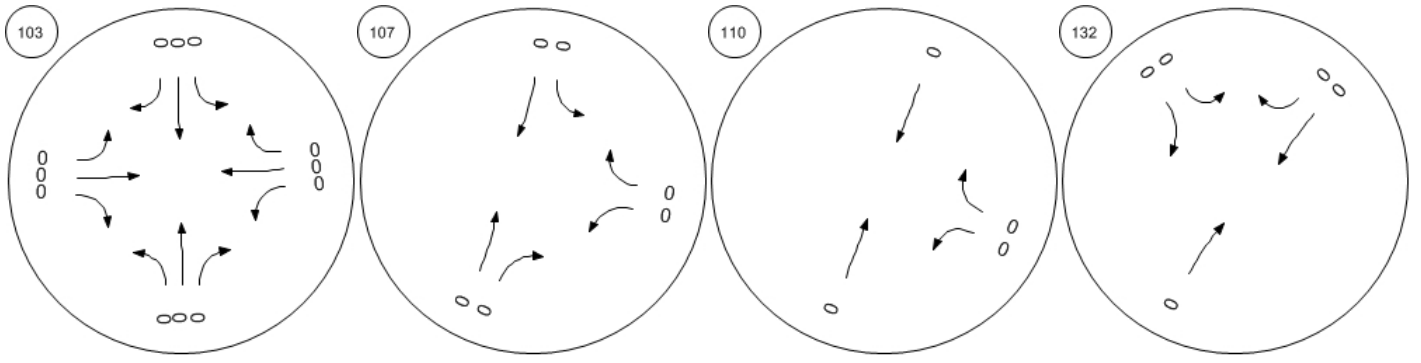
Sand Hill Rd/Santa Cruz Ave University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr



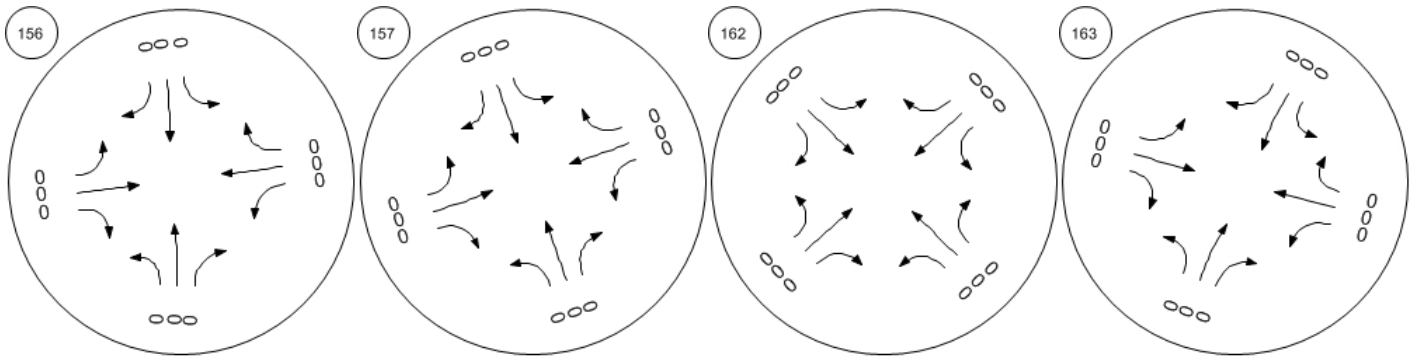
Traffic Volume - In-Process Volume



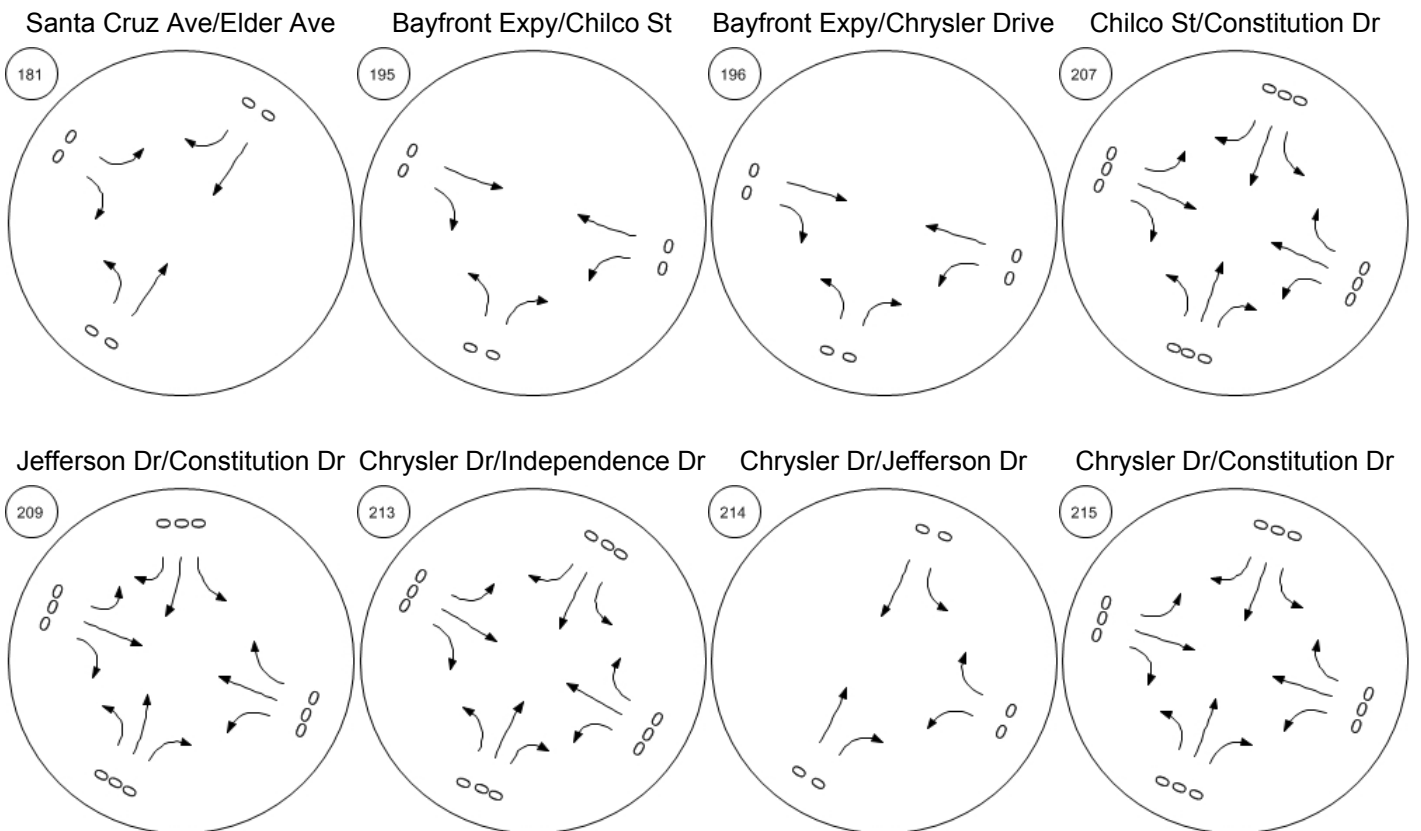
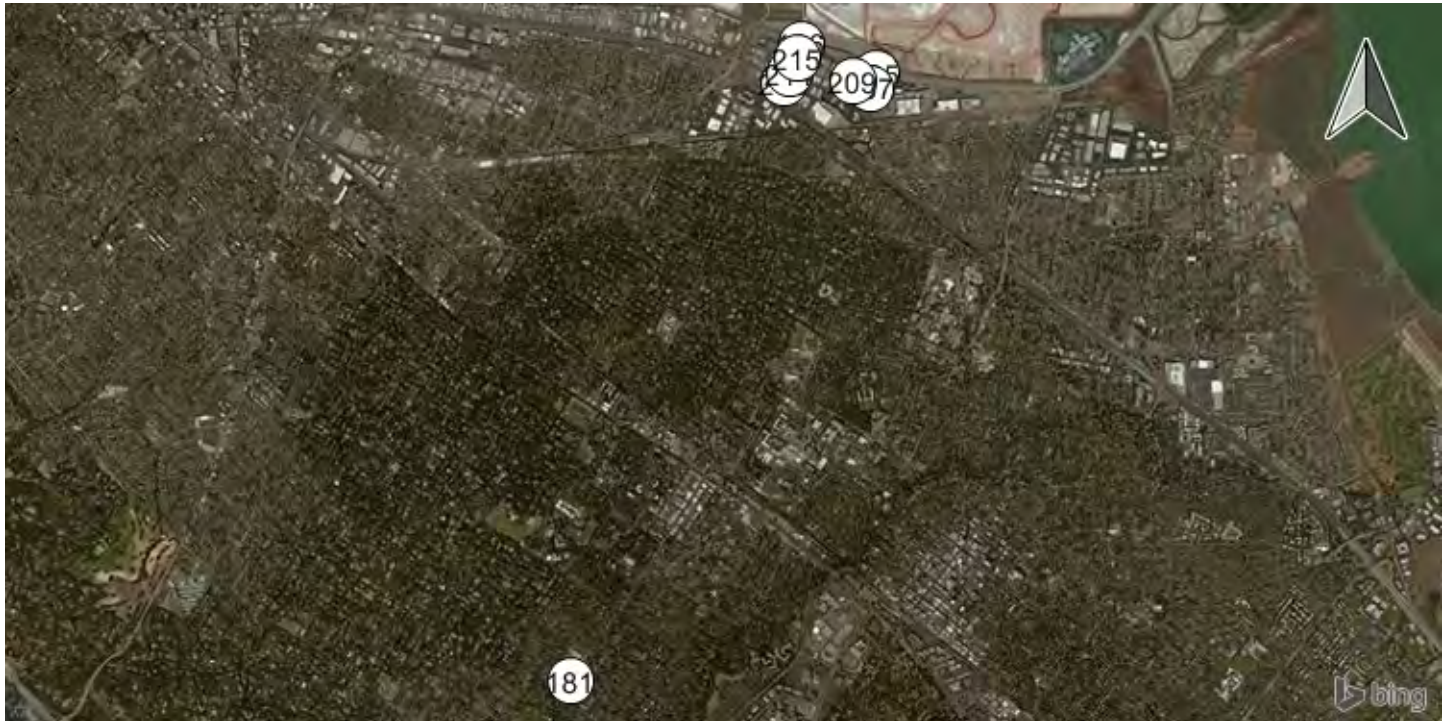
Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road/101 NB Ramps Oak Ave/Sand Hill Rd



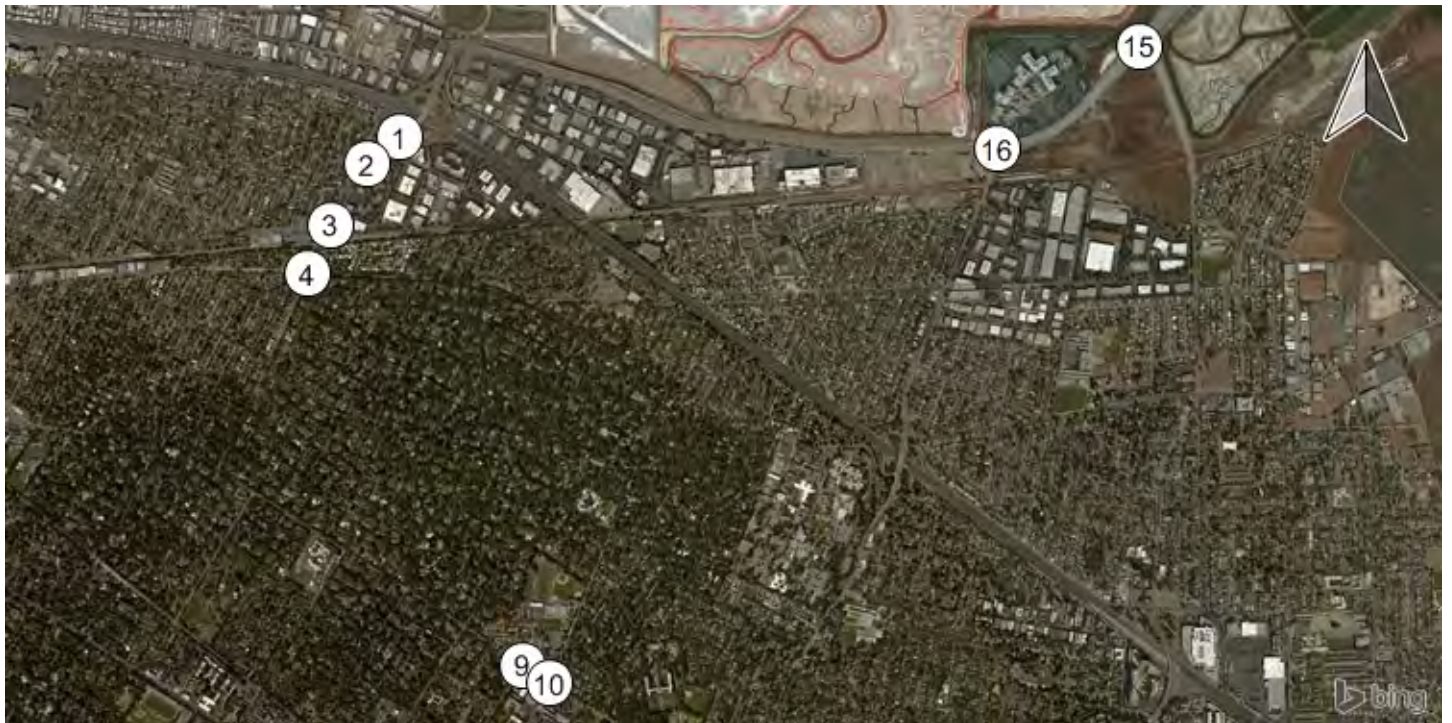
Saga Ln/Sand Hill Rd Branner Dr/Sand Hill Rd Sharon Park Dr/ Sand Hill Rd Bayfront Expy/Marsh Rd



Traffic Volume - In-Process Volume

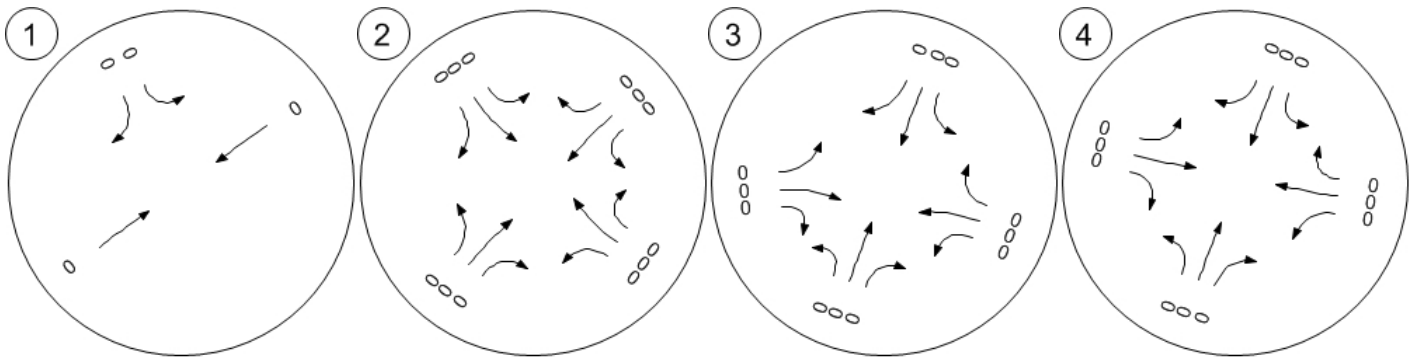


Traffic Volume - Net New Site Trips



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

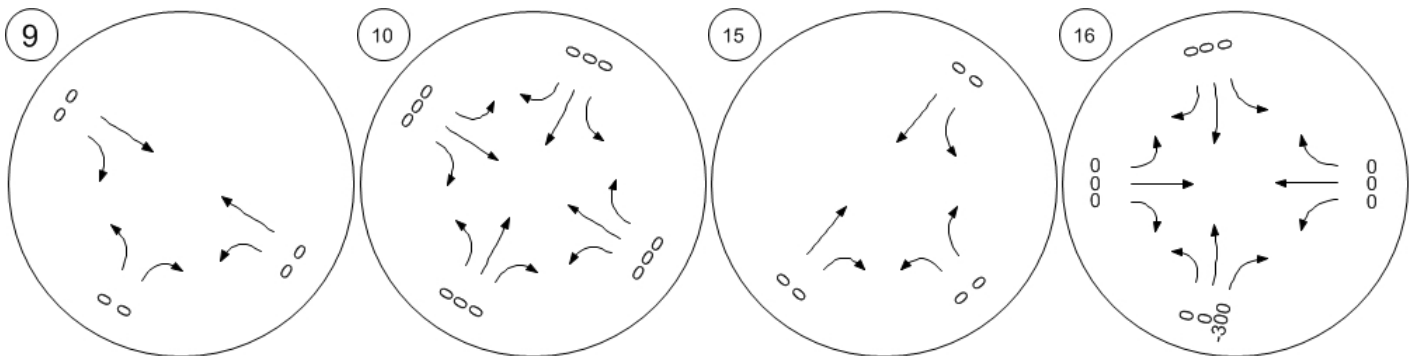


Middlefield Rd/Ravenswood

Middlefield Rd/Ringwood Ave

Bayfront Expy (SR 84)/Univer

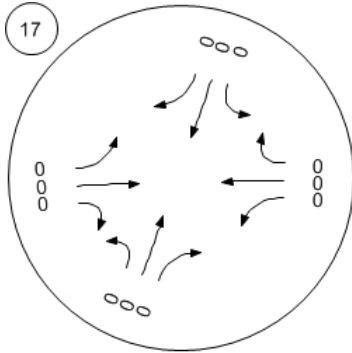
Bayfront Expy (SR 84)/Willow



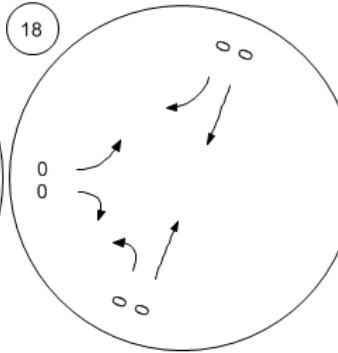
Traffic Volume - Net New Site Trips



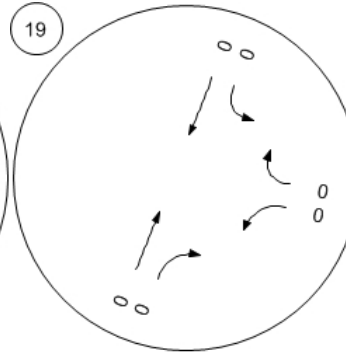
Willow Rd (SR 114)/Hamilton



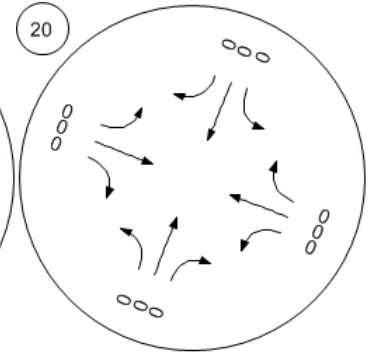
Willow Rd (SR 114)/Ivy Dr



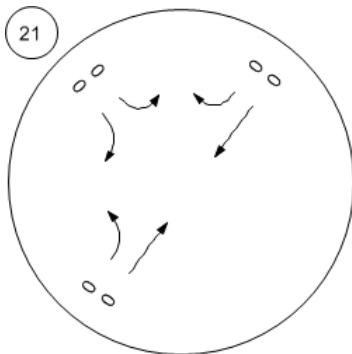
Willow Rd (SR 114)/O'Brien



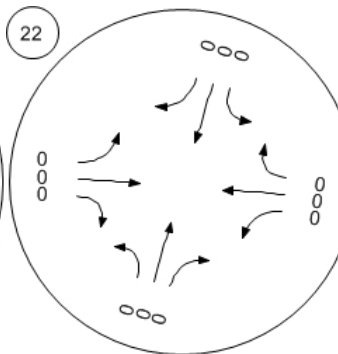
Willow Rd (SR 114)/Newbrid



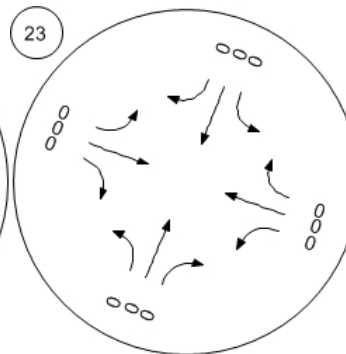
Willow Rd/Bay Rd



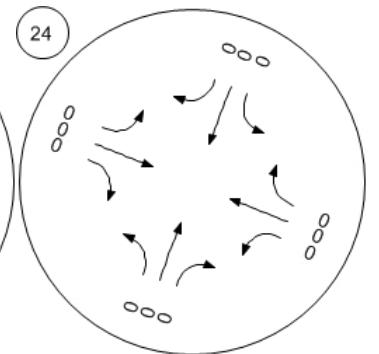
Willow Rd/Durham St-VA Me



Willow Rd/Coleman Ave



Willow Rd/Gilbert Ave



Traffic Volume - Net New Site Trips

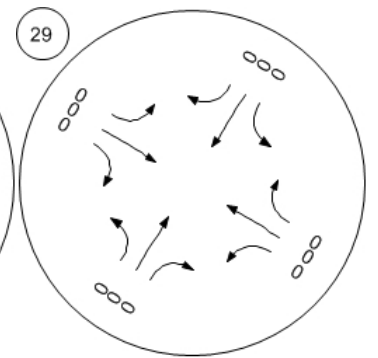
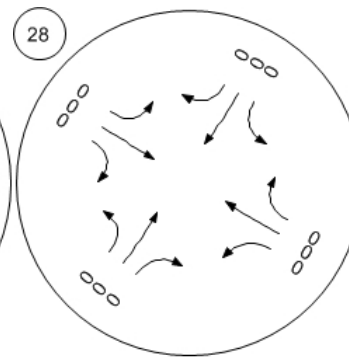
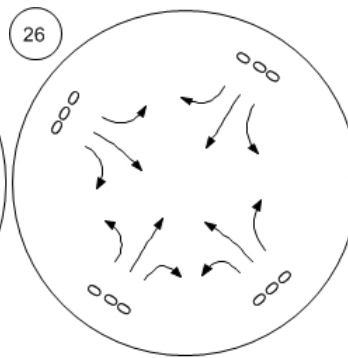
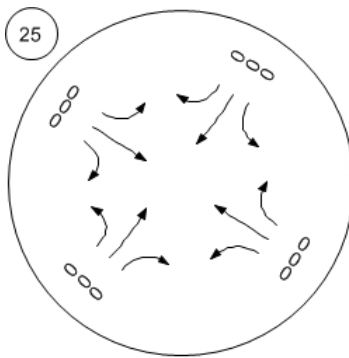


Middlefield Rd-Willow Rd

Ravenswood Ave/Laurel St

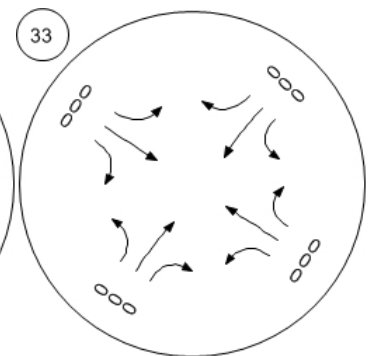
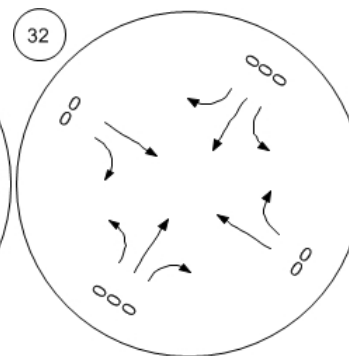
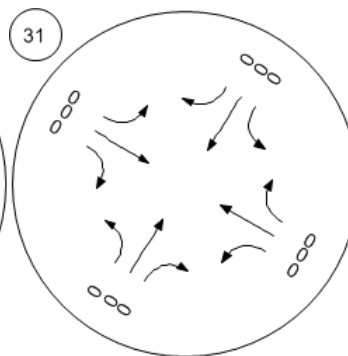
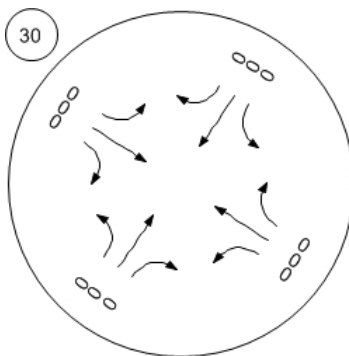
Oak Grove Ave/Laurel St

El Camino Real (SR 82)/Enci



El Camino Real (SR 82)/Glen

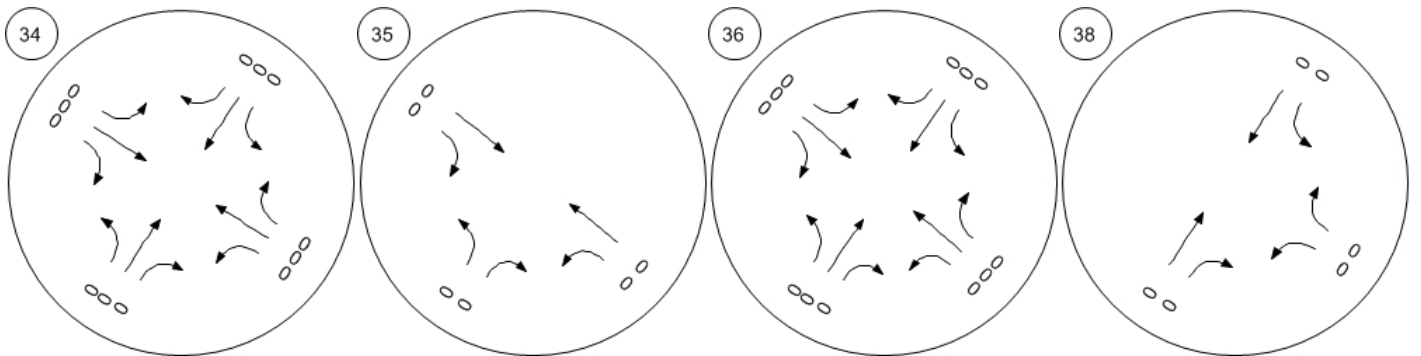
El Camino Real (SR 82)/Oak El Camino Real (SR 82)/Sant El Camino Real (SR 82)/Rav



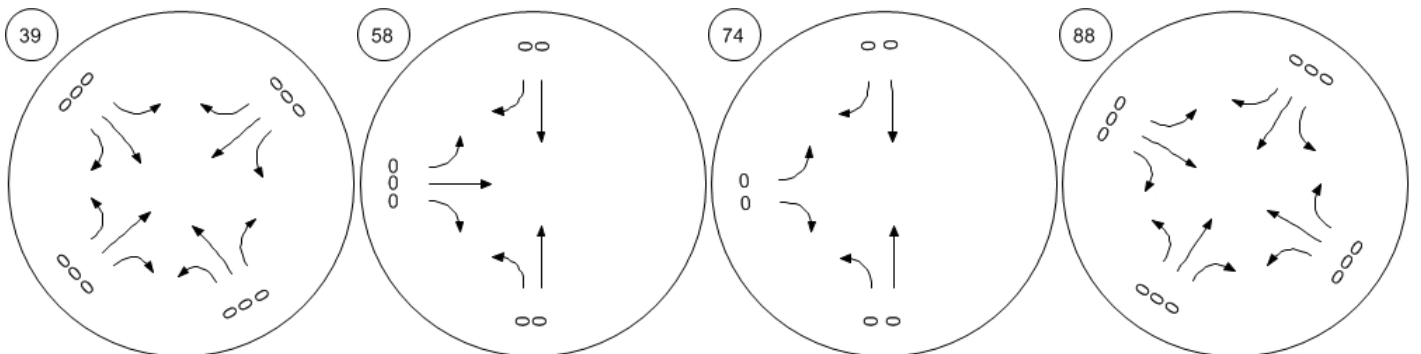
Traffic Volume - Net New Site Trips



El Camino Real (SR 82)/Robl El Camino Real (SR 82)/Middle El Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



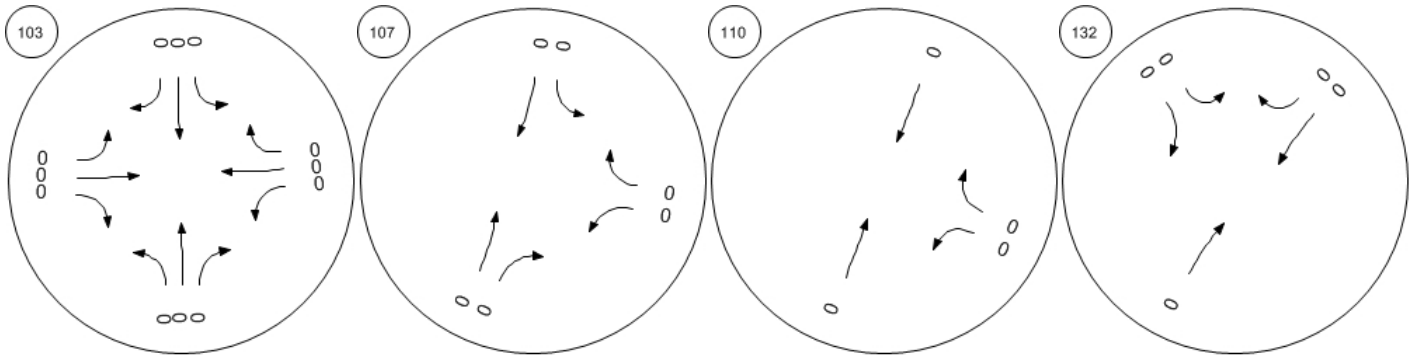
Sand Hill Rd/Santa Cruz Ave University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr



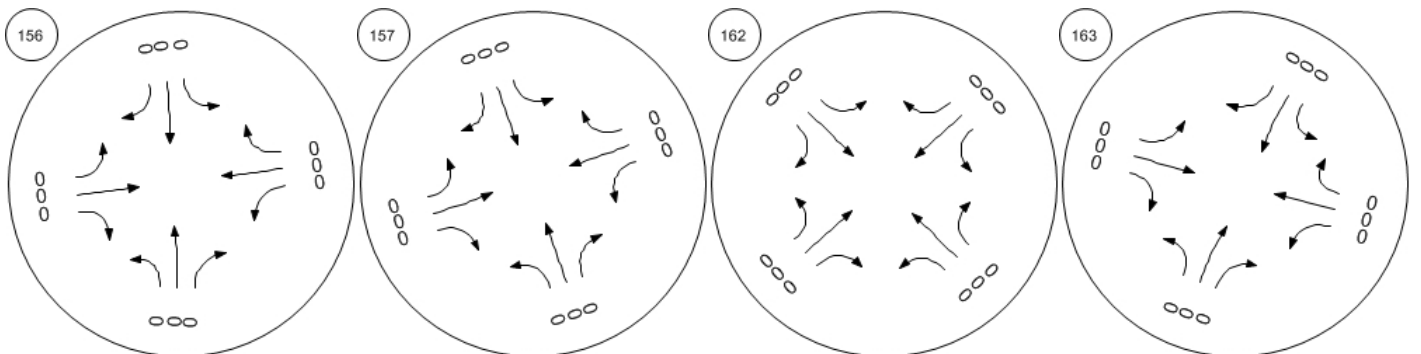
Traffic Volume - Net New Site Trips



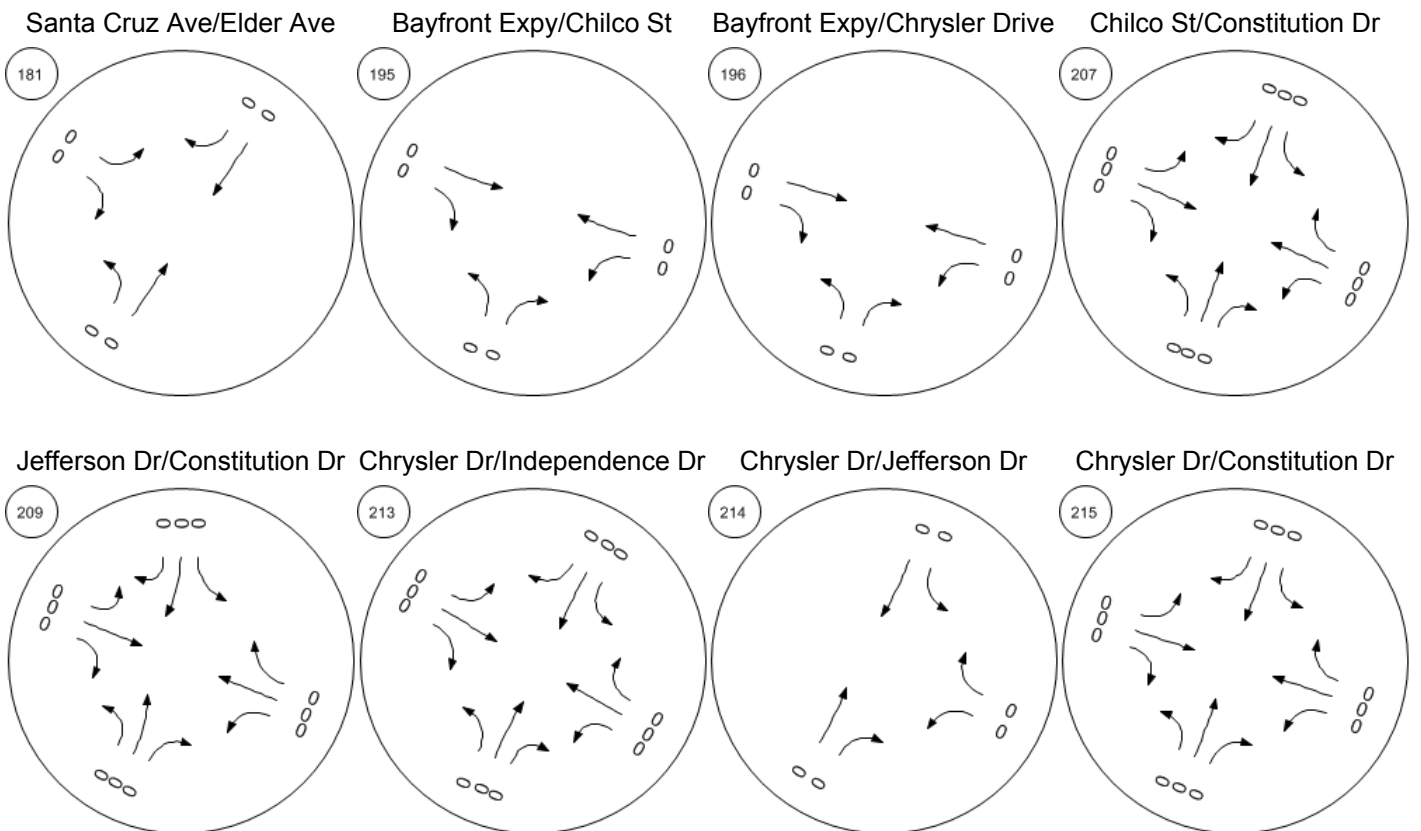
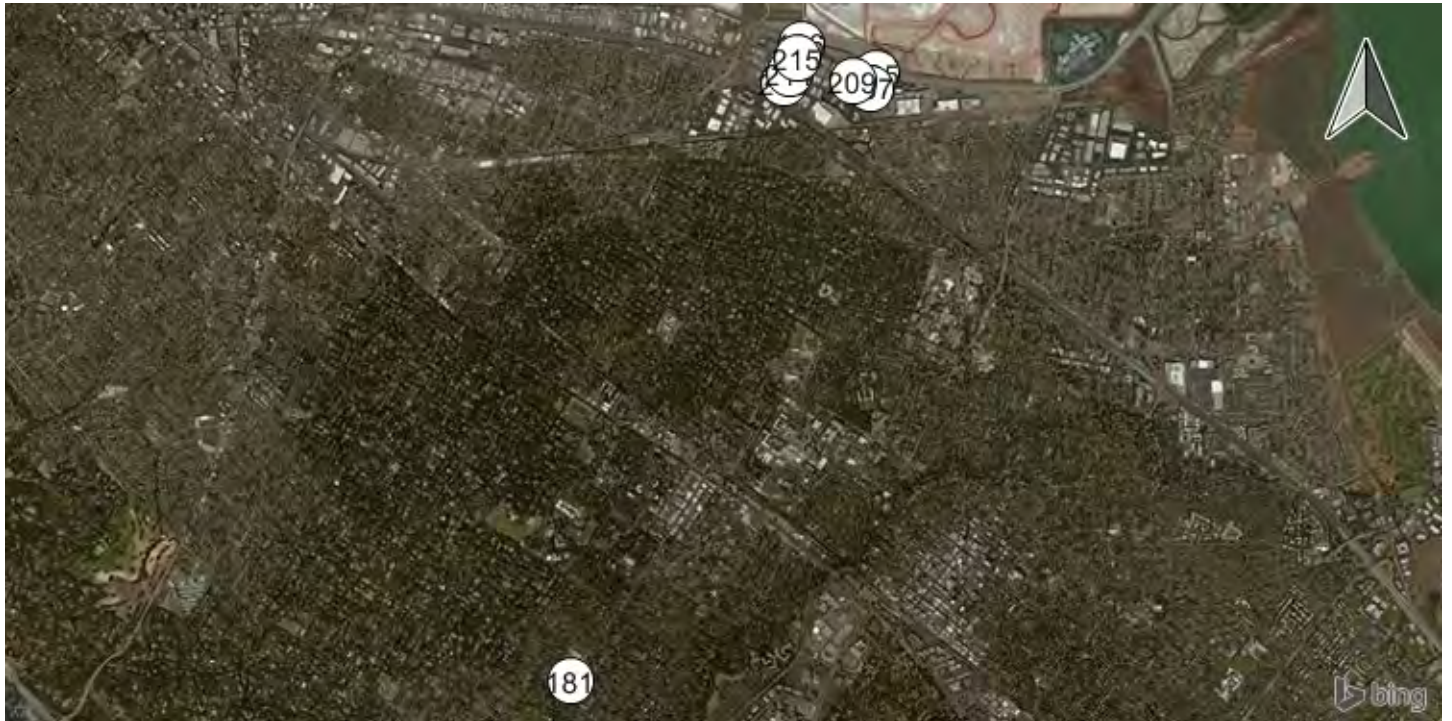
Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road/101 NB Ramps Oak Ave/Sand Hill Rd



Saga Ln/Sand Hill Rd Branner Dr/Sand Hill Rd Sharon Park Dr/ Sand Hill Rd Bayfront Expy/Marsh Rd



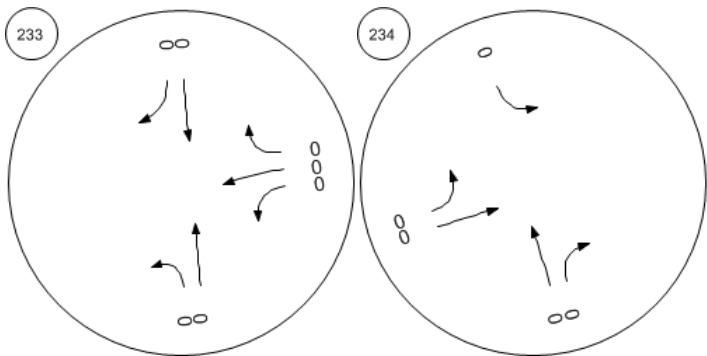
Traffic Volume - Net New Site Trips



Traffic Volume - Net New Site Trips



Sand Hill Circle/Sand Hill Ro Sand Hill Rd/Hwy 280 NB Off

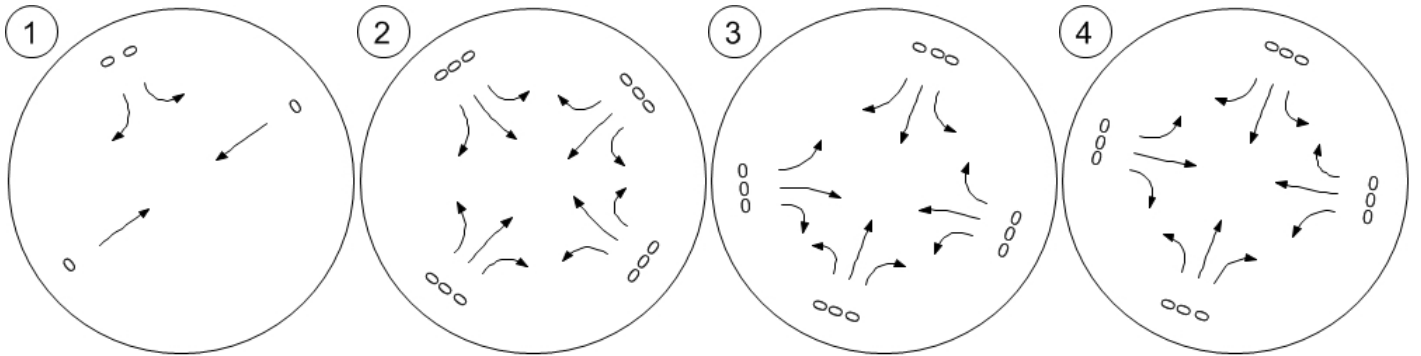


Traffic Volume - Other Volume



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

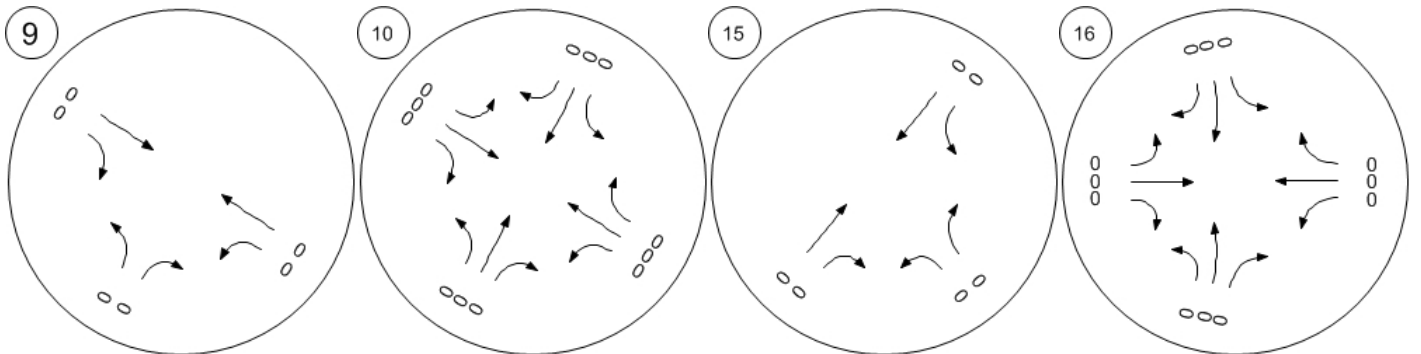


Middlefield Rd/Ravenswood

Middlefield Rd/Ringwood Ave

Bayfront Expy (SR 84)/Univer

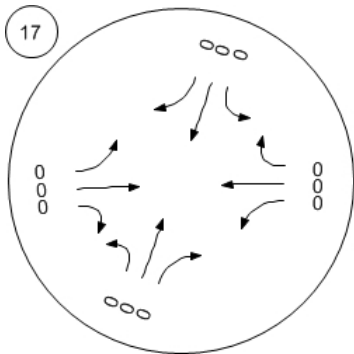
Bayfront Expy (SR 84)/Willow



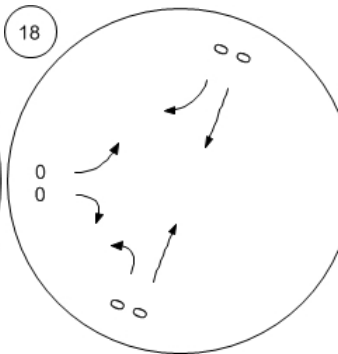
Traffic Volume - Other Volume



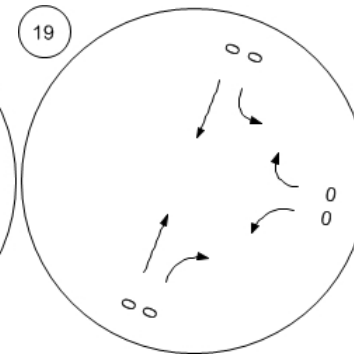
Willow Rd (SR 114)/Hamilton



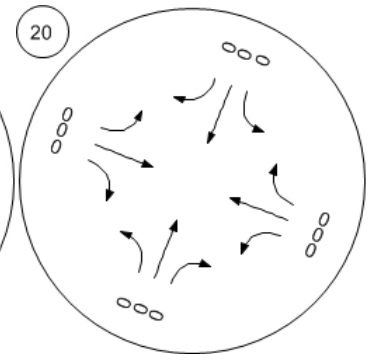
Willow Rd (SR 114)/Ivy Dr



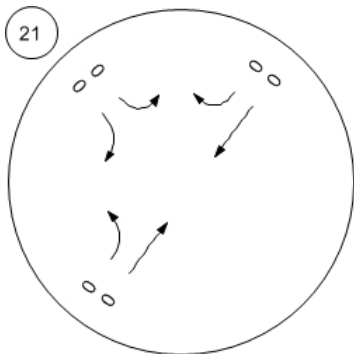
Willow Rd (SR 114)/O'Brien



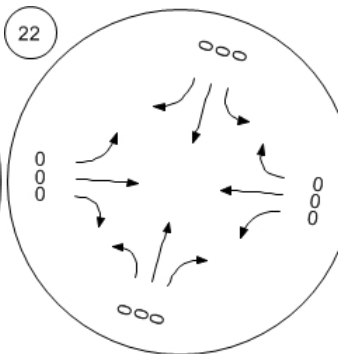
Willow Rd (SR 114)/Newbrid



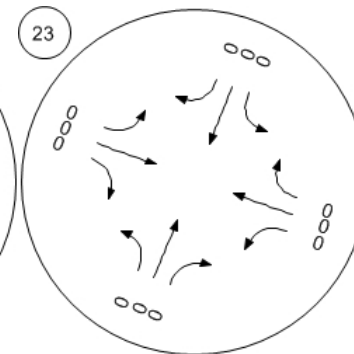
Willow Rd/Bay Rd



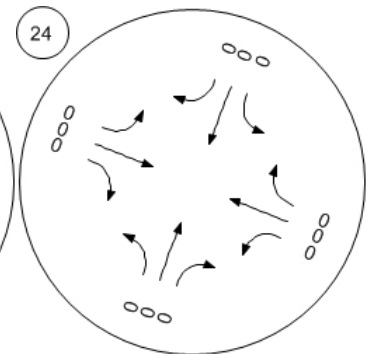
Willow Rd/Durham St-VA Me



Willow Rd/Coleman Ave



Willow Rd/Gilbert Ave



Traffic Volume - Other Volume

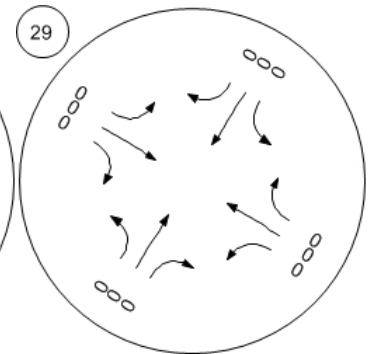
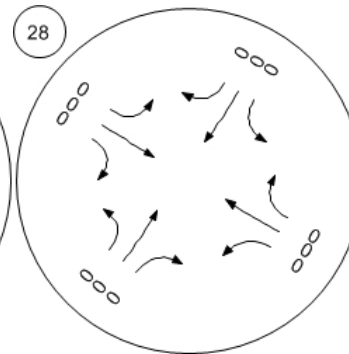
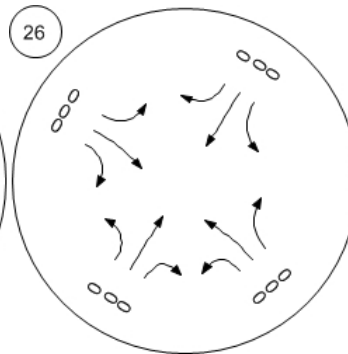
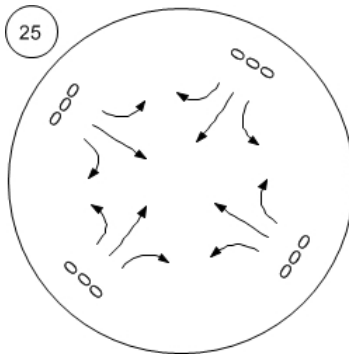


Middlefield Rd-Willow Rd

Ravenswood Ave/Laurel St

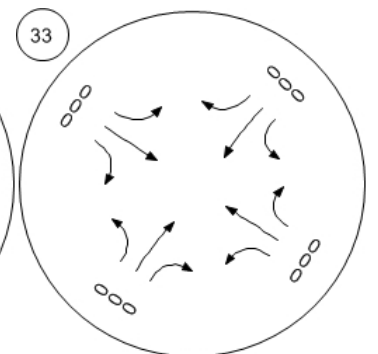
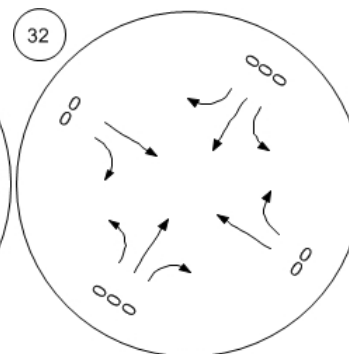
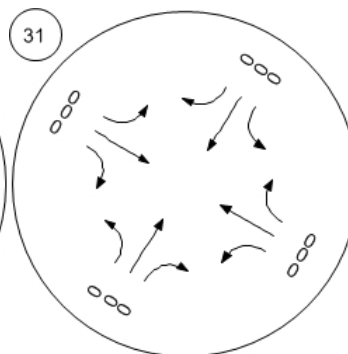
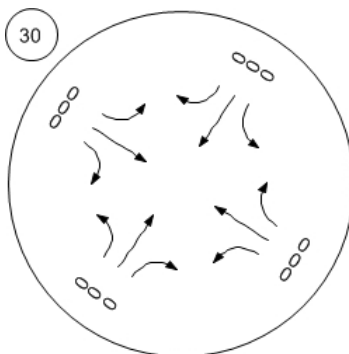
Oak Grove Ave/Laurel St

El Camino Real (SR 82)/Enci



El Camino Real (SR 82)/Glen

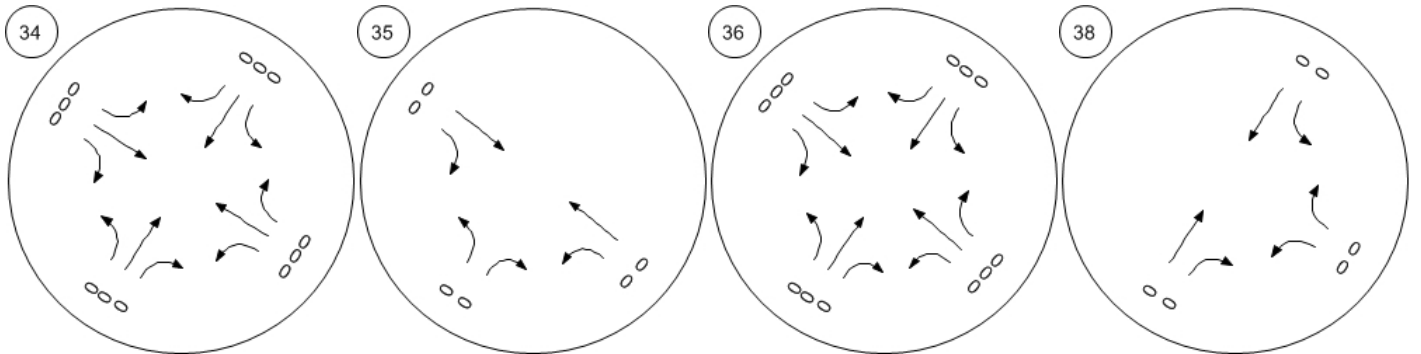
El Camino Real (SR 82)/Oak El Camino Real (SR 82)/Sant El Camino Real (SR 82)/Rav



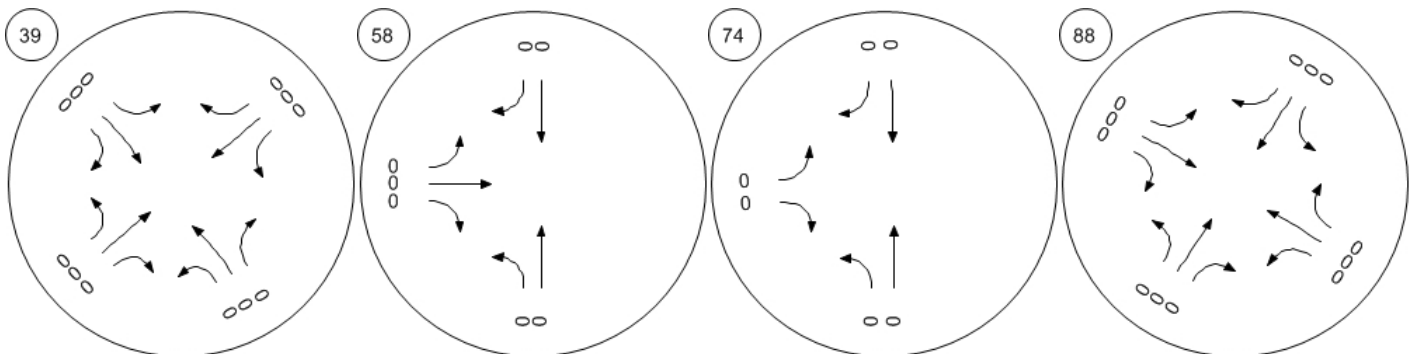
Traffic Volume - Other Volume



El Camino Real (SR 82)/Robl El Camino Real (SR 82)/Middle El Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



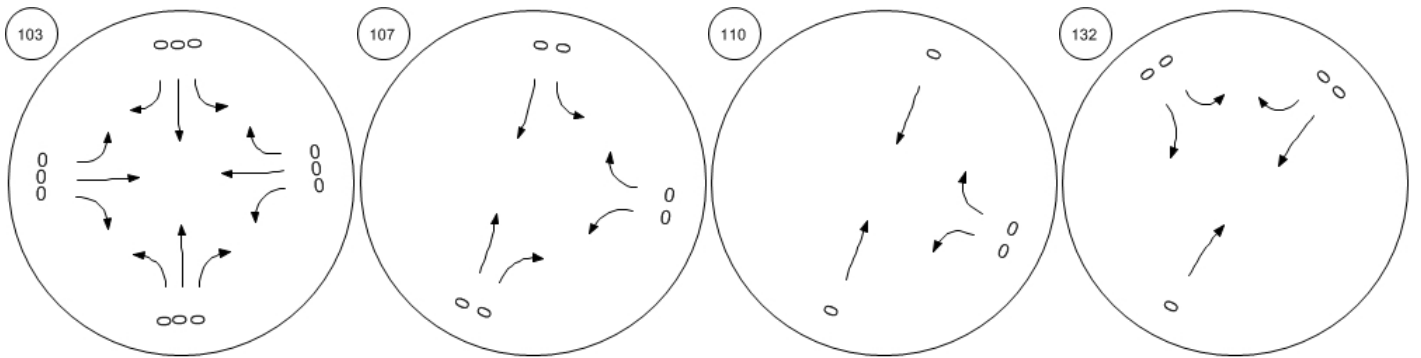
Sand Hill Rd/Santa Cruz Ave University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr



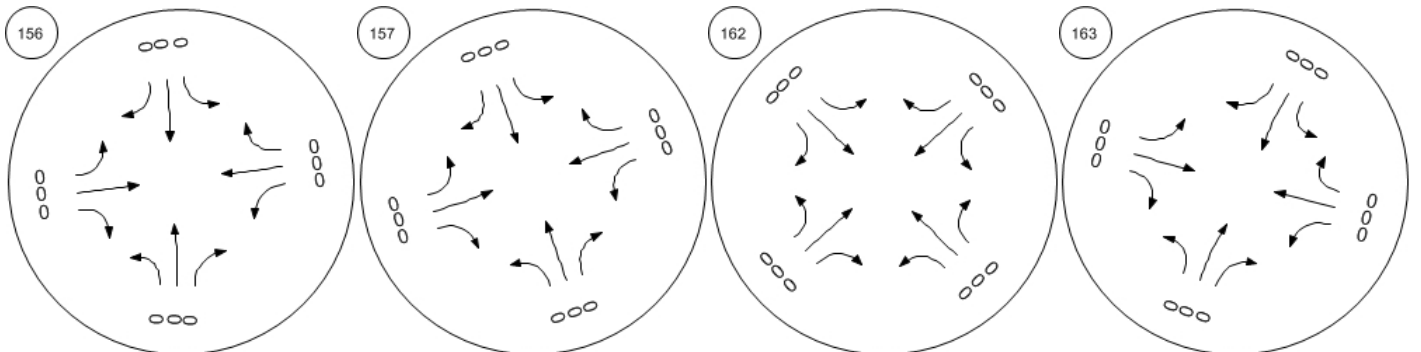
Traffic Volume - Other Volume



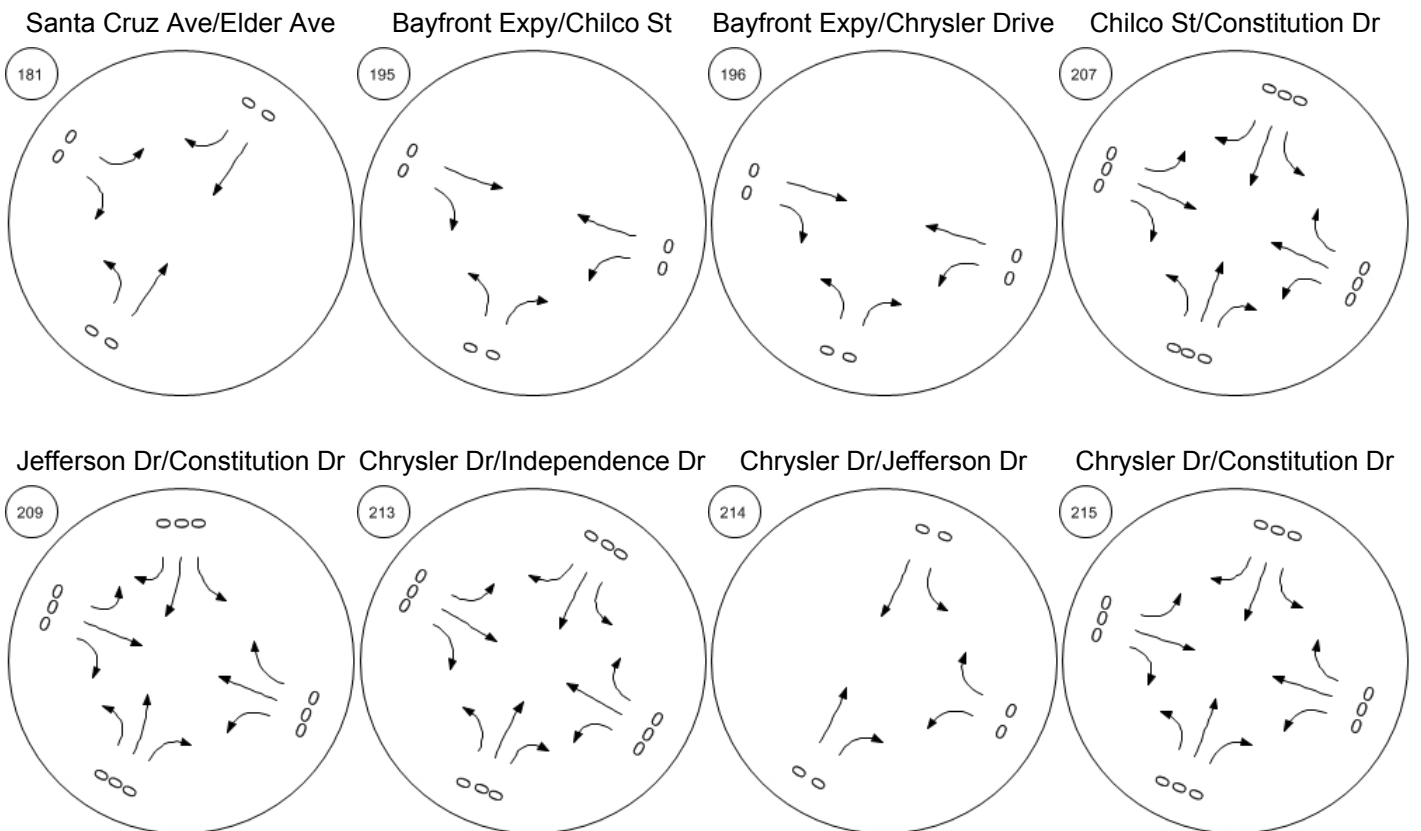
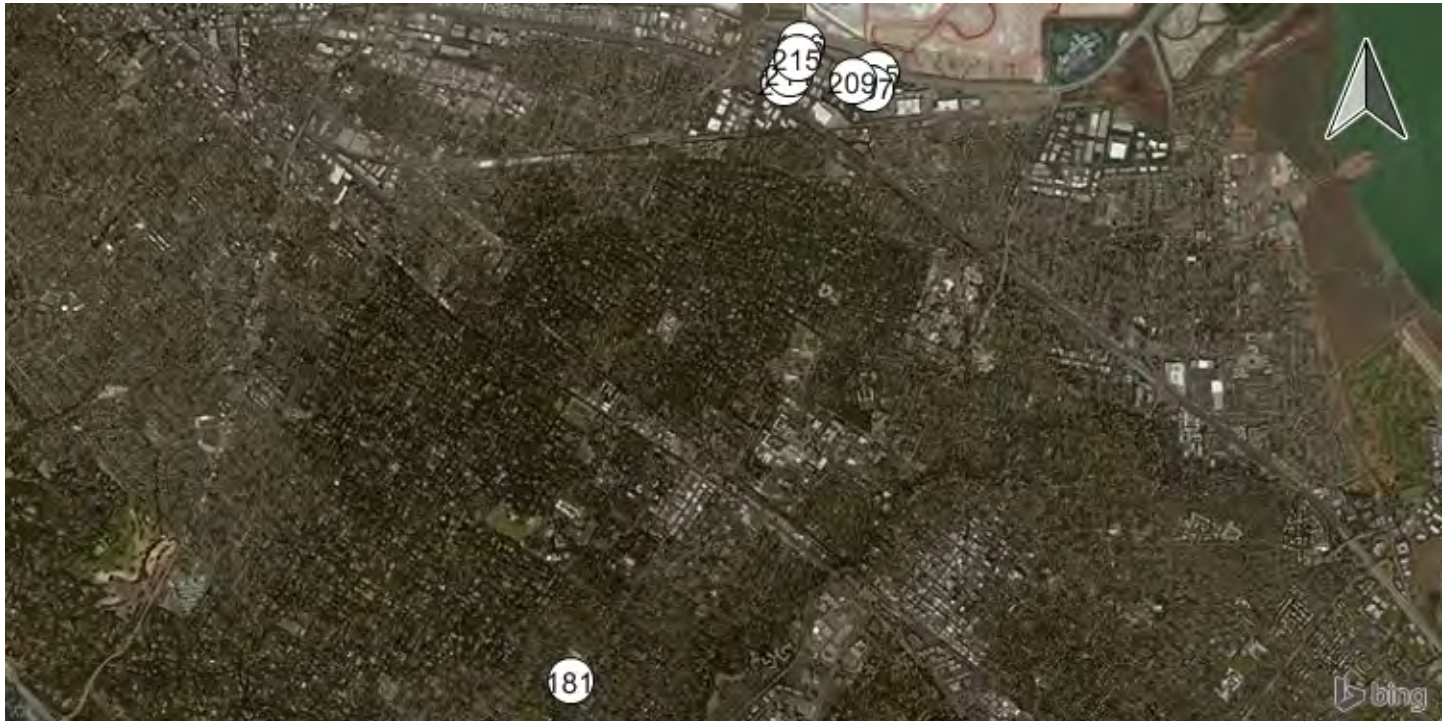
Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road/101 NB Ramps Oak Ave/Sand Hill Rd



Saga Ln/Sand Hill Rd Branner Dr/Sand Hill Rd Sharon Park Dr/ Sand Hill Rd Bayfront Expy/Marsh Rd



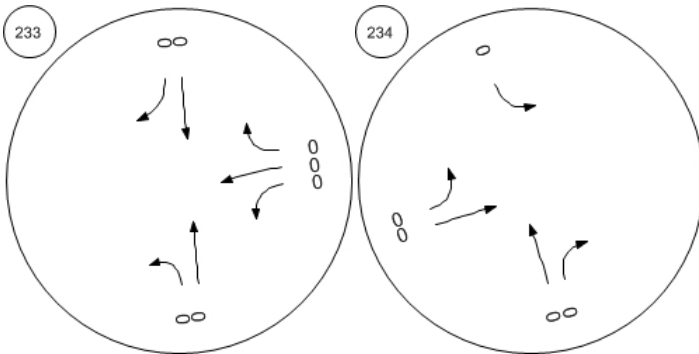
Traffic Volume - Other Volume



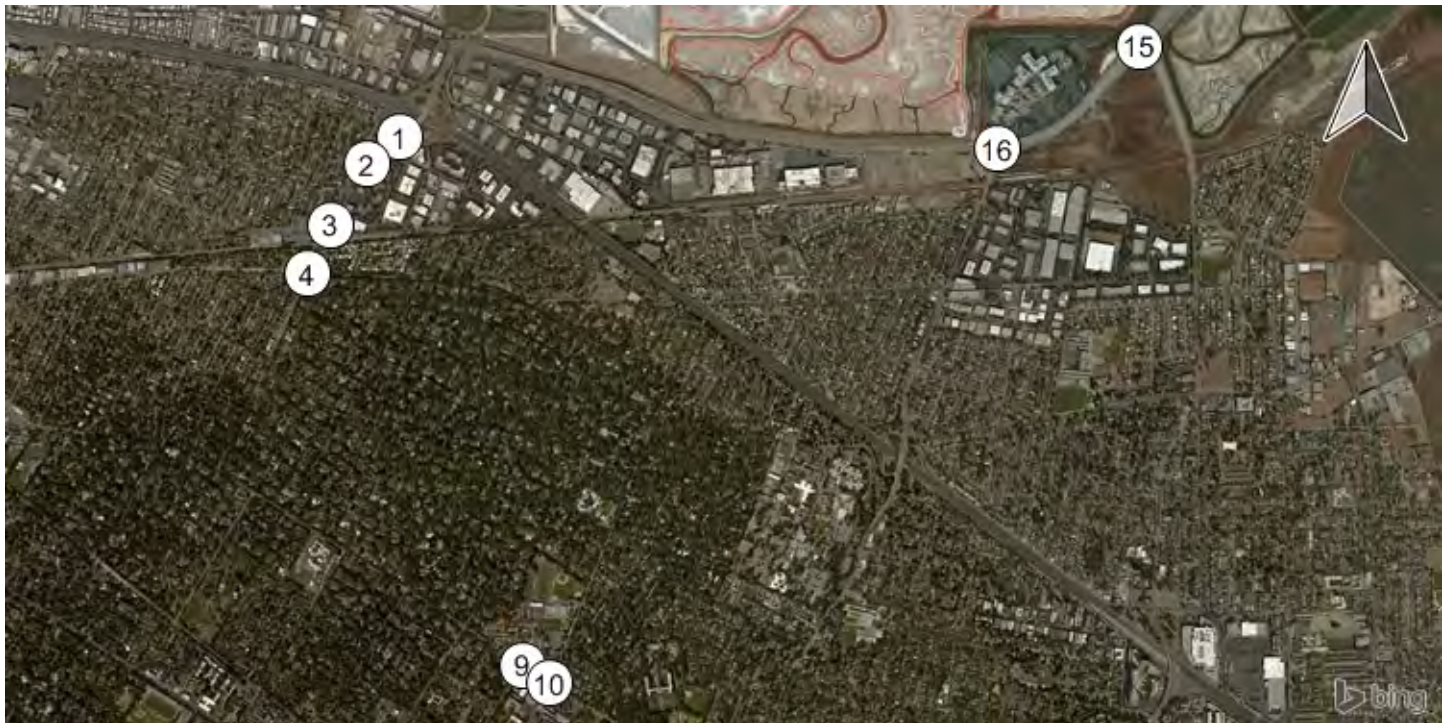
Traffic Volume - Other Volume



Sand Hill Circle/Sand Hill Ro Sand Hill Rd/Hwy 280 NB Off

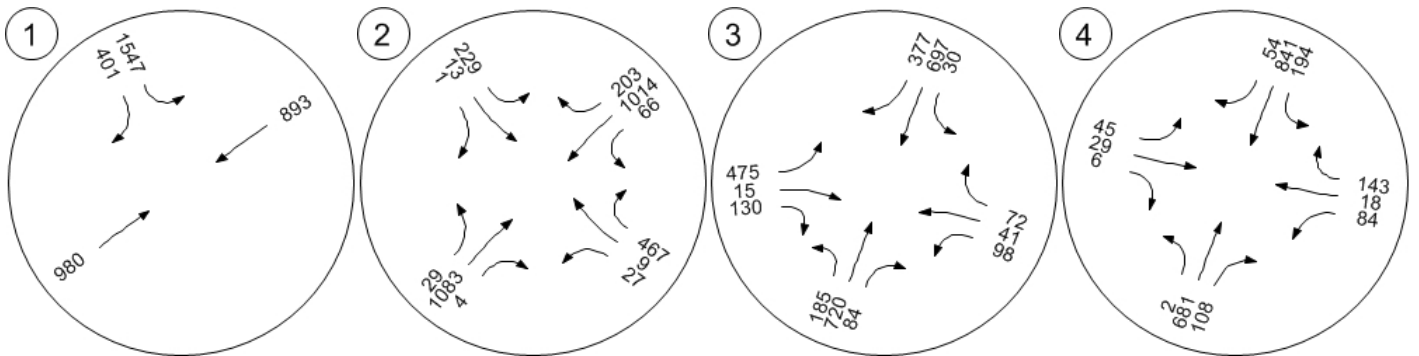


Traffic Volume - Future Total Volume



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

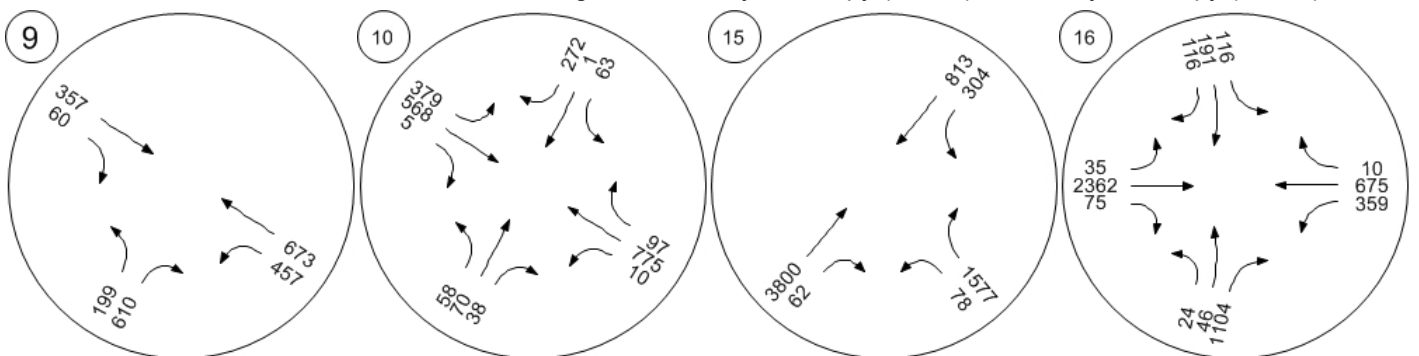


Middlefield Rd/Ravenswood

Middlefield Rd/Ringwood Ave

Bayfront Expy (SR 84)/Univer

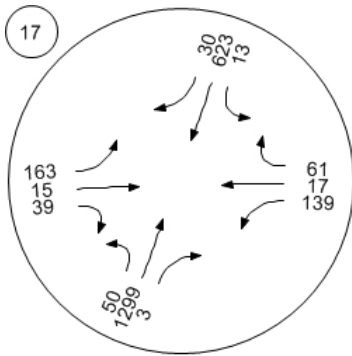
Bayfront Expy (SR 84)/Willow



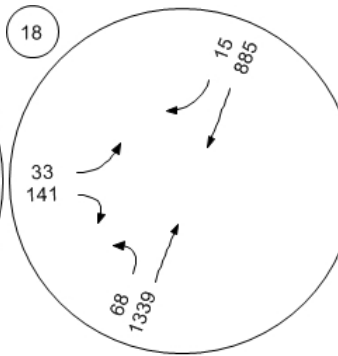
Traffic Volume - Future Total Volume



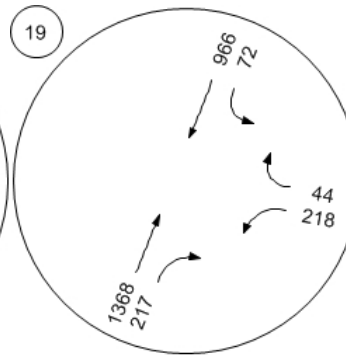
Willow Rd (SR 114)/Hamilton



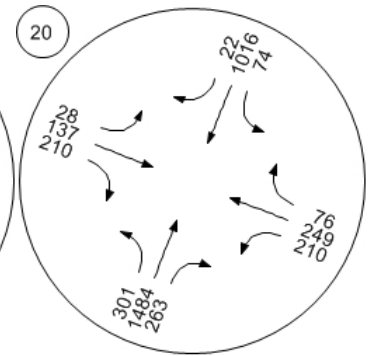
Willow Rd (SR 114)/Ivy Dr



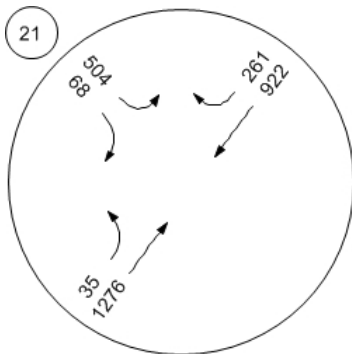
Willow Rd (SR 114)/O'Brien



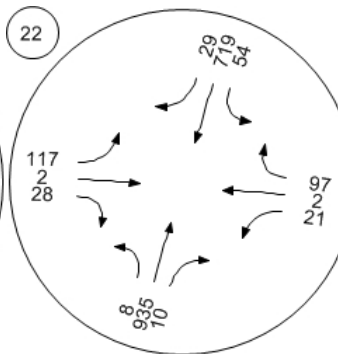
Willow Rd (SR 114)/Newbrid



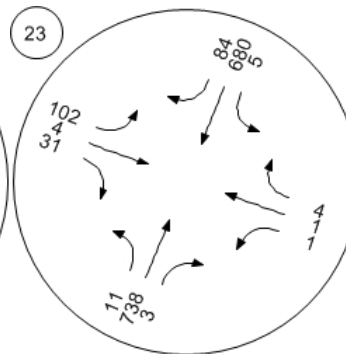
Willow Rd/Bay Rd



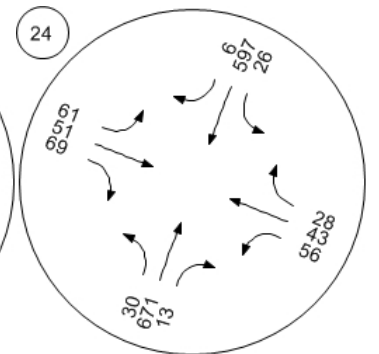
Willow Rd/Durham St-VA Me



Willow Rd/Coleman Ave



Willow Rd/Gilbert Ave



Traffic Volume - Future Total Volume

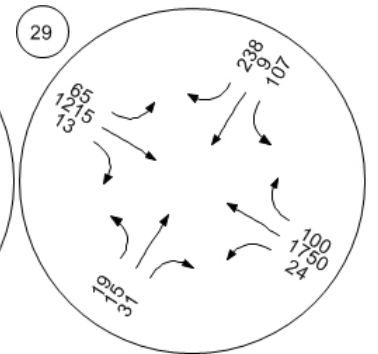
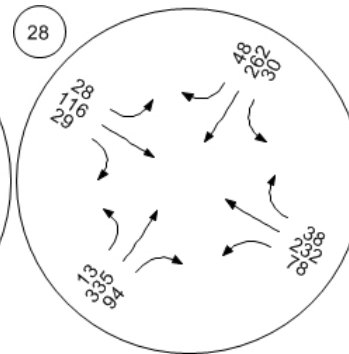
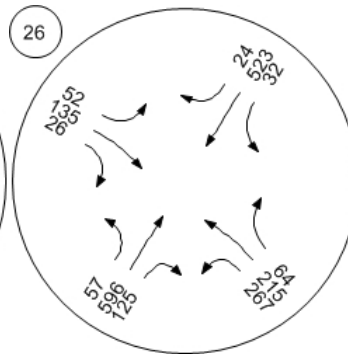
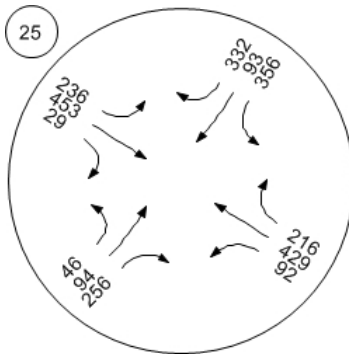


Middlefield Rd-Willow Rd

Ravenswood Ave/Laurel St

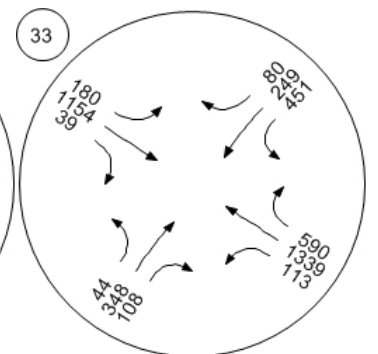
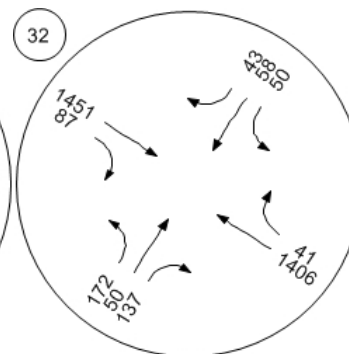
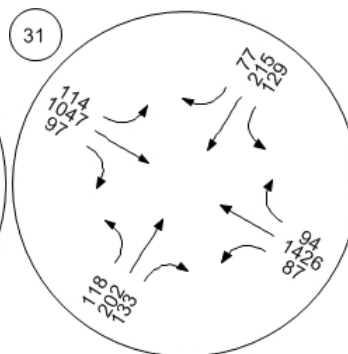
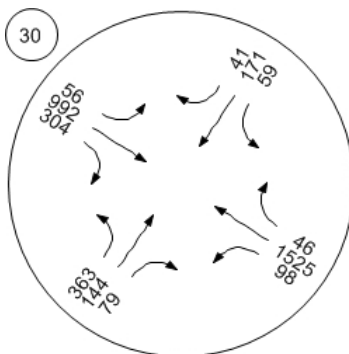
Oak Grove Ave/Laurel St

El Camino Real (SR 82)/Enci



El Camino Real (SR 82)/Glen

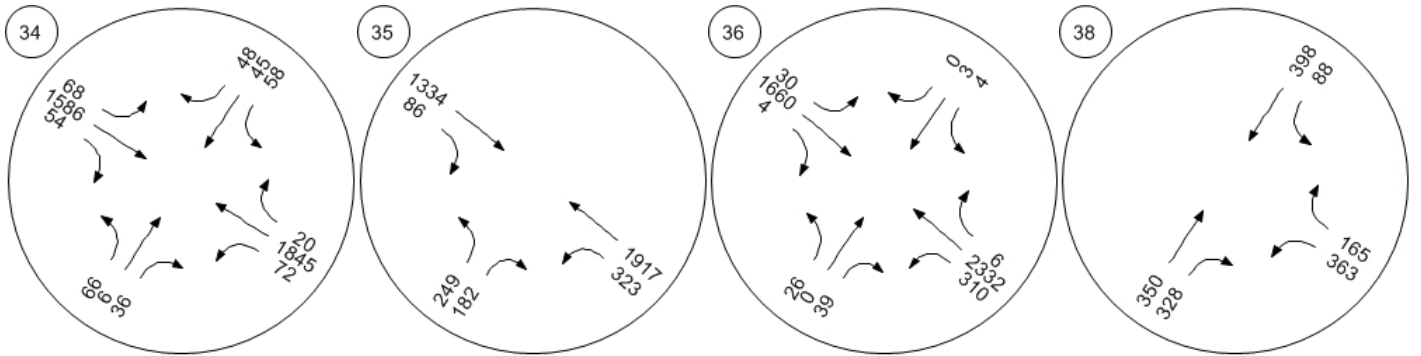
El Camino Real (SR 82)/Oak El Camino Real (SR 82)/Sant El Camino Real (SR 82)/Rav



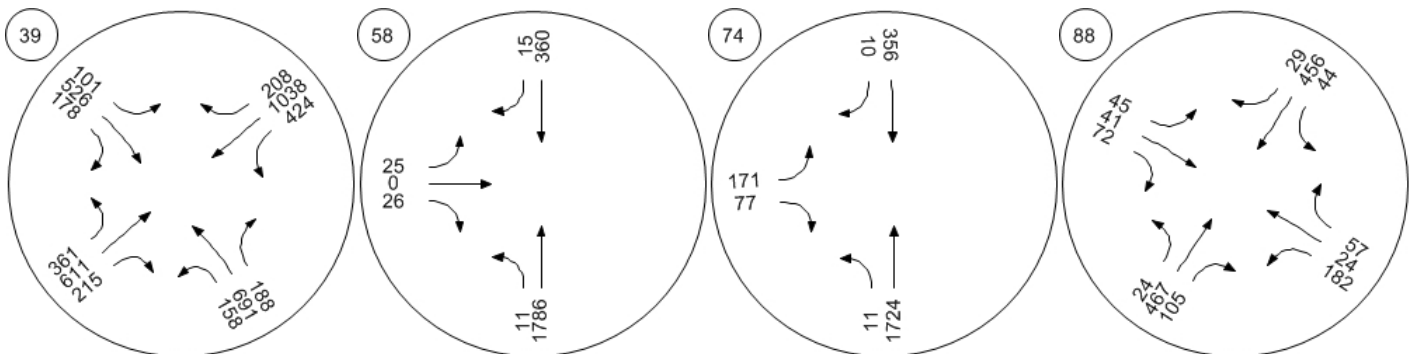
Traffic Volume - Future Total Volume



El Camino Real (SR 82)/Rob El Camino Real (SR 82)/Middle Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



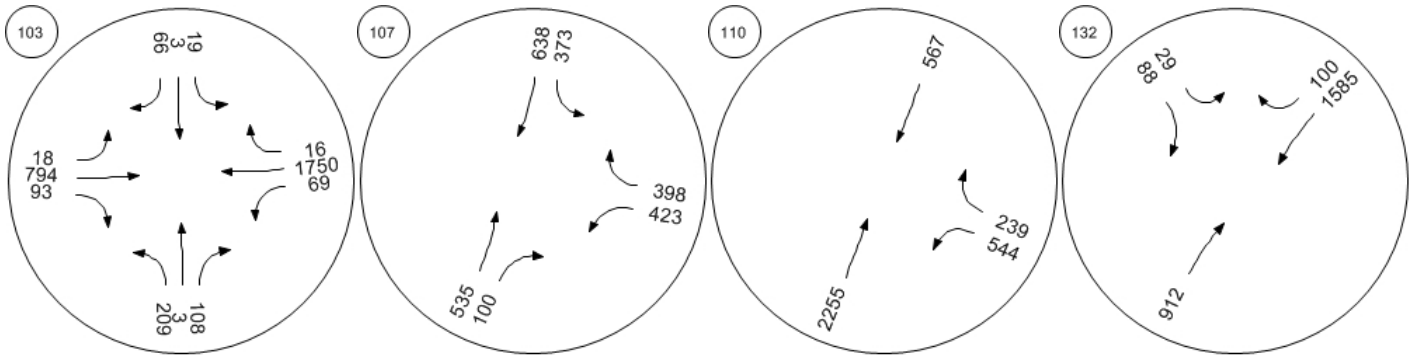
Sand Hill Rd/Santa Cruz Ave University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr



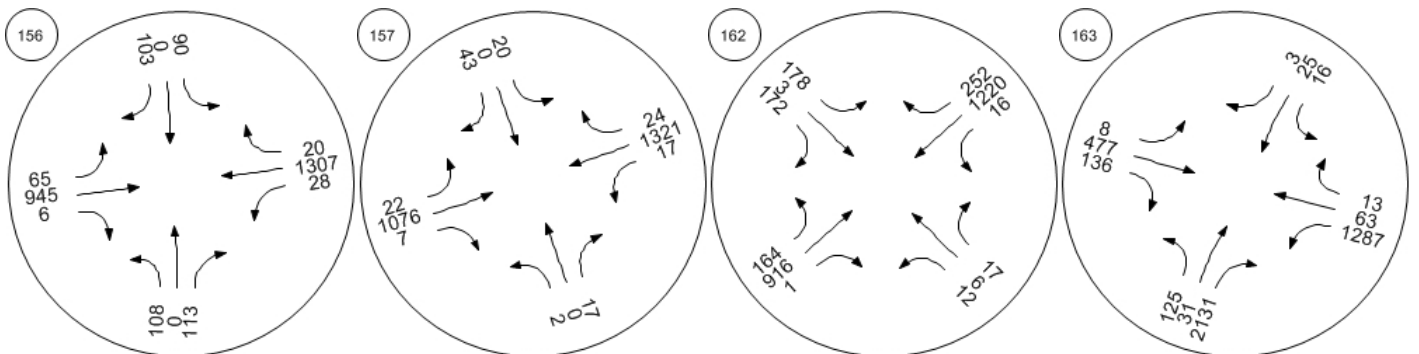
Traffic Volume - Future Total Volume



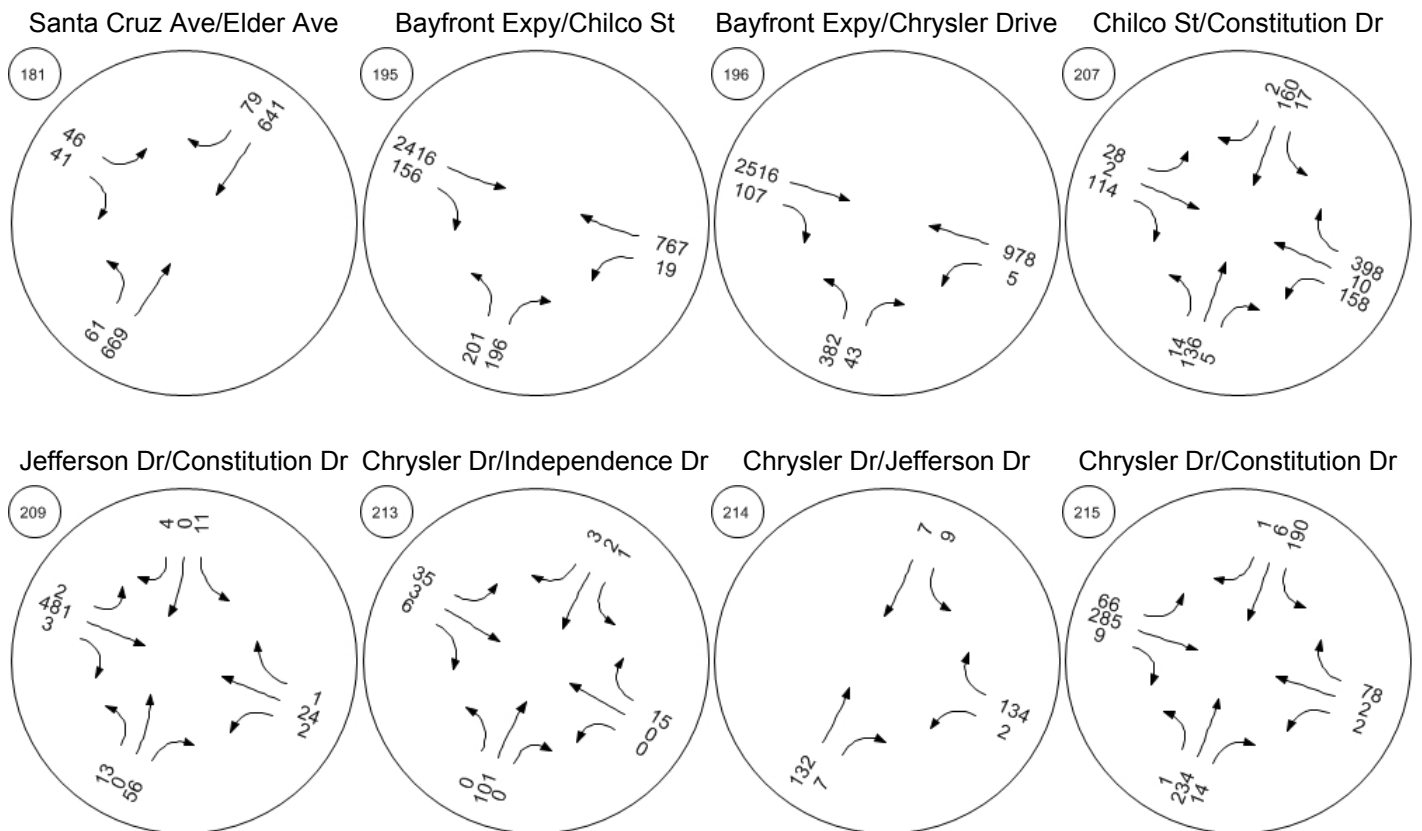
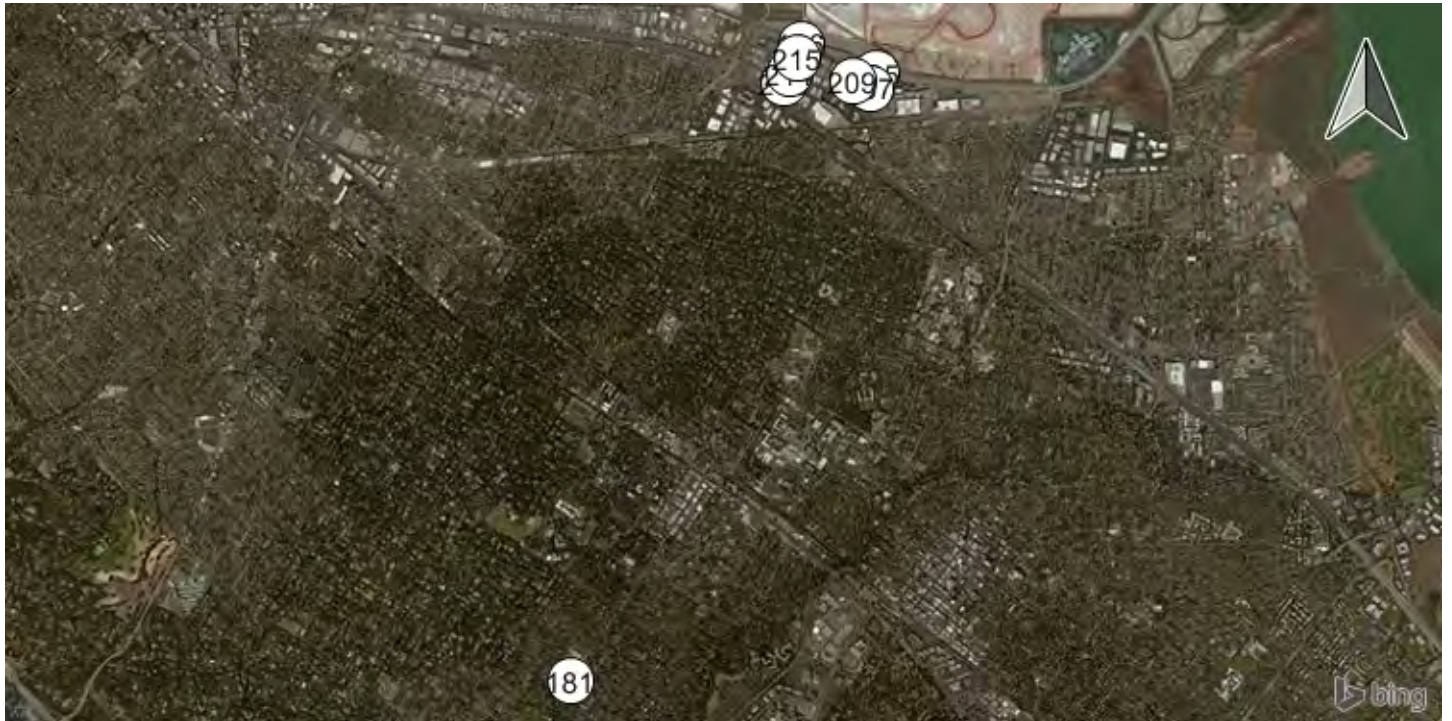
Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road/101 NB Ramps Oak Ave/Sand Hill Rd



Saga Ln/Sand Hill Rd Branner Dr/Sand Hill Rd Sharon Park Dr/ Sand Hill Rd Bayfront Expy/Marsh Rd



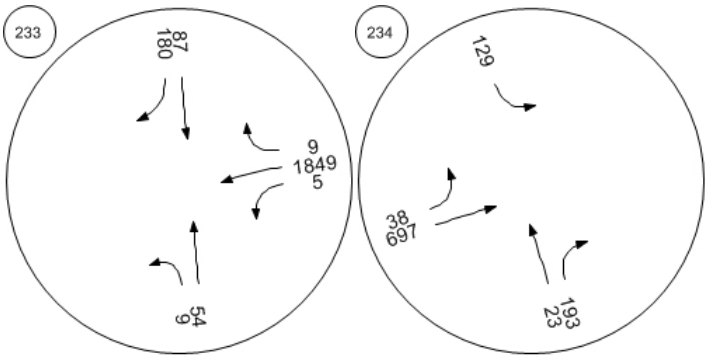
Traffic Volume - Future Total Volume



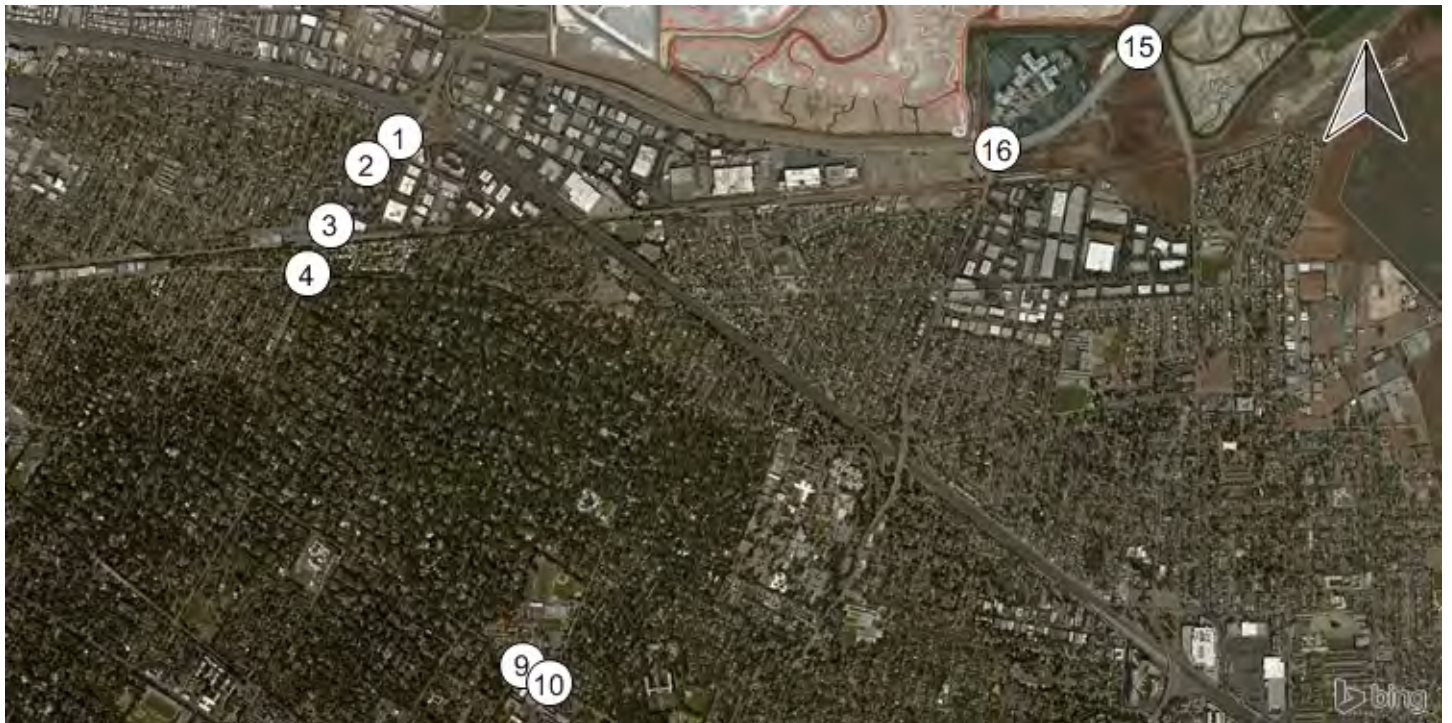
Traffic Volume - Future Total Volume



Sand Hill Circle/Sand Hill Ro Sand Hill Rd/Hwy 280 NB Off

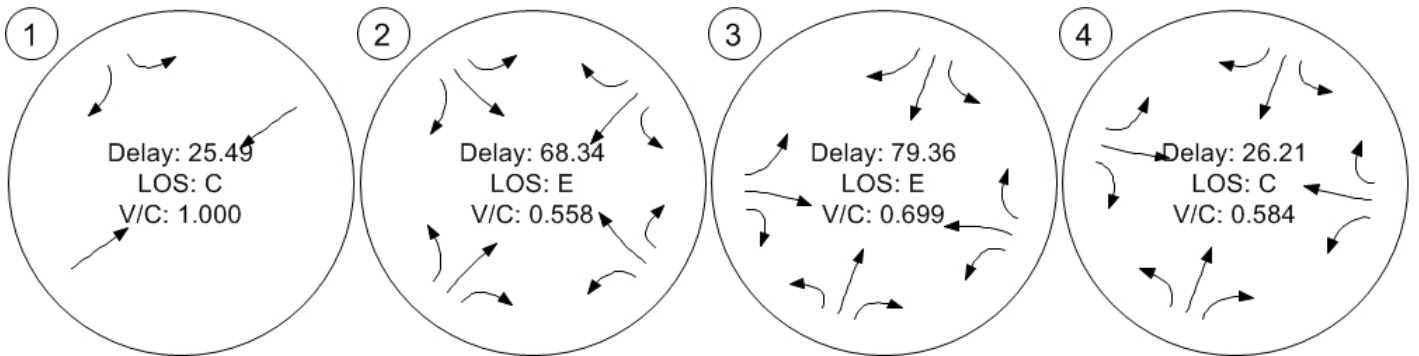


Traffic Conditions



Marsh Rd (SR 84)/US 101 S Marsh Rd/Rolison Rd-Scott D Marsh Rd/Florence St-Bohan

Marsh Rd/Bay Rd

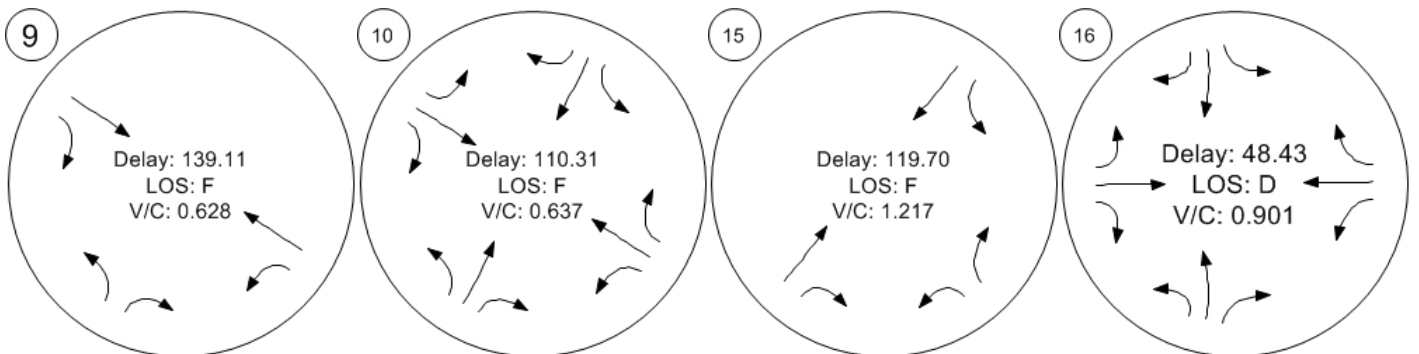


Middlefield Rd/Ravenswood

Middlefield Rd/Ringwood Ave

Bayfront Expy (SR 84)/Univer

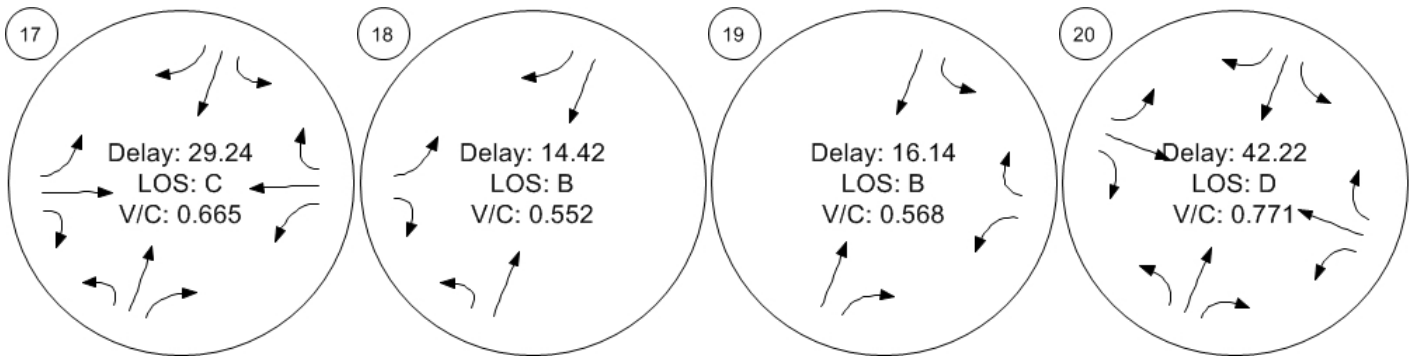
Bayfront Expy (SR 84)/Willow



Traffic Conditions



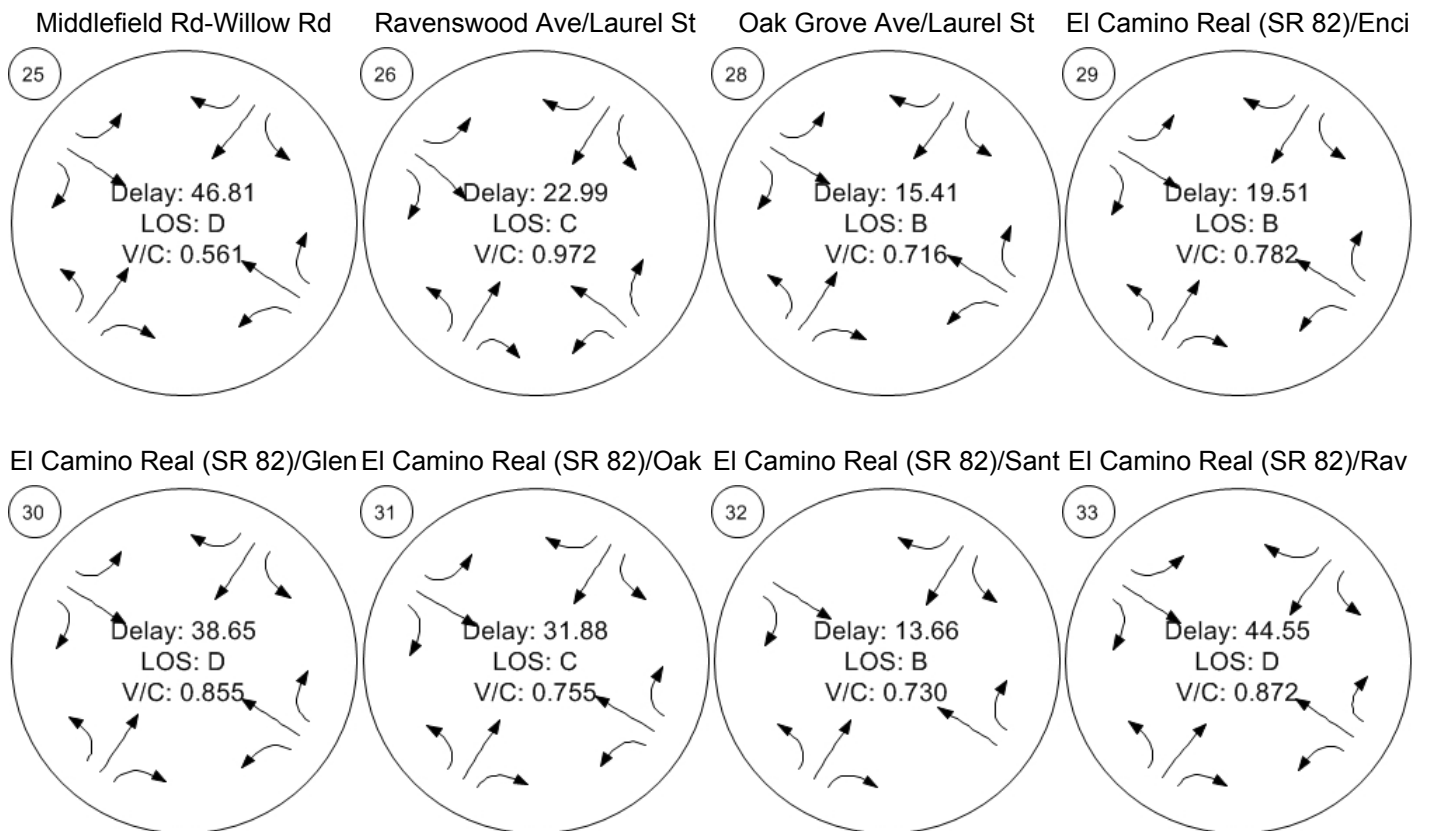
Willow Rd (SR 114)/Hamilton Willow Rd (SR 114)/Ivy Dr Willow Rd (SR 114)/O'Brien Willow Rd (SR 114)/Newbrid



Willow Rd/Bay Rd Willow Rd/Durham St-VA Me Willow Rd/Coleman Ave Willow Rd/Gilbert Ave



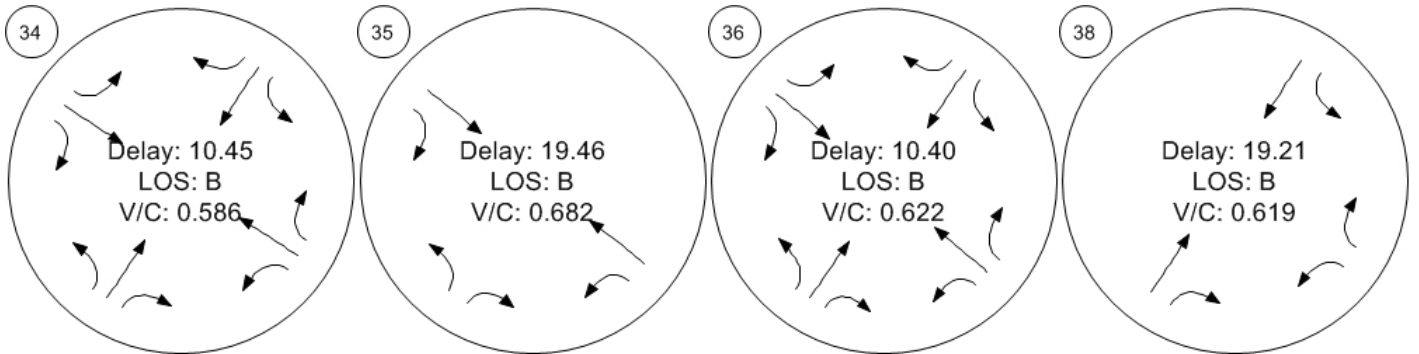
Traffic Conditions



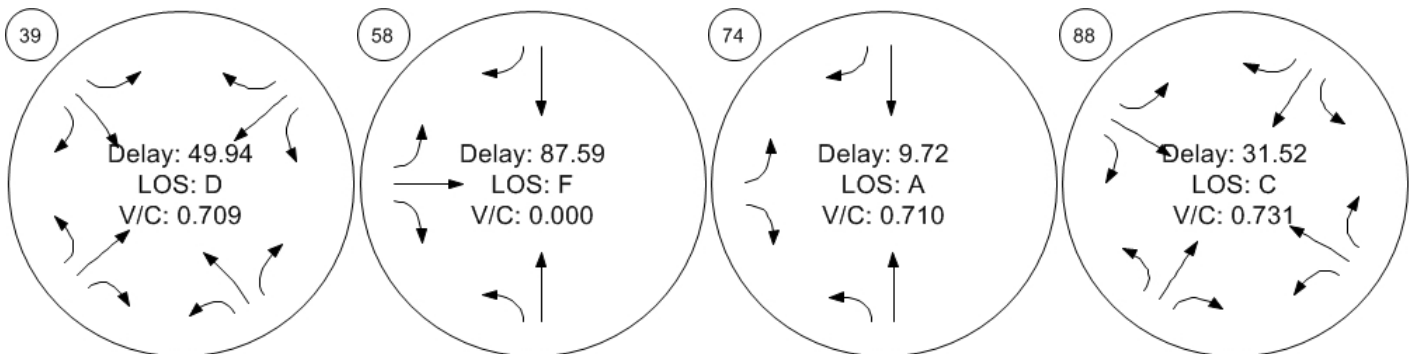
Traffic Conditions



El Camino Real (SR 82)/Robl El Camino Real (SR 82)/Middle El Camino Real (SR 82)/Cam Santa Cruz Ave/University Dr



Sand Hill Rd/Santa Cruz Ave University Avenue and Adam University Ave/O'Brien Dr Valparaiso Ave/ University Dr



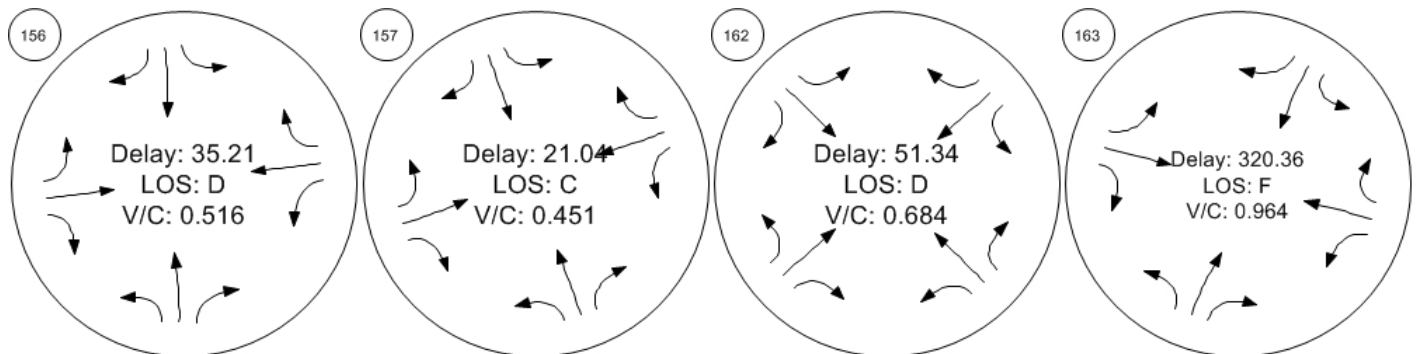
Traffic Conditions



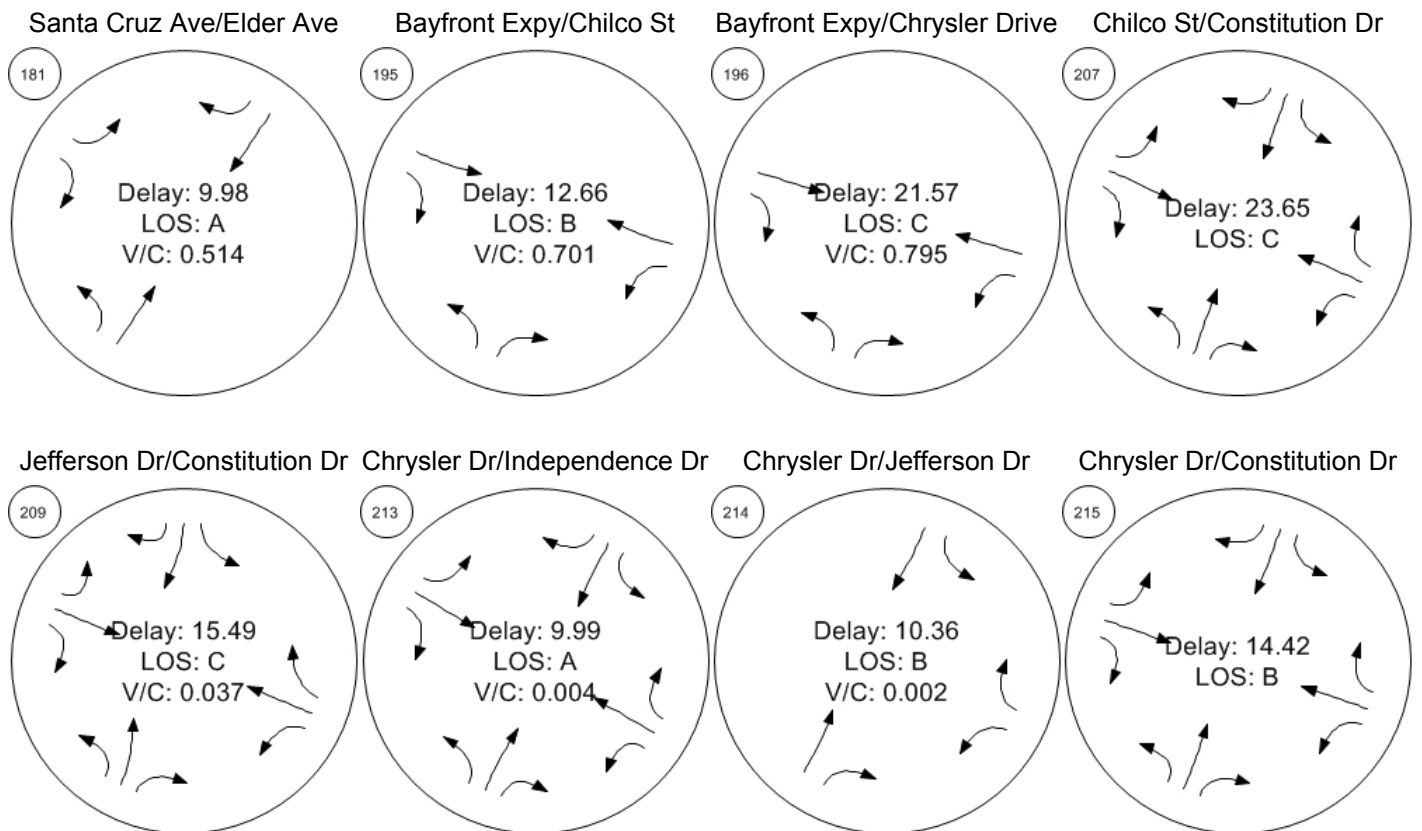
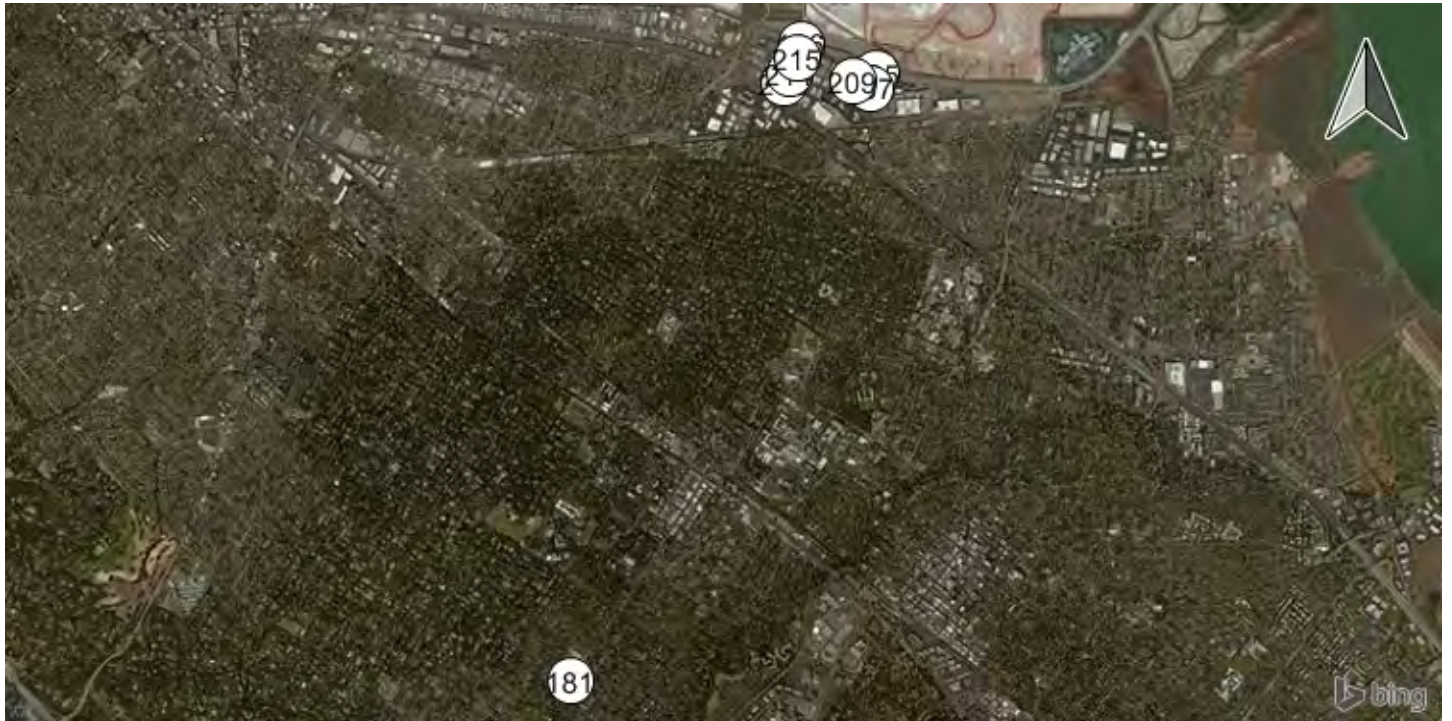
Addison Wesley/Sand Hill Rd Alpine Rd/Santa Cruz Ave&J Marsh Road/101 NB Ramps Oak Ave/Sand Hill Rd



Saga Ln/Sand Hill Rd Branner Dr/Sand Hill Rd Sharon Park Dr/ Sand Hill Rd Bayfront Expy/Marsh Rd



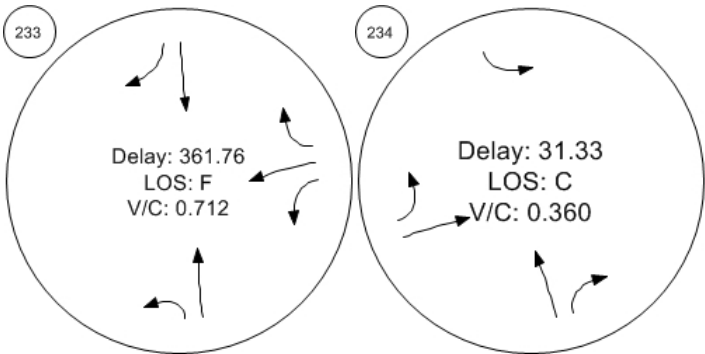
Traffic Conditions



Traffic Conditions



Sand Hill Circle/Sand Hill Ro Sand Hill Rd/Hwy 280 NB Off



A P P E N D I X C

A D T D A T A S H E E T S



LOCATION: 1. Alameda De Las Pulgas **QC JOB #:** 12899401
SPECIFIC LOCATION: 100 ft from Avy Ave **DIRECTION:** NB/SB
CITY/STATE: Menlo Park, CA **DATE:** Sep 23 2014 - Sep 23 2014

Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday	Sat	Sun	Average Week	Average Week Profile
	23-Sep-14					Hourly Traffic			Hourly Traffic	
12:00 AM		39				39			39	
1:00 AM		12				12			12	
2:00 AM		11				11			11	
3:00 AM		10				10			10	
4:00 AM		26				26			26	
5:00 AM		91				91			91	
6:00 AM		252				252			252	
7:00 AM		844				844			844	
8:00 AM		1067				1067			1067	
9:00 AM		829				829			829	
10:00 AM		677				677			677	
11:00 AM		749				749			749	
12:00 PM		829				829			829	
1:00 PM		742				742			742	
2:00 PM		754				754			754	
3:00 PM		976				976			976	
4:00 PM		949				949			949	
5:00 PM		1052				1052			1052	
6:00 PM		1024				1024			1024	
7:00 PM		629				629			629	
8:00 PM		408				408			408	
9:00 PM		257				257			257	
10:00 PM		145				145			145	
11:00 PM		77				77			77	
Day Total		12449				12449			12449	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		1067				1067			1067	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		1052				1052			1052	

Comments: none

LOCATION: 2. Alameda De Las Pulgas SPECIFIC LOCATION: 100 ft from Valparaiso Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899402 DIRECTION: NB/SB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		46				46			46	
1:00 AM		13				13			13	
2:00 AM		9				9			9	
3:00 AM		13				13			13	
4:00 AM		41				41			41	
5:00 AM		113				113			113	
6:00 AM		311				311			311	
7:00 AM		1026				1026			1026	
8:00 AM		1274				1274			1274	
9:00 AM		1031				1031			1031	
10:00 AM		871				871			871	
11:00 AM		893				893			893	
12:00 PM		977				977			977	
1:00 PM		907				907			907	
2:00 PM		989				989			989	
3:00 PM		1279				1279			1279	
4:00 PM		1207				1207			1207	
5:00 PM		1338				1338			1338	
6:00 PM		1202				1202			1202	
7:00 PM		757				757			757	
8:00 PM		490				490			490	
9:00 PM		304				304			304	
10:00 PM		147				147			147	
11:00 PM		91				91			91	
Day Total		15329				15329			15329	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		1274				1274			1274	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		1338				1338			1338	
<i>Comments:</i> none										

LOCATION: 3. Alameda De Las Pulgas SPECIFIC LOCATION: 100 ft from City Limits CITY/STATE: Menlo Park, CA						QC JOB #: 12899403 DIRECTION: NB/SB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				42		42			42	
1:00 AM				22		22			22	
2:00 AM				12		12			12	
3:00 AM				12		12			12	
4:00 AM				33		33			33	
5:00 AM				107		107			107	
6:00 AM				339		339			339	
7:00 AM				1056		1056			1056	
8:00 AM				1406		1406			1406	
9:00 AM				1100		1100			1100	
10:00 AM				1016		1016			1016	
11:00 AM				977		977			977	
12:00 PM				1034		1034			1034	
1:00 PM				1051		1051			1051	
2:00 PM				1136		1136			1136	
3:00 PM				1398		1398			1398	
4:00 PM				1289		1289			1289	
5:00 PM				1349		1349			1349	
6:00 PM				1094		1094			1094	
7:00 PM				640		640			640	
8:00 PM				439		439			439	
9:00 PM				301		301			301	
10:00 PM				186		186			186	
11:00 PM				102		102			102	
Day Total				16141		16141			16141	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				1406		1406			1406	
PM Peak				3:00 PM		3:00 PM			3:00 PM	
Volume				1398		1398			1398	
<i>Comments:</i> none										

LOCATION: 4. Alma St SPECIFIC LOCATION: 100 ft from Ravenswood Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899404 DIRECTION: NB/SB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				6		6			6	
1:00 AM				3		3			3	
2:00 AM				7		7			7	
3:00 AM				3		3			3	
4:00 AM				12		12			12	
5:00 AM				21		21			21	
6:00 AM				40		40			40	
7:00 AM				82		82			82	
8:00 AM				89		89			89	
9:00 AM				94		94			94	
10:00 AM				93		93			93	
11:00 AM				126		126			126	
12:00 PM				106		106			106	
1:00 PM				122		122			122	
2:00 PM				107		107			107	
3:00 PM				151		151			151	
4:00 PM				135		135			135	
5:00 PM				123		123			123	
6:00 PM				115		115			115	
7:00 PM				70		70			70	
8:00 PM				46		46			46	
9:00 PM				39		39			39	
10:00 PM				36		36			36	
11:00 PM				14		14			14	
Day Total				1640		1640			1640	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				11:00 AM		11:00 AM			11:00 AM	
Volume				126		126			126	
PM Peak				3:00 PM		3:00 PM			3:00 PM	
Volume				151		151			151	
<i>Comments:</i> none										

LOCATION: 5. Alma St SPECIFIC LOCATION: 100 ft from Willow Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899405 DIRECTION: NB/SB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				4		4			4	
1:00 AM				3		3			3	
2:00 AM				2		2			2	
3:00 AM				2		2			2	
4:00 AM				2		2			2	
5:00 AM				23		23			23	
6:00 AM				52		52			52	
7:00 AM				134		134			134	
8:00 AM				216		216			216	
9:00 AM				190		190			190	
10:00 AM				171		171			171	
11:00 AM				169		169			169	
12:00 PM				210		210			210	
1:00 PM				249		249			249	
2:00 PM				224		224			224	
3:00 PM				265		265			265	
4:00 PM				316		316			316	
5:00 PM				320		320			320	
6:00 PM				240		240			240	
7:00 PM				150		150			150	
8:00 PM				151		151			151	
9:00 PM				79		79			79	
10:00 PM				48		48			48	
11:00 PM				20		20			20	
Day Total				3240		3240			3240	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak Volume				8:00 AM 216		8:00 AM 216			8:00 AM 216	
PM Peak Volume				5:00 PM 320		5:00 PM 320			5:00 PM 320	
<i>Comments:</i> none										

LOCATION: 6. Alpine Rd SPECIFIC LOCATION: 100 ft from City Limits CITY/STATE: Menlo Park, CA						QC JOB #: 12899406 DIRECTION: EB/WB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue 21-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		55				55			55	
1:00 AM		25				25			25	
2:00 AM		12				12			12	
3:00 AM		28				28			28	
4:00 AM		56				56			56	
5:00 AM		282				282			282	
6:00 AM		837				837			837	
7:00 AM		1681				1681			1681	
8:00 AM		1857				1857			1857	
9:00 AM		1733				1733			1733	
10:00 AM		1442				1442			1442	
11:00 AM		1278				1278			1278	
12:00 PM		1355				1355			1355	
1:00 PM		1279				1279			1279	
2:00 PM		1536				1536			1536	
3:00 PM		1721				1721			1721	
4:00 PM		1751				1751			1751	
5:00 PM		1935				1935			1935	
6:00 PM		1760				1760			1760	
7:00 PM		1099				1099			1099	
8:00 PM		627				627			627	
9:00 PM		543				543			543	
10:00 PM		267				267			267	
11:00 PM		146				146			146	
Day Total		23305				23305			23305	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		1857				1857			1857	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		1935				1935			1935	
<i>Comments:</i> none										

LOCATION: 7. Avy Ave SPECIFIC LOCATION: 100 ft from City Limit CITY/STATE: Menlo Park, CA						QC JOB #: 12899407 DIRECTION: EB/WB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		6				6			6	
1:00 AM		4				4			4	
2:00 AM		0				0			0	
3:00 AM		3				3			3	
4:00 AM		9				9			9	
5:00 AM		18				18			18	
6:00 AM		60				60			60	
7:00 AM		302				302			302	
8:00 AM		505				505			505	
9:00 AM		291				291			291	
10:00 AM		244				244			244	
11:00 AM		309				309			309	
12:00 PM		313				313			313	
1:00 PM		256				256			256	
2:00 PM		337				337			337	
3:00 PM		450				450			450	
4:00 PM		396				396			396	
5:00 PM		416				416			416	
6:00 PM		298				298			298	
7:00 PM		185				185			185	
8:00 PM		94				94			94	
9:00 PM		66				66			66	
10:00 PM		27				27			27	
11:00 PM		17				17			17	
Day Total		4606				4606			4606	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		505				505			505	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		450				450			450	
<i>Comments:</i> none										

LOCATION: 8. Avy Ave SPECIFIC LOCATION: 100 ft from Alameda de las Pulgas CITY/STATE: Menlo Park, CA						QC JOB #: 12899408 DIRECTION: EB/WB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		6				6			6	
1:00 AM		5				5			5	
2:00 AM		1				1			1	
3:00 AM		4				4			4	
4:00 AM		8				8			8	
5:00 AM		29				29			29	
6:00 AM		73				73			73	
7:00 AM		303				303			303	
8:00 AM		507				507			507	
9:00 AM		406				406			406	
10:00 AM		331				331			331	
11:00 AM		388				388			388	
12:00 PM		417				417			417	
1:00 PM		386				386			386	
2:00 PM		383				383			383	
3:00 PM		458				458			458	
4:00 PM		459				459			459	
5:00 PM		588				588			588	
6:00 PM		524				524			524	
7:00 PM		305				305			305	
8:00 PM		166				166			166	
9:00 PM		119				119			119	
10:00 PM		42				42			42	
11:00 PM		27				27			27	
Day Total		5935				5935			5935	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		507				507			507	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		588				588			588	
<i>Comments:</i> none										

LOCATION: 9. Bay Rd SPECIFIC LOCATION: 100 ft from Greenwood CITY/STATE: Menlo Park, CA						QC JOB #: 12899409 DIRECTION: NB/SB DATE: Oct 02 2014 - Oct 02 2014				
Start Time	Mon	Tue	Wed	Thu 02-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				13		13			13	
1:00 AM				6		6			6	
2:00 AM				4		4			4	
3:00 AM				4		4			4	
4:00 AM				9		9			9	
5:00 AM				33		33			33	
6:00 AM				84		84			84	
7:00 AM				470		470			470	
8:00 AM				586		586			586	
9:00 AM				446		446			446	
10:00 AM				244		244			244	
11:00 AM				259		259			259	
12:00 PM				290		290			290	
1:00 PM				277		277			277	
2:00 PM				291		291			291	
3:00 PM				441		441			441	
4:00 PM				441		441			441	
5:00 PM				617		617			617	
6:00 PM				410		410			410	
7:00 PM				299		299			299	
8:00 PM				158		158			158	
9:00 PM				98		98			98	
10:00 PM				42		42			42	
11:00 PM				26		26			26	
Day Total				5548		5548			5548	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				586		586			586	
PM Peak				5:00 PM		5:00 PM			5:00 PM	
Volume				617		617			617	
<i>Comments:</i> none										

LOCATION: 10. Bay Rd SPECIFIC LOCATION: 100 ft from Ringwood Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899410 DIRECTION: NB/SB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue 21-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		10				10			10	
1:00 AM		8				8			8	
2:00 AM		2				2			2	
3:00 AM		5				5			5	
4:00 AM		10				10			10	
5:00 AM		29				29			29	
6:00 AM		112				112			112	
7:00 AM		508				508			508	
8:00 AM		686				686			686	
9:00 AM		352				352			352	
10:00 AM		221				221			221	
11:00 AM		259				259			259	
12:00 PM		288				288			288	
1:00 PM		257				257			257	
2:00 PM		297				297			297	
3:00 PM		482				482			482	
4:00 PM		487				487			487	
5:00 PM		624				624			624	
6:00 PM		475				475			475	
7:00 PM		240				240			240	
8:00 PM		134				134			134	
9:00 PM		107				107			107	
10:00 PM		44				44			44	
11:00 PM		21				21			21	
Day Total		5658				5658			5658	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		686				686			686	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		624				624			624	
<i>Comments:</i> none										

LOCATION: 11. Bay Rd SPECIFIC LOCATION: 100 ft from Willow Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899411 DIRECTION: NB/SB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue 21-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		9				9			9	
1:00 AM		8				8			8	
2:00 AM		2				2			2	
3:00 AM		3				3			3	
4:00 AM		12				12			12	
5:00 AM		51				51			51	
6:00 AM		194				194			194	
7:00 AM		752				752			752	
8:00 AM		815				815			815	
9:00 AM		481				481			481	
10:00 AM		303				303			303	
11:00 AM		328				328			328	
12:00 PM		383				383			383	
1:00 PM		336				336			336	
2:00 PM		454				454			454	
3:00 PM		695				695			695	
4:00 PM		767				767			767	
5:00 PM		835				835			835	
6:00 PM		583				583			583	
7:00 PM		240				240			240	
8:00 PM		144				144			144	
9:00 PM		118				118			118	
10:00 PM		41				41			41	
11:00 PM		27				27			27	
Day Total		7581				7581			7581	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		815				815			815	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		835				835			835	
<i>Comments:</i> none										

LOCATION: 16. Bohannon Dr SPECIFIC LOCATION: 100 ft from Campell Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899412 DIRECTION: NB/SB DATE: Oct 02 2014 - Oct 02 2014				
Start Time	Mon	Tue	Wed	Thu 02-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				13		13			13	
1:00 AM				1		1			1	
2:00 AM				3		3			3	
3:00 AM				13		13			13	
4:00 AM				72		72			72	
5:00 AM				64		64			64	
6:00 AM				115		115			115	
7:00 AM				217		217			217	
8:00 AM				264		264			264	
9:00 AM				282		282			282	
10:00 AM				245		245			245	
11:00 AM				295		295			295	
12:00 PM				331		331			331	
1:00 PM				377		377			377	
2:00 PM				287		287			287	
3:00 PM				270		270			270	
4:00 PM				297		297			297	
5:00 PM				367		367			367	
6:00 PM				175		175			175	
7:00 PM				78		78			78	
8:00 PM				41		41			41	
9:00 PM				31		31			31	
10:00 PM				43		43			43	
11:00 PM				27		27			27	
Day Total				3908		3908			3908	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak Volume				11:00 AM 295		11:00 AM 295			11:00 AM 295	
PM Peak Volume				1:00 PM 377		1:00 PM 377			1:00 PM 377	
<i>Comments:</i> none										

LOCATION: 17. Chilco St SPECIFIC LOCATION: 100 ft from Constitution Dr CITY/STATE: Menlo Park, CA						QC JOB #: 12899413 DIRECTION: EB/WB DATE: Oct 14 2014 - Oct 14 2014				
Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
		14-Oct-14								
12:00 AM		62				62			62	
1:00 AM		37				37			37	
2:00 AM		32				32			32	
3:00 AM		40				40			40	
4:00 AM		73				73			73	
5:00 AM		190				190			190	
6:00 AM		360				360			360	
7:00 AM		482				482			482	
8:00 AM		505				505			505	
9:00 AM		390				390			390	
10:00 AM		297				297			297	
11:00 AM		289				289			289	
12:00 PM		336				336			336	
1:00 PM		331				331			331	
2:00 PM		491				491			491	
3:00 PM		582				582			582	
4:00 PM		564				564			564	
5:00 PM		579				579			579	
6:00 PM		428				428			428	
7:00 PM		293				293			293	
8:00 PM		200				200			200	
9:00 PM		157				157			157	
10:00 PM		183				183			183	
11:00 PM		98				98			98	
Day Total		6999				6999			6999	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		505				505			505	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		582				582			582	
<i>Comments:</i> none										

LOCATION: 18. Chrysler Drive SPECIFIC LOCATION: 100 ft from Constitution Dr CITY/STATE: Menlo Park, CA						QC JOB #: 12899414 DIRECTION: EB/WB DATE: Oct 21 2014 - Oct 21 2014					
Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile	
	21-Oct-14										
12:00 AM		8				8			8		
1:00 AM		7				7			7		
2:00 AM		13				13			13		
3:00 AM		11				11			11		
4:00 AM		12				12			12		
5:00 AM		44				44			44		
6:00 AM		104				104			104		
7:00 AM		207				207			207		
8:00 AM		229				229			229		
9:00 AM		166				166			166		
10:00 AM		123				123			123		
11:00 AM		246				246			246		
12:00 PM		293				293			293		
1:00 PM		211				211			211		
2:00 PM		217				217			217		
3:00 PM		338				338			338		
4:00 PM		481				481			481		
5:00 PM		697				697			697		
6:00 PM		377				377			377		
7:00 PM		134				134			134		
8:00 PM		40				40			40		
9:00 PM		52				52			52		
10:00 PM		27				27			27		
11:00 PM		31				31			31		
Day Total		4068				4068			4068		
% Weekday Average		100.0%									
% Week Average		100.0%				100.0%					
AM Peak		11:00 AM				11:00 AM			11:00 AM		
Volume		246				246			246		
PM Peak		5:00 PM				5:00 PM			5:00 PM		
Volume		697				697			697		
<i>Comments:</i> none											

LOCATION: 19. Constitution Dr SPECIFIC LOCATION: 100 ft from Chilco St CITY/STATE: Menlo Park, CA						QC JOB #: 12899415 DIRECTION: NB/SB DATE: Oct 14 2014 - Oct 14 2014				
Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
		14-Oct-14								
12:00 AM		6				6			6	
1:00 AM		9				9			9	
2:00 AM		10				10			10	
3:00 AM		6				6			6	
4:00 AM		14				14			14	
5:00 AM		14				14			14	
6:00 AM		54				54			54	
7:00 AM		166				166			166	
8:00 AM		233				233			233	
9:00 AM		140				140			140	
10:00 AM		92				92			92	
11:00 AM		96				96			96	
12:00 PM		97				97			97	
1:00 PM		101				101			101	
2:00 PM		108				108			108	
3:00 PM		122				122			122	
4:00 PM		226				226			226	
5:00 PM		560				560			560	
6:00 PM		153				153			153	
7:00 PM		60				60			60	
8:00 PM		34				34			34	
9:00 PM		25				25			25	
10:00 PM		19				19			19	
11:00 PM		14				14			14	
Day Total		2359				2359			2359	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		233				233			233	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		560				560			560	
<i>Comments:</i> none										

LOCATION: 20. Crane St SPECIFIC LOCATION: 100 ft from Oak Grove Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899416 DIRECTION: EB/WB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		2				2			2	
1:00 AM		0				0			0	
2:00 AM		1				1			1	
3:00 AM		1				1			1	
4:00 AM		15				15			15	
5:00 AM		10				10			10	
6:00 AM		22				22			22	
7:00 AM		103				103			103	
8:00 AM		148				148			148	
9:00 AM		186				186			186	
10:00 AM		177				177			177	
11:00 AM		235				235			235	
12:00 PM		251				251			251	
1:00 PM		215				215			215	
2:00 PM		192				192			192	
3:00 PM		233				233			233	
4:00 PM		224				224			224	
5:00 PM		213				213			213	
6:00 PM		179				179			179	
7:00 PM		108				108			108	
8:00 PM		59				59			59	
9:00 PM		66				66			66	
10:00 PM		12				12			12	
11:00 PM		10				10			10	
Day Total		2662				2662			2662	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		11:00 AM				11:00 AM			11:00 AM	
Volume		235				235			235	
PM Peak		12:00 PM				12:00 PM			12:00 PM	
Volume		251				251			251	
<i>Comments:</i> none										

LOCATION: 21. Crane St **QC JOB #:** 12899417
SPECIFIC LOCATION: 100 ft from Santa Cruz Ave **DIRECTION:** NB/SB
CITY/STATE: Menlo Park, CA **DATE:** Sep 23 2014 - Sep 23 2014

Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		2				2			2	
1:00 AM		0				0			0	
2:00 AM		2				2			2	
3:00 AM		0				0			0	
4:00 AM		6				6			6	
5:00 AM		2				2			2	
6:00 AM		19				19			19	
7:00 AM		74				74			74	
8:00 AM		146				146			146	
9:00 AM		150				150			150	
10:00 AM		144				144			144	
11:00 AM		213				213			213	
12:00 PM		252				252			252	
1:00 PM		220				220			220	
2:00 PM		194				194			194	
3:00 PM		235				235			235	
4:00 PM		183				183			183	
5:00 PM		170				170			170	
6:00 PM		174				174			174	
7:00 PM		82				82			82	
8:00 PM		83				83			83	
9:00 PM		48				48			48	
10:00 PM		12				12			12	
11:00 PM		7				7			7	
Day Total		2418				2418			2418	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		11:00 AM				11:00 AM			11:00 AM	
Volume		213				213			213	
PM Peak		12:00 PM				12:00 PM			12:00 PM	
Volume		252				252			252	

Comments: none

LOCATION: 27. Encinal Ave SPECIFIC LOCATION: 100 ft from El Camino Real CITY/STATE: Menlo Park, CA						QC JOB #: 12899418 DIRECTION: EB/WB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				14		14			14	
1:00 AM				15		15			15	
2:00 AM				10		10			10	
3:00 AM				5		5			5	
4:00 AM				6		6			6	
5:00 AM				32		32			32	
6:00 AM				84		84			84	
7:00 AM				441		441			441	
8:00 AM				552		552			552	
9:00 AM				391		391			391	
10:00 AM				275		275			275	
11:00 AM				308		308			308	
12:00 PM				350		350			350	
1:00 PM				387		387			387	
2:00 PM				367		367			367	
3:00 PM				445		445			445	
4:00 PM				495		495			495	
5:00 PM				530		530			530	
6:00 PM				342		342			342	
7:00 PM				208		208			208	
8:00 PM				136		136			136	
9:00 PM				109		109			109	
10:00 PM				70		70			70	
11:00 PM				25		25			25	
Day Total				5597		5597			5597	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak Volume				8:00 AM 552		8:00 AM 552			8:00 AM 552	
PM Peak Volume				5:00 PM 530		5:00 PM 530			5:00 PM 530	
<i>Comments:</i> none										

LOCATION: 28. Encinal Ave SPECIFIC LOCATION: 100 ft from Laurel St CITY/STATE: Menlo Park, CA						QC JOB #: 12899419 DIRECTION: EB/WB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				11		11			11	
1:00 AM				11		11			11	
2:00 AM				9		9			9	
3:00 AM				5		5			5	
4:00 AM				8		8			8	
5:00 AM				26		26			26	
6:00 AM				76		76			76	
7:00 AM				318		318			318	
8:00 AM				557		557			557	
9:00 AM				311		311			311	
10:00 AM				271		271			271	
11:00 AM				268		268			268	
12:00 PM				279		279			279	
1:00 PM				417		417			417	
2:00 PM				331		331			331	
3:00 PM				415		415			415	
4:00 PM				387		387			387	
5:00 PM				434		434			434	
6:00 PM				316		316			316	
7:00 PM				176		176			176	
8:00 PM				133		133			133	
9:00 PM				102		102			102	
10:00 PM				65		65			65	
11:00 PM				23		23			23	
Day Total				4949		4949			4949	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak Volume				8:00 AM 557		8:00 AM 557			8:00 AM 557	
PM Peak Volume				5:00 PM 434		5:00 PM 434			5:00 PM 434	
<i>Comments:</i> none										

LOCATION: 29. Glenwood Ave **QC JOB #:** 12899420
SPECIFIC LOCATION: 100 ft from El Camino Real **DIRECTION:** EB/WB
CITY/STATE: Menlo Park, CA **DATE:** Oct 16 2014 - Oct 16 2014

Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				17		17			17	
1:00 AM				11		11			11	
2:00 AM				5		5			5	
3:00 AM				5		5			5	
4:00 AM				9		9			9	
5:00 AM				54		54			54	
6:00 AM				127		127			127	
7:00 AM				445		445			445	
8:00 AM				470		470			470	
9:00 AM				423		423			423	
10:00 AM				337		337			337	
11:00 AM				352		352			352	
12:00 PM				356		356			356	
1:00 PM				371		371			371	
2:00 PM				479		479			479	
3:00 PM				496		496			496	
4:00 PM				467		467			467	
5:00 PM				522		522			522	
6:00 PM				389		389			389	
7:00 PM				240		240			240	
8:00 PM				148		148			148	
9:00 PM				136		136			136	
10:00 PM				69		69			69	
11:00 PM				51		51			51	
Day Total				5979		5979			5979	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak Volume				8:00 AM 470		8:00 AM 470			8:00 AM 470	
PM Peak Volume				5:00 PM 522		5:00 PM 522			5:00 PM 522	

Comments: none

LOCATION: 31. Haven Avenue SPECIFIC LOCATION: 100 ft from Bayfront Expy/Marsh Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899422 DIRECTION: NB/SB DATE: Oct 02 2014 - Oct 02 2014				
Start Time	Mon	Tue	Wed	Thu 02-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				29		29			29	
1:00 AM				20		20			20	
2:00 AM				13		13			13	
3:00 AM				25		25			25	
4:00 AM				32		32			32	
5:00 AM				98		98			98	
6:00 AM				294		294			294	
7:00 AM				543		543			543	
8:00 AM				713		713			713	
9:00 AM				595		595			595	
10:00 AM				363		363			363	
11:00 AM				421		421			421	
12:00 PM				421		421			421	
1:00 PM				365		365			365	
2:00 PM				441		441			441	
3:00 PM				495		495			495	
4:00 PM				666		666			666	
5:00 PM				704		704			704	
6:00 PM				518		518			518	
7:00 PM				274		274			274	
8:00 PM				151		151			151	
9:00 PM				109		109			109	
10:00 PM				65		65			65	
11:00 PM				42		42			42	
Day Total				7397		7397			7397	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak Volume				8:00 AM 713		8:00 AM 713			8:00 AM 713	
PM Peak Volume				5:00 PM 704		5:00 PM 704			5:00 PM 704	
<i>Comments:</i> none										

LOCATION: 35. Junipero Serra Blvd **QC JOB #:** 12899423
SPECIFIC LOCATION: 100 ft from City Limit **DIRECTION:** NB/SB
CITY/STATE: Menlo Park, CA **DATE:** Oct 16 2014 - Oct 16 2014

Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				39		39			39	
1:00 AM				21		21			21	
2:00 AM				13		13			13	
3:00 AM				34		34			34	
4:00 AM				143		143			143	
5:00 AM				480		480			480	
6:00 AM				1094		1094			1094	
7:00 AM				1503		1503			1503	
8:00 AM				1262		1262			1262	
9:00 AM				925		925			925	
10:00 AM				872		872			872	
11:00 AM				841		841			841	
12:00 PM				878		878			878	
1:00 PM				951		951			951	
2:00 PM				1114		1114			1114	
3:00 PM				1295		1295			1295	
4:00 PM				1394		1394			1394	
5:00 PM				1109		1109			1109	
6:00 PM				644		644			644	
7:00 PM				488		488			488	
8:00 PM				399		399			399	
9:00 PM				201		201			201	
10:00 PM				180		180			180	
11:00 PM				130		130			130	
Day Total				16010		16010			16010	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				7:00 AM		7:00 AM			7:00 AM	
Volume				1503		1503			1503	
PM Peak				4:00 PM		4:00 PM			4:00 PM	
Volume				1394		1394			1394	

Comments: none

LOCATION: 36. Laurel St SPECIFIC LOCATION: 100 ft from Oak Grove Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899424 DIRECTION: NB/SB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				4		4			4	
1:00 AM				3		3			3	
2:00 AM				2		2			2	
3:00 AM				1		1			1	
4:00 AM				7		7			7	
5:00 AM				26		26			26	
6:00 AM				38		38			38	
7:00 AM				346		346			346	
8:00 AM				457		457			457	
9:00 AM				333		333			333	
10:00 AM				189		189			189	
11:00 AM				205		205			205	
12:00 PM				200		200			200	
1:00 PM				262		262			262	
2:00 PM				252		252			252	
3:00 PM				390		390			390	
4:00 PM				377		377			377	
5:00 PM				474		474			474	
6:00 PM				249		249			249	
7:00 PM				110		110			110	
8:00 PM				49		49			49	
9:00 PM				50		50			50	
10:00 PM				23		23			23	
11:00 PM				8		8			8	
Day Total				4055		4055			4055	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				457		457			457	
PM Peak				5:00 PM		5:00 PM			5:00 PM	
Volume				474		474			474	
<i>Comments:</i> none										

LOCATION: 37. Laurel St SPECIFIC LOCATION: 100 ft from Ravenswood Ave CITY/STATE: Menlo Park, CA							QC JOB #: 12899425 DIRECTION: NB/SB DATE: Sep 24 2014 - Sep 24 2014			
Start Time	Mon	Tue	Wed 24-Sep-14	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM			10			10			10	
1:00 AM			2			2			2	
2:00 AM			2			2			2	
3:00 AM			0			0			0	
4:00 AM			8			8			8	
5:00 AM			27			27			27	
6:00 AM			76			76			76	
7:00 AM			327			327			327	
8:00 AM			504			504			504	
9:00 AM			303			303			303	
10:00 AM			197			197			197	
11:00 AM			208			208			208	
12:00 PM			230			230			230	
1:00 PM			311			311			311	
2:00 PM			282			282			282	
3:00 PM			386			386			386	
4:00 PM			414			414			414	
5:00 PM			507			507			507	
6:00 PM			290			290			290	
7:00 PM			156			156			156	
8:00 PM			69			69			69	
9:00 PM			56			56			56	
10:00 PM			32			32			32	
11:00 PM			11			11			11	
Day Total			4408			4408			4408	
% Weekday Average			100.0%							
% Week Average			100.0%			100.0%				
AM Peak			8:00 AM			8:00 AM			8:00 AM	
Volume			504			504			504	
PM Peak			5:00 PM			5:00 PM			5:00 PM	
Volume			507			507			507	
<i>Comments:</i> none										

LOCATION: 38. Laurel St SPECIFIC LOCATION: 100 ft from Willow Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899426 DIRECTION: NB/SB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				5		5			5	
1:00 AM				4		4			4	
2:00 AM				3		3			3	
3:00 AM				4		4			4	
4:00 AM				5		5			5	
5:00 AM				45		45			45	
6:00 AM				61		61			61	
7:00 AM				280		280			280	
8:00 AM				420		420			420	
9:00 AM				280		280			280	
10:00 AM				220		220			220	
11:00 AM				260		260			260	
12:00 PM				263		263			263	
1:00 PM				292		292			292	
2:00 PM				304		304			304	
3:00 PM				393		393			393	
4:00 PM				462		462			462	
5:00 PM				547		547			547	
6:00 PM				314		314			314	
7:00 PM				151		151			151	
8:00 PM				81		81			81	
9:00 PM				41		41			41	
10:00 PM				26		26			26	
11:00 PM				10		10			10	
Day Total				4471		4471			4471	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				420		420			420	
PM Peak				5:00 PM		5:00 PM			5:00 PM	
Volume				547		547			547	
<i>Comments:</i> none										

LOCATION: 39. Marsh Rd SPECIFIC LOCATION: 100 ft from City Limit CITY/STATE: Menlo Park, CA						QC JOB #: 12899427 DIRECTION: EB/WB DATE: Oct 02 2014 - Oct 02 2014				
Start Time	Mon	Tue	Wed	Thu 02-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				104		104			104	
1:00 AM				52		52			52	
2:00 AM				47		47			47	
3:00 AM				48		48			48	
4:00 AM				75		75			75	
5:00 AM				269		269			269	
6:00 AM				704		704			704	
7:00 AM				1443		1443			1443	
8:00 AM				1577		1577			1577	
9:00 AM				1467		1467			1467	
10:00 AM				1293		1293			1293	
11:00 AM				1449		1449			1449	
12:00 PM				1554		1554			1554	
1:00 PM				1533		1533			1533	
2:00 PM				1470		1470			1470	
3:00 PM				1693		1693			1693	
4:00 PM				1607		1607			1607	
5:00 PM				1656		1656			1656	
6:00 PM				1523		1523			1523	
7:00 PM				1096		1096			1096	
8:00 PM				840		840			840	
9:00 PM				646		646			646	
10:00 PM				464		464			464	
11:00 PM				235		235			235	
Day Total				22845		22845			22845	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				1577		1577			1577	
PM Peak				3:00 PM		3:00 PM			3:00 PM	
Volume				1693		1693			1693	
<i>Comments:</i> none										

LOCATION: 40. Marsh Rd SPECIFIC LOCATION: 100 ft from Bay Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899428 DIRECTION: EB/WB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue 21-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		102				102			102	
1:00 AM		61				61			61	
2:00 AM		32				32			32	
3:00 AM		38				38			38	
4:00 AM		100				100			100	
5:00 AM		303				303			303	
6:00 AM		798				798			798	
7:00 AM		1885				1885			1885	
8:00 AM		2150				2150			2150	
9:00 AM		1652				1652			1652	
10:00 AM		1448				1448			1448	
11:00 AM		1522				1522			1522	
12:00 PM		1575				1575			1575	
1:00 PM		1600				1600			1600	
2:00 PM		1690				1690			1690	
3:00 PM		2024				2024			2024	
4:00 PM		1807				1807			1807	
5:00 PM		1703				1703			1703	
6:00 PM		1763				1763			1763	
7:00 PM		1274				1274			1274	
8:00 PM		917				917			917	
9:00 PM		740				740			740	
10:00 PM		400				400			400	
11:00 PM		244				244			244	
Day Total		25828				25828			25828	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		2150				2150			2150	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		2024				2024			2024	
<i>Comments:</i> none										

LOCATION: 41. Marsh Rd SPECIFIC LOCATION: 100 ft from Bohannon Dr CITY/STATE: Menlo Park, CA						QC JOB #: 12899429 DIRECTION: NB/SB DATE: Oct 14 2014 - Oct 14 2014				
Start Time	Mon	Tue 14-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		166				166			166	
1:00 AM		86				86			86	
2:00 AM		72				72			72	
3:00 AM		95				95			95	
4:00 AM		191				191			191	
5:00 AM		587				587			587	
6:00 AM		1287				1287			1287	
7:00 AM		2475				2475			2475	
8:00 AM		2432				2432			2432	
9:00 AM		2174				2174			2174	
10:00 AM		1763				1763			1763	
11:00 AM		1782				1782			1782	
12:00 PM		1843				1843			1843	
1:00 PM		1936				1936			1936	
2:00 PM		2049				2049			2049	
3:00 PM		2361				2361			2361	
4:00 PM		2192				2192			2192	
5:00 PM		1963				1963			1963	
6:00 PM		1988				1988			1988	
7:00 PM		1690				1690			1690	
8:00 PM		1206				1206			1206	
9:00 PM		1059				1059			1059	
10:00 PM		612				612			612	
11:00 PM		399				399			399	
Day Total		32408				32408			32408	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		7:00 AM				7:00 AM			7:00 AM	
Volume		2475				2475			2475	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		2361				2361			2361	
<i>Comments:</i> none										

LOCATION: 44. Menlo Ave SPECIFIC LOCATION: 100 ft from Crane St CITY/STATE: Menlo Park, CA						QC JOB #: 12899431 DIRECTION: EB/WB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		12				12			12	
1:00 AM		8				8			8	
2:00 AM		9				9			9	
3:00 AM		9				9			9	
4:00 AM		19				19			19	
5:00 AM		72				72			72	
6:00 AM		148				148			148	
7:00 AM		443				443			443	
8:00 AM		695				695			695	
9:00 AM		623				623			623	
10:00 AM		460				460			460	
11:00 AM		510				510			510	
12:00 PM		523				523			523	
1:00 PM		538				538			538	
2:00 PM		575				575			575	
3:00 PM		736				736			736	
4:00 PM		707				707			707	
5:00 PM		784				784			784	
6:00 PM		702				702			702	
7:00 PM		515				515			515	
8:00 PM		311				311			311	
9:00 PM		152				152			152	
10:00 PM		68				68			68	
11:00 PM		28				28			28	
Day Total		8647				8647			8647	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		695				695			695	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		784				784			784	
<i>Comments:</i> none										

LOCATION: 45. Middle Ave SPECIFIC LOCATION: 100 ft from Olive St CITY/STATE: Menlo Park, CA						QC JOB #: 12899432 DIRECTION: EB/WB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		15				15			15	
1:00 AM		4				4			4	
2:00 AM		0				0			0	
3:00 AM		1				1			1	
4:00 AM		5				5			5	
5:00 AM		34				34			34	
6:00 AM		109				109			109	
7:00 AM		477				477			477	
8:00 AM		707				707			707	
9:00 AM		514				514			514	
10:00 AM		423				423			423	
11:00 AM		426				426			426	
12:00 PM		382				382			382	
1:00 PM		431				431			431	
2:00 PM		507				507			507	
3:00 PM		697				697			697	
4:00 PM		552				552			552	
5:00 PM		657				657			657	
6:00 PM		547				547			547	
7:00 PM		333				333			333	
8:00 PM		222				222			222	
9:00 PM		119				119			119	
10:00 PM		65				65			65	
11:00 PM		22				22			22	
Day Total		7249				7249			7249	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		707				707			707	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		697				697			697	
<i>Comments:</i> none										

LOCATION: 46. Middle Ave SPECIFIC LOCATION: 100 ft from University Dr CITY/STATE: Menlo Park, CA						QC JOB #: 12899433 DIRECTION: EB/WB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue 21-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		17				17			17	
1:00 AM		15				15			15	
2:00 AM		1				1			1	
3:00 AM		5				5			5	
4:00 AM		16				16			16	
5:00 AM		39				39			39	
6:00 AM		150				150			150	
7:00 AM		413				413			413	
8:00 AM		813				813			813	
9:00 AM		577				577			577	
10:00 AM		537				537			537	
11:00 AM		438				438			438	
12:00 PM		584				584			584	
1:00 PM		629				629			629	
2:00 PM		590				590			590	
3:00 PM		709				709			709	
4:00 PM		763				763			763	
5:00 PM		745				745			745	
6:00 PM		686				686			686	
7:00 PM		506				506			506	
8:00 PM		300				300			300	
9:00 PM		213				213			213	
10:00 PM		112				112			112	
11:00 PM		58				58			58	
Day Total		8916				8916			8916	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		813				813			813	
PM Peak		4:00 PM				4:00 PM			4:00 PM	
Volume		763				763			763	
<i>Comments:</i> none										

LOCATION: 47. Middlefield Rd **QC JOB #:** 12899434
SPECIFIC LOCATION: 100 ft from Ravenswood Ave **DIRECTION:** NB/SB
CITY/STATE: Menlo Park, CA **DATE:** Sep 30 2014 - Sep 30 2014

Start Time	Mon	Tue 30-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		38				38			38	
1:00 AM		14				14			14	
2:00 AM		17				17			17	
3:00 AM		15				15			15	
4:00 AM		31				31			31	
5:00 AM		118				118			118	
6:00 AM		313				313			313	
7:00 AM		840				840			840	
8:00 AM		1038				1038			1038	
9:00 AM		832				832			832	
10:00 AM		781				781			781	
11:00 AM		895				895			895	
12:00 PM		1007				1007			1007	
1:00 PM		956				956			956	
2:00 PM		985				985			985	
3:00 PM		1204				1204			1204	
4:00 PM		1337				1337			1337	
5:00 PM		1498				1498			1498	
6:00 PM		1086				1086			1086	
7:00 PM		699				699			699	
8:00 PM		421				421			421	
9:00 PM		310				310			310	
10:00 PM		206				206			206	
11:00 PM		116				116			116	
Day Total		14757				14757			14757	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		1038				1038			1038	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		1498				1498			1498	

Comments: none

LOCATION: 48. Middlefield Rd **QC JOB #:** 12899435
SPECIFIC LOCATION: 100 ft from Willow Rd **DIRECTION:** NB/SB
CITY/STATE: Menlo Park, CA **DATE:** Sep 30 2014 - Sep 30 2014

Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
	30-Sep-14									
12:00 AM		71				71			71	
1:00 AM		26				26			26	
2:00 AM		31				31			31	
3:00 AM		37				37			37	
4:00 AM		94				94			94	
5:00 AM		247				247			247	
6:00 AM		631				631			631	
7:00 AM		1206				1206			1206	
8:00 AM		1491				1491			1491	
9:00 AM		1204				1204			1204	
10:00 AM		1068				1068			1068	
11:00 AM		1157				1157			1157	
12:00 PM		1248				1248			1248	
1:00 PM		1249				1249			1249	
2:00 PM		1278				1278			1278	
3:00 PM		1431				1431			1431	
4:00 PM		1466				1466			1466	
5:00 PM		1543				1543			1543	
6:00 PM		1415				1415			1415	
7:00 PM		1022				1022			1022	
8:00 PM		683				683			683	
9:00 PM		524				524			524	
10:00 PM		371				371			371	
11:00 PM		191				191			191	
Day Total		19684				19684			19684	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak Volume		8:00 AM 1491				8:00 AM 1491			8:00 AM 1491	
PM Peak Volume		5:00 PM 1543				5:00 PM 1543			5:00 PM 1543	

Comments: none

LOCATION: 49. Middlefield Rd SPECIFIC LOCATION: 100 ft from City Limits CITY/STATE: Menlo Park, CA						QC JOB #: 12899436 DIRECTION: NB/SB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
	21-Oct-14									
12:00 AM		73				73			73	
1:00 AM		47				47			47	
2:00 AM		18				18			18	
3:00 AM		17				17			17	
4:00 AM		57				57			57	
5:00 AM		214				214			214	
6:00 AM		580				580			580	
7:00 AM		1047				1047			1047	
8:00 AM		1382				1382			1382	
9:00 AM		1204				1204			1204	
10:00 AM		1072				1072			1072	
11:00 AM		1088				1088			1088	
12:00 PM		1182				1182			1182	
1:00 PM		1123				1123			1123	
2:00 PM		1303				1303			1303	
3:00 PM		1208				1208			1208	
4:00 PM		1453				1453			1453	
5:00 PM		1163				1163			1163	
6:00 PM		1073				1073			1073	
7:00 PM		1076				1076			1076	
8:00 PM		767				767			767	
9:00 PM		655				655			655	
10:00 PM		386				386			386	
11:00 PM		228				228			228	
Day Total		18416				18416			18416	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		1382				1382			1382	
PM Peak		4:00 PM				4:00 PM			4:00 PM	
Volume		1453				1453			1453	
<i>Comments:</i> none										

LOCATION: 50. Newbridge St SPECIFIC LOCATION: 100 ft from Willow Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899437 DIRECTION: NB/SB DATE: Oct 08 2014 - Oct 08 2014				
Start Time	Mon	Tue	Wed 08-Oct-14	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM			51			51			51	
1:00 AM			30			30			30	
2:00 AM			33			33			33	
3:00 AM			21			21			21	
4:00 AM			37			37			37	
5:00 AM			95			95			95	
6:00 AM			270			270			270	
7:00 AM			431			431			431	
8:00 AM			488			488			488	
9:00 AM			295			295			295	
10:00 AM			278			278			278	
11:00 AM			297			297			297	
12:00 PM			336			336			336	
1:00 PM			426			426			426	
2:00 PM			367			367			367	
3:00 PM			509			509			509	
4:00 PM			541			541			541	
5:00 PM			582			582			582	
6:00 PM			600			600			600	
7:00 PM			492			492			492	
8:00 PM			353			353			353	
9:00 PM			252			252			252	
10:00 PM			188			188			188	
11:00 PM			93			93			93	
Day Total			7065			7065			7065	
% Weekday Average			100.0%							
% Week Average			100.0%			100.0%				
AM Peak			8:00 AM			8:00 AM			8:00 AM	
Volume			488			488			488	
PM Peak			6:00 PM			6:00 PM			6:00 PM	
Volume			600			600			600	
<i>Comments:</i> none										

LOCATION: 51. Newbridge St SPECIFIC LOCATION: 100 ft from Willow Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899438 DIRECTION: NB/SB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue 21-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		47				47			47	
1:00 AM		41				41			41	
2:00 AM		29				29			29	
3:00 AM		24				24			24	
4:00 AM		34				34			34	
5:00 AM		108				108			108	
6:00 AM		247				247			247	
7:00 AM		525				525			525	
8:00 AM		555				555			555	
9:00 AM		408				408			408	
10:00 AM		318				318			318	
11:00 AM		384				384			384	
12:00 PM		413				413			413	
1:00 PM		422				422			422	
2:00 PM		555				555			555	
3:00 PM		667				667			667	
4:00 PM		720				720			720	
5:00 PM		733				733			733	
6:00 PM		741				741			741	
7:00 PM		608				608			608	
8:00 PM		418				418			418	
9:00 PM		352				352			352	
10:00 PM		181				181			181	
11:00 PM		96				96			96	
Day Total		8626				8626			8626	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		555				555			555	
PM Peak		6:00 PM				6:00 PM			6:00 PM	
Volume		741				741			741	
Comments: none										

LOCATION: 52. Oak Grove Ave SPECIFIC LOCATION: 100 ft from University Dr CITY/STATE: Menlo Park, CA						QC JOB #: 12899439 DIRECTION: EB/WB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		10				10			10	
1:00 AM		4				4			4	
2:00 AM		2				2			2	
3:00 AM		9				9			9	
4:00 AM		10				10			10	
5:00 AM		24				24			24	
6:00 AM		88				88			88	
7:00 AM		422				422			422	
8:00 AM		493				493			493	
9:00 AM		432				432			432	
10:00 AM		412				412			412	
11:00 AM		400				400			400	
12:00 PM		442				442			442	
1:00 PM		417				417			417	
2:00 PM		463				463			463	
3:00 PM		635				635			635	
4:00 PM		576				576			576	
5:00 PM		512				512			512	
6:00 PM		439				439			439	
7:00 PM		237				237			237	
8:00 PM		175				175			175	
9:00 PM		79				79			79	
10:00 PM		50				50			50	
11:00 PM		20				20			20	
Day Total		6351				6351			6351	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		493				493			493	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		635				635			635	
<i>Comments:</i> none										

LOCATION: 53. Oak Grove Ave SPECIFIC LOCATION: 100 ft from Crane St CITY/STATE: Menlo Park, CA						QC JOB #: 12899440 DIRECTION: EB/WB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		12				12			12	
1:00 AM		6				6			6	
2:00 AM		3				3			3	
3:00 AM		10				10			10	
4:00 AM		17				17			17	
5:00 AM		38				38			38	
6:00 AM		117				117			117	
7:00 AM		442				442			442	
8:00 AM		620				620			620	
9:00 AM		527				527			527	
10:00 AM		503				503			503	
11:00 AM		496				496			496	
12:00 PM		545				545			545	
1:00 PM		524				524			524	
2:00 PM		566				566			566	
3:00 PM		731				731			731	
4:00 PM		678				678			678	
5:00 PM		627				627			627	
6:00 PM		537				537			537	
7:00 PM		290				290			290	
8:00 PM		197				197			197	
9:00 PM		120				120			120	
10:00 PM		59				59			59	
11:00 PM		32				32			32	
Day Total		7697				7697			7697	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		620				620			620	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		731				731			731	
<i>Comments:</i> none										

LOCATION: 54. Oak Grove Ave **QC JOB #:** 12899441
SPECIFIC LOCATION: 100 ft from El Camino Real **DIRECTION:** EB/WB
CITY/STATE: Menlo Park, CA **DATE:** Oct 21 2014 - Oct 21 2014

Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday	Sat	Sun	Average Week	Average Week Profile
	21-Oct-14					Hourly Traffic			Hourly Traffic	
12:00 AM		22				22			22	
1:00 AM		15				15			15	
2:00 AM		5				5			5	
3:00 AM		7				7			7	
4:00 AM		11				11			11	
5:00 AM		77				77			77	
6:00 AM		181				181			181	
7:00 AM		580				580			580	
8:00 AM		886				886			886	
9:00 AM		606				606			606	
10:00 AM		544				544			544	
11:00 AM		584				584			584	
12:00 PM		631				631			631	
1:00 PM		653				653			653	
2:00 PM		654				654			654	
3:00 PM		868				868			868	
4:00 PM		777				777			777	
5:00 PM		784				784			784	
6:00 PM		595				595			595	
7:00 PM		441				441			441	
8:00 PM		263				263			263	
9:00 PM		229				229			229	
10:00 PM		91				91			91	
11:00 PM		66				66			66	
Day Total		9570				9570			9570	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		886				886			886	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		868				868			868	

Comments: none

LOCATION: 55. Oak Grove Ave SPECIFIC LOCATION: 100 ft from Laurel St CITY/STATE: Menlo Park, CA						QC JOB #: 12899442 DIRECTION: EB/WB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				28		28			28	
1:00 AM				16		16			16	
2:00 AM				6		6			6	
3:00 AM				5		5			5	
4:00 AM				20		20			20	
5:00 AM				74		74			74	
6:00 AM				165		165			165	
7:00 AM				584		584			584	
8:00 AM				645		645			645	
9:00 AM				713		713			713	
10:00 AM				487		487			487	
11:00 AM				506		506			506	
12:00 PM				583		583			583	
1:00 PM				595		595			595	
2:00 PM				604		604			604	
3:00 PM				792		792			792	
4:00 PM				630		630			630	
5:00 PM				655		655			655	
6:00 PM				531		531			531	
7:00 PM				392		392			392	
8:00 PM				222		222			222	
9:00 PM				222		222			222	
10:00 PM				110		110			110	
11:00 PM				66		66			66	
Day Total				8651		8651			8651	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak Volume				9:00 AM 713		9:00 AM 713			9:00 AM 713	
PM Peak Volume				3:00 PM 792		3:00 PM 792			3:00 PM 792	
<i>Comments:</i> none										

LOCATION: 56. O'Brien Dr SPECIFIC LOCATION: 100 ft from Kavanaugh Dr CITY/STATE: Menlo Park, CA							QC JOB #: 12899443 DIRECTION: NB/SB DATE: Oct 08 2014 - Oct 08 2014			
Start Time	Mon	Tue	Wed 08-Oct-14	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM			36			36			36	
1:00 AM			15			15			15	
2:00 AM			7			7			7	
3:00 AM			19			19			19	
4:00 AM			35			35			35	
5:00 AM			177			177			177	
6:00 AM			265			265			265	
7:00 AM			504			504			504	
8:00 AM			646			646			646	
9:00 AM			318			318			318	
10:00 AM			362			362			362	
11:00 AM			326			326			326	
12:00 PM			369			369			369	
1:00 PM			347			347			347	
2:00 PM			447			447			447	
3:00 PM			421			421			421	
4:00 PM			493			493			493	
5:00 PM			447			447			447	
6:00 PM			460			460			460	
7:00 PM			252			252			252	
8:00 PM			198			198			198	
9:00 PM			124			124			124	
10:00 PM			68			68			68	
11:00 PM			38			38			38	
Day Total			6374			6374			6374	
% Weekday Average			100.0%							
% Week Average			100.0%			100.0%				
AM Peak			8:00 AM			8:00 AM			8:00 AM	
Volume			646			646			646	
PM Peak			4:00 PM			4:00 PM			4:00 PM	
Volume			493			493			493	
<i>Comments:</i> none										

LOCATION: 57. O'Brien Dr SPECIFIC LOCATION: 100 ft from University Ave CITY/STATE: Menlo Park, CA							QC JOB #: 12899444 DIRECTION: NB/SB DATE: Oct 08 2014 - Oct 08 2014			
Start Time	Mon	Tue	Wed 08-Oct-14	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM			19			19			19	
1:00 AM			7			7			7	
2:00 AM			3			3			3	
3:00 AM			19			19			19	
4:00 AM			27			27			27	
5:00 AM			96			96			96	
6:00 AM			129			129			129	
7:00 AM			294			294			294	
8:00 AM			367			367			367	
9:00 AM			194			194			194	
10:00 AM			195			195			195	
11:00 AM			188			188			188	
12:00 PM			168			168			168	
1:00 PM			175			175			175	
2:00 PM			190			190			190	
3:00 PM			199			199			199	
4:00 PM			245			245			245	
5:00 PM			269			269			269	
6:00 PM			220			220			220	
7:00 PM			130			130			130	
8:00 PM			70			70			70	
9:00 PM			39			39			39	
10:00 PM			24			24			24	
11:00 PM			12			12			12	
Day Total			3279			3279			3279	
% Weekday Average			100.0%							
% Week Average			100.0%			100.0%				
AM Peak			8:00 AM			8:00 AM			8:00 AM	
Volume			367			367			367	
PM Peak			5:00 PM			5:00 PM			5:00 PM	
Volume			269			269			269	
<i>Comments:</i> none										

LOCATION: 60. Ravenswood Ave SPECIFIC LOCATION: 100 ft from Alma St CITY/STATE: Menlo Park, CA						QC JOB #: 12899446 DIRECTION: EB/WB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				36		36			36	
1:00 AM				36		36			36	
2:00 AM				36		36			36	
3:00 AM				36		36			36	
4:00 AM				71		71			71	
5:00 AM				206		206			206	
6:00 AM				500		500			500	
7:00 AM				1079		1079			1079	
8:00 AM				1242		1242			1242	
9:00 AM				1328		1328			1328	
10:00 AM				1094		1094			1094	
11:00 AM				1119		1119			1119	
12:00 PM				1219		1219			1219	
1:00 PM				1156		1156			1156	
2:00 PM				1269		1269			1269	
3:00 PM				1419		1419			1419	
4:00 PM				1455		1455			1455	
5:00 PM				1651		1651			1651	
6:00 PM				1363		1363			1363	
7:00 PM				877		877			877	
8:00 PM				648		648			648	
9:00 PM				481		481			481	
10:00 PM				276		276			276	
11:00 PM				165		165			165	
Day Total				18762		18762			18762	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				9:00 AM		9:00 AM			9:00 AM	
Volume				1328		1328			1328	
PM Peak				5:00 PM		5:00 PM			5:00 PM	
Volume				1651		1651			1651	
<i>Comments:</i> none										

LOCATION: 61. Ravenswood Ave SPECIFIC LOCATION: 100 ft from Laurel St CITY/STATE: Menlo Park, CA						QC JOB #: 12899447 DIRECTION: EB/WB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				38		38			38	
1:00 AM				39		39			39	
2:00 AM				38		38			38	
3:00 AM				38		38			38	
4:00 AM				68		68			68	
5:00 AM				192		192			192	
6:00 AM				484		484			484	
7:00 AM				914		914			914	
8:00 AM				1070		1070			1070	
9:00 AM				1195		1195			1195	
10:00 AM				953		953			953	
11:00 AM				952		952			952	
12:00 PM				1091		1091			1091	
1:00 PM				986		986			986	
2:00 PM				1144		1144			1144	
3:00 PM				1247		1247			1247	
4:00 PM				1250		1250			1250	
5:00 PM				1361		1361			1361	
6:00 PM				1192		1192			1192	
7:00 PM				793		793			793	
8:00 PM				594		594			594	
9:00 PM				477		477			477	
10:00 PM				276		276			276	
11:00 PM				161		161			161	
Day Total				16553		16553			16553	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				9:00 AM		9:00 AM			9:00 AM	
Volume				1195		1195			1195	
PM Peak				5:00 PM		5:00 PM			5:00 PM	
Volume				1361		1361			1361	
<i>Comments:</i> none										

LOCATION: 63. Sand Hill Rd SPECIFIC LOCATION: 100 ft from I-280 CITY/STATE: Menlo Park, CA						QC JOB #: 12899449 DIRECTION: EB/WB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue 21-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		80				80			80	
1:00 AM		39				39			39	
2:00 AM		16				16			16	
3:00 AM		28				28			28	
4:00 AM		87				87			87	
5:00 AM		334				334			334	
6:00 AM		1021				1021			1021	
7:00 AM		1933				1933			1933	
8:00 AM		2426				2426			2426	
9:00 AM		1994				1994			1994	
10:00 AM		1559				1559			1559	
11:00 AM		1773				1773			1773	
12:00 PM		1796				1796			1796	
1:00 PM		1818				1818			1818	
2:00 PM		1892				1892			1892	
3:00 PM		2236				2236			2236	
4:00 PM		2053				2053			2053	
5:00 PM		1706				1706			1706	
6:00 PM		1909				1909			1909	
7:00 PM		1259				1259			1259	
8:00 PM		862				862			862	
9:00 PM		676				676			676	
10:00 PM		361				361			361	
11:00 PM		190				190			190	
Day Total		28048				28048			28048	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		2426				2426			2426	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		2236				2236			2236	
<i>Comments:</i> none										

LOCATION: 64. Sand Hill Rd **QC JOB #:** 12899450
SPECIFIC LOCATION: 100 ft from Santa Cruz Ave **DIRECTION:** EB/WB
CITY/STATE: Menlo Park, CA **DATE:** Sep 25 2014 - Sep 25 2014

Start Time	Mon	Tue	Wed	Thu 25-Sep-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				118		118			118	
1:00 AM				49		49			49	
2:00 AM				27		27			27	
3:00 AM				36		36			36	
4:00 AM				89		89			89	
5:00 AM				335		335			335	
6:00 AM				975		975			975	
7:00 AM				1484		1484			1484	
8:00 AM				2500		2500			2500	
9:00 AM				2222		2222			2222	
10:00 AM				1750		1750			1750	
11:00 AM				1868		1868			1868	
12:00 PM				1885		1885			1885	
1:00 PM				1879		1879			1879	
2:00 PM				2102		2102			2102	
3:00 PM				2350		2350			2350	
4:00 PM				2488		2488			2488	
5:00 PM				2370		2370			2370	
6:00 PM				2083		2083			2083	
7:00 PM				1501		1501			1501	
8:00 PM				1054		1054			1054	
9:00 PM				857		857			857	
10:00 PM				467		467			467	
11:00 PM				296		296			296	
Day Total				30785		30785			30785	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				2500		2500			2500	
PM Peak				4:00 PM		4:00 PM			4:00 PM	
Volume				2488		2488			2488	

Comments: none

LOCATION: 65. Sand Hill Rd SPECIFIC LOCATION: 100 ft from Santa Cruz Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899451 DIRECTION: EB/WB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		114				114			114	
1:00 AM		51				51			51	
2:00 AM		41				41			41	
3:00 AM		41				41			41	
4:00 AM		112				112			112	
5:00 AM		446				446			446	
6:00 AM		1286				1286			1286	
7:00 AM		2101				2101			2101	
8:00 AM		2491				2491			2491	
9:00 AM		2326				2326			2326	
10:00 AM		1940				1940			1940	
11:00 AM		1934				1934			1934	
12:00 PM		2076				2076			2076	
1:00 PM		2006				2006			2006	
2:00 PM		2357				2357			2357	
3:00 PM		2395				2395			2395	
4:00 PM		2331				2331			2331	
5:00 PM		2334				2334			2334	
6:00 PM		2221				2221			2221	
7:00 PM		1570				1570			1570	
8:00 PM		1110				1110			1110	
9:00 PM		751				751			751	
10:00 PM		439				439			439	
11:00 PM		269				269			269	
Day Total		32742				32742			32742	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		2491				2491			2491	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		2395				2395			2395	
<i>Comments:</i> none										

LOCATION: 66. Santa Cruz Ave SPECIFIC LOCATION: 100 ft from Junipero Serra Blvd CITY/STATE: Menlo Park, CA						QC JOB #: 12899452 DIRECTION: NB/SB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue 21-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		81				81			81	
1:00 AM		41				41			41	
2:00 AM		17				17			17	
3:00 AM		30				30			30	
4:00 AM		60				60			60	
5:00 AM		304				304			304	
6:00 AM		790				790			790	
7:00 AM		1759				1759			1759	
8:00 AM		2032				2032			2032	
9:00 AM		1916				1916			1916	
10:00 AM		1663				1663			1663	
11:00 AM		1597				1597			1597	
12:00 PM		1648				1648			1648	
1:00 PM		1589				1589			1589	
2:00 PM		1859				1859			1859	
3:00 PM		2030				2030			2030	
4:00 PM		1950				1950			1950	
5:00 PM		2057				2057			2057	
6:00 PM		1906				1906			1906	
7:00 PM		1209				1209			1209	
8:00 PM		800				800			800	
9:00 PM		627				627			627	
10:00 PM		339				339			339	
11:00 PM		180				180			180	
Day Total		26484				26484			26484	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		2032				2032			2032	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		2057				2057			2057	
<i>Comments:</i> none										

LOCATION: 67. Santa Cruz Ave SPECIFIC LOCATION: 100 ft from Sand Hill Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899453 DIRECTION: NB/SB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				45		45			45	
1:00 AM				26		26			26	
2:00 AM				16		16			16	
3:00 AM				17		17			17	
4:00 AM				36		36			36	
5:00 AM				158		158			158	
6:00 AM				493		493			493	
7:00 AM				1651		1651			1651	
8:00 AM				1836		1836			1836	
9:00 AM				1573		1573			1573	
10:00 AM				1395		1395			1395	
11:00 AM				1385		1385			1385	
12:00 PM				1375		1375			1375	
1:00 PM				1450		1450			1450	
2:00 PM				1549		1549			1549	
3:00 PM				1964		1964			1964	
4:00 PM				1906		1906			1906	
5:00 PM				1999		1999			1999	
6:00 PM				1656		1656			1656	
7:00 PM				1021		1021			1021	
8:00 PM				634		634			634	
9:00 PM				528		528			528	
10:00 PM				306		306			306	
11:00 PM				208		208			208	
Day Total				23227		23227			23227	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				1836		1836			1836	
PM Peak				5:00 PM		5:00 PM			5:00 PM	
Volume				1999		1999			1999	
<i>Comments:</i> none										

LOCATION: 68. Santa Cruz Ave SPECIFIC LOCATION: 100 ft from Alameda de las Pulgas CITY/STATE: Menlo Park, CA						QC JOB #: 12899454 DIRECTION: NB/SB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		22				22			22	
1:00 AM		11				11			11	
2:00 AM		3				3			3	
3:00 AM		4				4			4	
4:00 AM		8				8			8	
5:00 AM		57				57			57	
6:00 AM		203				203			203	
7:00 AM		834				834			834	
8:00 AM		921				921			921	
9:00 AM		813				813			813	
10:00 AM		657				657			657	
11:00 AM		615				615			615	
12:00 PM		605				605			605	
1:00 PM		597				597			597	
2:00 PM		689				689			689	
3:00 PM		928				928			928	
4:00 PM		892				892			892	
5:00 PM		955				955			955	
6:00 PM		865				865			865	
7:00 PM		538				538			538	
8:00 PM		338				338			338	
9:00 PM		200				200			200	
10:00 PM		96				96			96	
11:00 PM		46				46			46	
Day Total		10897				10897			10897	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		921				921			921	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		955				955			955	
Comments: none										

LOCATION: 69. Santa Cruz Ave SPECIFIC LOCATION: 100 ft from Avy/Orange Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899455 DIRECTION: EB/WB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				16		16			16	
1:00 AM				12		12			12	
2:00 AM				5		5			5	
3:00 AM				7		7			7	
4:00 AM				17		17			17	
5:00 AM				76		76			76	
6:00 AM				260		260			260	
7:00 AM				881		881			881	
8:00 AM				1126		1126			1126	
9:00 AM				1046		1046			1046	
10:00 AM				929		929			929	
11:00 AM				872		872			872	
12:00 PM				980		980			980	
1:00 PM				1003		1003			1003	
2:00 PM				1012		1012			1012	
3:00 PM				1226		1226			1226	
4:00 PM				1190		1190			1190	
5:00 PM				1229		1229			1229	
6:00 PM				998		998			998	
7:00 PM				646		646			646	
8:00 PM				393		393			393	
9:00 PM				313		313			313	
10:00 PM				195		195			195	
11:00 PM				92		92			92	
Day Total				14524		14524			14524	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				1126		1126			1126	
PM Peak				5:00 PM		5:00 PM			5:00 PM	
Volume				1229		1229			1229	
<i>Comments:</i> none										

LOCATION: 70. Santa Cruz Ave SPECIFIC LOCATION: 100 ft from Olive St CITY/STATE: Menlo Park, CA						QC JOB #: 12899456 DIRECTION: EB/WB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				17		17			17	
1:00 AM				12		12			12	
2:00 AM				3		3			3	
3:00 AM				7		7			7	
4:00 AM				20		20			20	
5:00 AM				75		75			75	
6:00 AM				267		267			267	
7:00 AM				1000		1000			1000	
8:00 AM				1273		1273			1273	
9:00 AM				1105		1105			1105	
10:00 AM				930		930			930	
11:00 AM				955		955			955	
12:00 PM				1029		1029			1029	
1:00 PM				993		993			993	
2:00 PM				1125		1125			1125	
3:00 PM				1272		1272			1272	
4:00 PM				1293		1293			1293	
5:00 PM				1286		1286			1286	
6:00 PM				1049		1049			1049	
7:00 PM				642		642			642	
8:00 PM				408		408			408	
9:00 PM				301		301			301	
10:00 PM				172		172			172	
11:00 PM				80		80			80	
Day Total				15314		15314			15314	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				1273		1273			1273	
PM Peak				4:00 PM		4:00 PM			4:00 PM	
Volume				1293		1293			1293	
<i>Comments:</i> none										

LOCATION: 71. Santa Cruz Ave
SPECIFIC LOCATION: 100 ft from University Dr
CITY/STATE: Menlo Park, CA

QC JOB #: 12899457
DIRECTION: EB/WB
DATE: Oct 16 2014 - Oct 16 2014

Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				8		8			8	
1:00 AM				6		6			6	
2:00 AM				1		1			1	
3:00 AM				8		8			8	
4:00 AM				5		5			5	
5:00 AM				29		29			29	
6:00 AM				89		89			89	
7:00 AM				335		335			335	
8:00 AM				498		498			498	
9:00 AM				551		551			551	
10:00 AM				513		513			513	
11:00 AM				562		562			562	
12:00 PM				591		591			591	
1:00 PM				548		548			548	
2:00 PM				587		587			587	
3:00 PM				646		646			646	
4:00 PM				640		640			640	
5:00 PM				642		642			642	
6:00 PM				501		501			501	
7:00 PM				349		349			349	
8:00 PM				224		224			224	
9:00 PM				171		171			171	
10:00 PM				67		67			67	
11:00 PM				43		43			43	
Day Total				7614		7614			7614	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak Volume				11:00 AM 562		11:00 AM 562			11:00 AM 562	
PM Peak Volume				3:00 PM 646		3:00 PM 646			3:00 PM 646	

Comments: none

LOCATION: 72. Santa Cruz Ave SPECIFIC LOCATION: 100 ft from Crane St CITY/STATE: Menlo Park, CA						QC JOB #: 12899458 DIRECTION: EB/WB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		9				9			9	
1:00 AM		4				4			4	
2:00 AM		11				11			11	
3:00 AM		3				3			3	
4:00 AM		9				9			9	
5:00 AM		31				31			31	
6:00 AM		98				98			98	
7:00 AM		323				323			323	
8:00 AM		472				472			472	
9:00 AM		493				493			493	
10:00 AM		469				469			469	
11:00 AM		534				534			534	
12:00 PM		579				579			579	
1:00 PM		562				562			562	
2:00 PM		568				568			568	
3:00 PM		607				607			607	
4:00 PM		585				585			585	
5:00 PM		583				583			583	
6:00 PM		583				583			583	
7:00 PM		362				362			362	
8:00 PM		242				242			242	
9:00 PM		168				168			168	
10:00 PM		53				53			53	
11:00 PM		25				25			25	
Day Total		7373				7373			7373	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		11:00 AM				11:00 AM			11:00 AM	
Volume		534				534			534	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		607				607			607	
<i>Comments:</i> none										

LOCATION: 73. Scott Dr SPECIFIC LOCATION: 100 ft from Marsh Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899459 DIRECTION: NB/SB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue 21-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		11				11			11	
1:00 AM		7				7			7	
2:00 AM		8				8			8	
3:00 AM		19				19			19	
4:00 AM		96				96			96	
5:00 AM		146				146			146	
6:00 AM		209				209			209	
7:00 AM		381				381			381	
8:00 AM		522				522			522	
9:00 AM		455				455			455	
10:00 AM		225				225			225	
11:00 AM		197				197			197	
12:00 PM		223				223			223	
1:00 PM		317				317			317	
2:00 PM		315				315			315	
3:00 PM		261				261			261	
4:00 PM		359				359			359	
5:00 PM		486				486			486	
6:00 PM		268				268			268	
7:00 PM		119				119			119	
8:00 PM		84				84			84	
9:00 PM		33				33			33	
10:00 PM		49				49			49	
11:00 PM		25				25			25	
Day Total		4815				4815			4815	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		522				522			522	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		486				486			486	
<i>Comments:</i> none										

LOCATION: 74. Sharon Park Dr SPECIFIC LOCATION: 100 ft from Sand Hill Rd CITY/STATE: Menlo Park, CA							QC JOB #: 12899460 DIRECTION: NB/SB DATE: Sep 24 2014 - Sep 24 2014			
Start Time	Mon	Tue	Wed 24-Sep-14	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM			53			53			53	
1:00 AM			11			11			11	
2:00 AM			11			11			11	
3:00 AM			13			13			13	
4:00 AM			30			30			30	
5:00 AM			118			118			118	
6:00 AM			243			243			243	
7:00 AM			496			496			496	
8:00 AM			686			686			686	
9:00 AM			588			588			588	
10:00 AM			586			586			586	
11:00 AM			685			685			685	
12:00 PM			728			728			728	
1:00 PM			709			709			709	
2:00 PM			767			767			767	
3:00 PM			733			733			733	
4:00 PM			731			731			731	
5:00 PM			745			745			745	
6:00 PM			667			667			667	
7:00 PM			472			472			472	
8:00 PM			395			395			395	
9:00 PM			269			269			269	
10:00 PM			153			153			153	
11:00 PM			81			81			81	
Day Total			9970			9970			9970	
% Weekday Average			100.0%							
% Week Average			100.0%			100.0%				
AM Peak			8:00 AM			8:00 AM			8:00 AM	
Volume			686			686			686	
PM Peak			2:00 PM			2:00 PM			2:00 PM	
Volume			767			767			767	
<i>Comments:</i> none										

LOCATION: 75. Sharon Rd SPECIFIC LOCATION: 100 ft from Sharon Park Dr CITY/STATE: Menlo Park, CA						QC JOB #: 12899461 DIRECTION: NB/SB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		9				9			9	
1:00 AM		2				2			2	
2:00 AM		1				1			1	
3:00 AM		4				4			4	
4:00 AM		9				9			9	
5:00 AM		18				18			18	
6:00 AM		55				55			55	
7:00 AM		313				313			313	
8:00 AM		328				328			328	
9:00 AM		206				206			206	
10:00 AM		184				184			184	
11:00 AM		204				204			204	
12:00 PM		189				189			189	
1:00 PM		197				197			197	
2:00 PM		346				346			346	
3:00 PM		369				369			369	
4:00 PM		308				308			308	
5:00 PM		349				349			349	
6:00 PM		297				297			297	
7:00 PM		190				190			190	
8:00 PM		103				103			103	
9:00 PM		57				57			57	
10:00 PM		30				30			30	
11:00 PM		13				13			13	
Day Total		3781				3781			3781	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		328				328			328	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		369				369			369	
<i>Comments:</i> none										

LOCATION: 76. University Dr SPECIFIC LOCATION: 100 ft from Middle Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899462 DIRECTION: EB/WB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		11				11			11	
1:00 AM		7				7			7	
2:00 AM		8				8			8	
3:00 AM		2				2			2	
4:00 AM		3				3			3	
5:00 AM		21				21			21	
6:00 AM		81				81			81	
7:00 AM		294				294			294	
8:00 AM		431				431			431	
9:00 AM		362				362			362	
10:00 AM		295				295			295	
11:00 AM		377				377			377	
12:00 PM		381				381			381	
1:00 PM		369				369			369	
2:00 PM		386				386			386	
3:00 PM		480				480			480	
4:00 PM		521				521			521	
5:00 PM		651				651			651	
6:00 PM		510				510			510	
7:00 PM		275				275			275	
8:00 PM		192				192			192	
9:00 PM		96				96			96	
10:00 PM		57				57			57	
11:00 PM		30				30			30	
Day Total		5840				5840			5840	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		431				431			431	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		651				651			651	
<i>Comments:</i> none										

LOCATION: 77. University Dr SPECIFIC LOCATION: 100 ft from Menlo Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899463 DIRECTION: NB/SB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue 21-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		14				14			14	
1:00 AM		3				3			3	
2:00 AM		1				1			1	
3:00 AM		0				0			0	
4:00 AM		7				7			7	
5:00 AM		59				59			59	
6:00 AM		132				132			132	
7:00 AM		489				489			489	
8:00 AM		812				812			812	
9:00 AM		672				672			672	
10:00 AM		550				550			550	
11:00 AM		570				570			570	
12:00 PM		598				598			598	
1:00 PM		582				582			582	
2:00 PM		613				613			613	
3:00 PM		806				806			806	
4:00 PM		872				872			872	
5:00 PM		883				883			883	
6:00 PM		739				739			739	
7:00 PM		412				412			412	
8:00 PM		241				241			241	
9:00 PM		173				173			173	
10:00 PM		55				55			55	
11:00 PM		27				27			27	
Day Total		9310				9310			9310	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		812				812			812	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		883				883			883	
<i>Comments:</i> none										

LOCATION: 78. University Dr SPECIFIC LOCATION: 100 ft from Santa Cruz Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899464 DIRECTION: NB/SB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
		23-Sep-14								
12:00 AM		7				7			7	
1:00 AM		7				7			7	
2:00 AM		1				1			1	
3:00 AM		5				5			5	
4:00 AM		5				5			5	
5:00 AM		20				20			20	
6:00 AM		88				88			88	
7:00 AM		450				450			450	
8:00 AM		512				512			512	
9:00 AM		507				507			507	
10:00 AM		424				424			424	
11:00 AM		487				487			487	
12:00 PM		535				535			535	
1:00 PM		467				467			467	
2:00 PM		478				478			478	
3:00 PM		657				657			657	
4:00 PM		636				636			636	
5:00 PM		654				654			654	
6:00 PM		552				552			552	
7:00 PM		266				266			266	
8:00 PM		245				245			245	
9:00 PM		91				91			91	
10:00 PM		43				43			43	
11:00 PM		21				21			21	
Day Total		7158				7158			7158	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		512				512			512	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		657				657			657	
<i>Comments:</i> none										

LOCATION: 79. University Dr SPECIFIC LOCATION: 100 ft from Oak Grove Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899465 DIRECTION: NB/SB DATE: Oct 23 2014 - Oct 23 2014				
Start Time	Mon	Tue	Wed	Thu 23-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				6		6			6	
1:00 AM				2		2			2	
2:00 AM				3		3			3	
3:00 AM				2		2			2	
4:00 AM				2		2			2	
5:00 AM				19		19			19	
6:00 AM				53		53			53	
7:00 AM				272		272			272	
8:00 AM				371		371			371	
9:00 AM				340		340			340	
10:00 AM				284		284			284	
11:00 AM				351		351			351	
12:00 PM				366		366			366	
1:00 PM				331		331			331	
2:00 PM				389		389			389	
3:00 PM				506		506			506	
4:00 PM				488		488			488	
5:00 PM				499		499			499	
6:00 PM				365		365			365	
7:00 PM				251		251			251	
8:00 PM				106		106			106	
9:00 PM				69		69			69	
10:00 PM				24		24			24	
11:00 PM				12		12			12	
Day Total				5111		5111			5111	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak Volume				8:00 AM 371		8:00 AM 371			8:00 AM 371	
PM Peak Volume				3:00 PM 506		3:00 PM 506			3:00 PM 506	
<i>Comments:</i> none										

LOCATION: 84. Valparaiso Ave SPECIFIC LOCATION: 100 ft from Alameda de las Pulgas CITY/STATE: Menlo Park, CA						QC JOB #: 12899466 DIRECTION: EB/WB DATE: Oct 15 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed 15-Oct-14	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				20		20			20	
1:00 AM				19		19			19	
2:00 AM				2		2			2	
3:00 AM				5		5			5	
4:00 AM				7		7			7	
5:00 AM				68		68			68	
6:00 AM				197		197			197	
7:00 AM				917		917			917	
8:00 AM				1031		1031			1031	
9:00 AM				847		847			847	
10:00 AM				744		744			744	
11:00 AM				669		669			669	
12:00 PM				658		658			658	
1:00 PM				732		732			732	
2:00 PM				936		936			936	
3:00 PM				1067		1067			1067	
4:00 PM				997		997			997	
5:00 PM				1007		1007			1007	
6:00 PM				851		851			851	
7:00 PM				490		490			490	
8:00 PM			386			386			386	
9:00 PM			231			231			231	
10:00 PM			111			111			111	
11:00 PM			60			60			60	
Day Total			788	11264		12052			12052	
% Weekday Average			6.5%	93.5%						
% Week Average			6.5%	93.5%		100.0%				
AM Peak Volume				8:00 AM 1031		8:00 AM 1031			8:00 AM 1031	
PM Peak Volume			8:00 PM 386	3:00 PM 1067		3:00 PM 1067			3:00 PM 1067	
<i>Comments:</i> none										

LOCATION: 85. Valparaiso Ave SPECIFIC LOCATION: 100 ft from Cotton St CITY/STATE: Menlo Park, CA						QC JOB #: 12899467 DIRECTION: EB/WB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				22		22			22	
1:00 AM				21		21			21	
2:00 AM				6		6			6	
3:00 AM				6		6			6	
4:00 AM				13		13			13	
5:00 AM				98		98			98	
6:00 AM				239		239			239	
7:00 AM				986		986			986	
8:00 AM				1162		1162			1162	
9:00 AM				982		982			982	
10:00 AM				932		932			932	
11:00 AM				849		849			849	
12:00 PM				908		908			908	
1:00 PM				904		904			904	
2:00 PM				1154		1154			1154	
3:00 PM				1302		1302			1302	
4:00 PM				1246		1246			1246	
5:00 PM				1219		1219			1219	
6:00 PM				949		949			949	
7:00 PM				569		569			569	
8:00 PM				362		362			362	
9:00 PM				303		303			303	
10:00 PM				135		135			135	
11:00 PM				69		69			69	
Day Total				14436		14436			14436	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				1162		1162			1162	
PM Peak				3:00 PM		3:00 PM			3:00 PM	
Volume				1302		1302			1302	
<i>Comments:</i> none										

LOCATION: 86. Valparaiso Ave SPECIFIC LOCATION: 100 ft from University Dr CITY/STATE: Menlo Park, CA						QC JOB #: 12899468 DIRECTION: EB/WB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				29		29			29	
1:00 AM				20		20			20	
2:00 AM				9		9			9	
3:00 AM				5		5			5	
4:00 AM				12		12			12	
5:00 AM				83		83			83	
6:00 AM				216		216			216	
7:00 AM				817		817			817	
8:00 AM				1026		1026			1026	
9:00 AM				936		936			936	
10:00 AM				842		842			842	
11:00 AM				845		845			845	
12:00 PM				822		822			822	
1:00 PM				833		833			833	
2:00 PM				1076		1076			1076	
3:00 PM				1128		1128			1128	
4:00 PM				1027		1027			1027	
5:00 PM				1057		1057			1057	
6:00 PM				871		871			871	
7:00 PM				521		521			521	
8:00 PM				325		325			325	
9:00 PM				295		295			295	
10:00 PM				148		148			148	
11:00 PM				68		68			68	
Day Total				13011		13011			13011	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				1026		1026			1026	
PM Peak				3:00 PM		3:00 PM			3:00 PM	
Volume				1128		1128			1128	
<i>Comments:</i> none										

LOCATION: 87. Willow Rd SPECIFIC LOCATION: 100 ft from Alma St CITY/STATE: Menlo Park, CA						QC JOB #: 12899469 DIRECTION: EB/WB DATE: Sep 30 2014 - Sep 30 2014				
Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
		30-Sep-14								
12:00 AM		6				6			6	
1:00 AM		2				2			2	
2:00 AM		3				3			3	
3:00 AM		6				6			6	
4:00 AM		8				8			8	
5:00 AM		32				32			32	
6:00 AM		51				51			51	
7:00 AM		169				169			169	
8:00 AM		247				247			247	
9:00 AM		213				213			213	
10:00 AM		216				216			216	
11:00 AM		190				190			190	
12:00 PM		175				175			175	
1:00 PM		234				234			234	
2:00 PM		234				234			234	
3:00 PM		289				289			289	
4:00 PM		299				299			299	
5:00 PM		309				309			309	
6:00 PM		245				245			245	
7:00 PM		188				188			188	
8:00 PM		129				129			129	
9:00 PM		66				66			66	
10:00 PM		42				42			42	
11:00 PM		9				9			9	
Day Total		3362				3362			3362	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		247				247			247	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		309				309			309	
<i>Comments:</i> none										

LOCATION: 88. Willow Rd SPECIFIC LOCATION: 100 ft from Laurel St CITY/STATE: Menlo Park, CA						QC JOB #: 12899470 DIRECTION: EB/WB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
		21-Oct-14								
12:00 AM		7				7			7	
1:00 AM		1				1			1	
2:00 AM		3				3			3	
3:00 AM		8				8			8	
4:00 AM		57				57			57	
5:00 AM		111				111			111	
6:00 AM		295				295			295	
7:00 AM		477				477			477	
8:00 AM		307				307			307	
9:00 AM		277				277			277	
10:00 AM		295				295			295	
11:00 AM		319				319			319	
12:00 PM		315				315			315	
1:00 PM		363				363			363	
2:00 PM		464				464			464	
3:00 PM		505				505			505	
4:00 PM		502				502			502	
5:00 PM		386				386			386	
6:00 PM		227				227			227	
7:00 PM		156				156			156	
8:00 PM		89				89			89	
9:00 PM		51				51			51	
10:00 PM		22				22			22	
11:00 PM		10				10			10	
Day Total		5247				5247			5247	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		7:00 AM				7:00 AM			7:00 AM	
Volume		477				477			477	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		505				505			505	
<i>Comments:</i> none										

LOCATION: 89. Willow Rd SPECIFIC LOCATION: 100 ft from Middlefield Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899471 DIRECTION: EB/WB DATE: Oct 21 2014 - Oct 21 2014				
Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
		21-Oct-14								
12:00 AM		113				113			113	
1:00 AM		62				62			62	
2:00 AM		52				52			52	
3:00 AM		55				55			55	
4:00 AM		114				114			114	
5:00 AM		495				495			495	
6:00 AM		1084				1084			1084	
7:00 AM		1553				1553			1553	
8:00 AM		1749				1749			1749	
9:00 AM		1580				1580			1580	
10:00 AM		1413				1413			1413	
11:00 AM		1442				1442			1442	
12:00 PM		1487				1487			1487	
1:00 PM		1538				1538			1538	
2:00 PM		1723				1723			1723	
3:00 PM		1485				1485			1485	
4:00 PM		1510				1510			1510	
5:00 PM		1252				1252			1252	
6:00 PM		1393				1393			1393	
7:00 PM		1413				1413			1413	
8:00 PM		1044				1044			1044	
9:00 PM		882				882			882	
10:00 PM		575				575			575	
11:00 PM		318				318			318	
Day Total		24332				24332			24332	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		1749				1749			1749	
PM Peak		2:00 PM				2:00 PM			2:00 PM	
Volume		1723				1723			1723	
<i>Comments:</i> none										

LOCATION: 112. Chilco St SPECIFIC LOCATION: 100 ft from Hamilton Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899483 DIRECTION: EB/WB DATE: Oct 08 2014 - Oct 08 2014				
Start Time	Mon	Tue	Wed 08-Oct-14	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM			17			17			17	
1:00 AM			15			15			15	
2:00 AM			11			11			11	
3:00 AM			11			11			11	
4:00 AM			24			24			24	
5:00 AM			43			43			43	
6:00 AM			134			134			134	
7:00 AM			434			434			434	
8:00 AM			364			364			364	
9:00 AM			212			212			212	
10:00 AM			200			200			200	
11:00 AM			189			189			189	
12:00 PM			238			238			238	
1:00 PM			215			215			215	
2:00 PM			203			203			203	
3:00 PM			258			258			258	
4:00 PM			383			383			383	
5:00 PM			576			576			576	
6:00 PM			495			495			495	
7:00 PM			258			258			258	
8:00 PM			176			176			176	
9:00 PM			168			168			168	
10:00 PM			113			113			113	
11:00 PM			39			39			39	
Day Total			4776			4776			4776	
% Weekday Average			100.0%							
% Week Average			100.0%			100.0%				
AM Peak			7:00 AM			7:00 AM			7:00 AM	
Volume			434			434			434	
PM Peak			5:00 PM			5:00 PM			5:00 PM	
Volume			576			576			576	
<i>Comments:</i> none										

LOCATION: 113. Chilco St SPECIFIC LOCATION: 100 ft from Ivy Dr CITY/STATE: Menlo Park, CA						QC JOB #: 12899484 DIRECTION: EB/WB DATE: Oct 08 2014 - Oct 08 2014				
Start Time	Mon	Tue	Wed 08-Oct-14	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM			8			8			8	
1:00 AM			9			9			9	
2:00 AM			3			3			3	
3:00 AM			7			7			7	
4:00 AM			3			3			3	
5:00 AM			16			16			16	
6:00 AM			63			63			63	
7:00 AM			196			196			196	
8:00 AM			202			202			202	
9:00 AM			108			108			108	
10:00 AM			77			77			77	
11:00 AM			103			103			103	
12:00 PM			105			105			105	
1:00 PM			136			136			136	
2:00 PM			115			115			115	
3:00 PM			154			154			154	
4:00 PM			258			258			258	
5:00 PM			390			390			390	
6:00 PM			337			337			337	
7:00 PM			146			146			146	
8:00 PM			80			80			80	
9:00 PM			65			65			65	
10:00 PM			53			53			53	
11:00 PM			20			20			20	
Day Total			2654			2654			2654	
% Weekday Average			100.0%							
% Week Average			100.0%			100.0%				
AM Peak			8:00 AM			8:00 AM			8:00 AM	
Volume			202			202			202	
PM Peak			5:00 PM			5:00 PM			5:00 PM	
Volume			390			390			390	
<i>Comments:</i> none										

LOCATION: 114. Chilco St SPECIFIC LOCATION: 100 ft from Newbridge St CITY/STATE: Menlo Park, CA							QC JOB #: 12899485 DIRECTION: EB/WB DATE: Oct 08 2014 - Oct 08 2014			
Start Time	Mon	Tue	Wed 08-Oct-14	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM			8			8			8	
1:00 AM			13			13			13	
2:00 AM			3			3			3	
3:00 AM			7			7			7	
4:00 AM			8			8			8	
5:00 AM			10			10			10	
6:00 AM			76			76			76	
7:00 AM			151			151			151	
8:00 AM			190			190			190	
9:00 AM			82			82			82	
10:00 AM			66			66			66	
11:00 AM			87			87			87	
12:00 PM			87			87			87	
1:00 PM			148			148			148	
2:00 PM			100			100			100	
3:00 PM			121			121			121	
4:00 PM			177			177			177	
5:00 PM			251			251			251	
6:00 PM			236			236			236	
7:00 PM			112			112			112	
8:00 PM			69			69			69	
9:00 PM			53			53			53	
10:00 PM			40			40			40	
11:00 PM			19			19			19	
Day Total			2114			2114			2114	
% Weekday Average			100.0%							
% Week Average			100.0%			100.0%				
AM Peak			8:00 AM			8:00 AM			8:00 AM	
Volume			190			190			190	
PM Peak			5:00 PM			5:00 PM			5:00 PM	
Volume			251			251			251	
<i>Comments:</i> none										

LOCATION: 119. Hamilton Ave SPECIFIC LOCATION: 100 ft from Willow Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899486 DIRECTION: NB/SB DATE: Oct 08 2014 - Oct 08 2014				
Start Time	Mon	Tue	Wed 08-Oct-14	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM			14			14			14	
1:00 AM			11			11			11	
2:00 AM			12			12			12	
3:00 AM			44			44			44	
4:00 AM			20			20			20	
5:00 AM			95			95			95	
6:00 AM			103			103			103	
7:00 AM			186			186			186	
8:00 AM			215			215			215	
9:00 AM			187			187			187	
10:00 AM			167			167			167	
11:00 AM			165			165			165	
12:00 PM			200			200			200	
1:00 PM			145			145			145	
2:00 PM			201			201			201	
3:00 PM			137			137			137	
4:00 PM			208			208			208	
5:00 PM			213			213			213	
6:00 PM			146			146			146	
7:00 PM			75			75			75	
8:00 PM			42			42			42	
9:00 PM			28			28			28	
10:00 PM			18			18			18	
11:00 PM			11			11			11	
Day Total			2643			2643			2643	
% Weekday Average			100.0%							
% Week Average			100.0%			100.0%				
AM Peak			8:00 AM			8:00 AM			8:00 AM	
Volume			215			215			215	
PM Peak			5:00 PM			5:00 PM			5:00 PM	
Volume			213			213			213	
<i>Comments:</i> none										

LOCATION: 120. Willow Rd SPECIFIC LOCATION: 100 ft from Gilbert Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899487 DIRECTION: EB/WB DATE: Oct 14 2014 - Oct 14 2014				
Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday	Sat	Sun	Average Week	Average Week Profile
	14-Oct-14					Hourly Traffic			Hourly Traffic	
12:00 AM		101				101			101	
1:00 AM		50				50			50	
2:00 AM		30				30			30	
3:00 AM		53				53			53	
4:00 AM		123				123			123	
5:00 AM		454				454			454	
6:00 AM		1030				1030			1030	
7:00 AM		1478				1478			1478	
8:00 AM		1612				1612			1612	
9:00 AM		1696				1696			1696	
10:00 AM		1440				1440			1440	
11:00 AM		1401				1401			1401	
12:00 PM		1433				1433			1433	
1:00 PM		1482				1482			1482	
2:00 PM		1718				1718			1718	
3:00 PM		1634				1634			1634	
4:00 PM		1604				1604			1604	
5:00 PM		1614				1614			1614	
6:00 PM		1629				1629			1629	
7:00 PM		1362				1362			1362	
8:00 PM		916				916			916	
9:00 PM		760				760			760	
10:00 PM		487				487			487	
11:00 PM		246				246			246	
Day Total		24353				24353			24353	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		9:00 AM				9:00 AM			9:00 AM	
Volume		1696				1696			1696	
PM Peak		2:00 PM				2:00 PM			2:00 PM	
Volume		1718				1718			1718	
<i>Comments:</i> none										

LOCATION: 121. Willow Rd SPECIFIC LOCATION: 100 ft from Coleman Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899488 DIRECTION: EB/WB DATE: Sep 30 2014 - Sep 30 2014				
Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
		30-Sep-14								
12:00 AM		195				195			195	
1:00 AM		75				75			75	
2:00 AM		92				92			92	
3:00 AM		96				96			96	
4:00 AM		219				219			219	
5:00 AM		762				762			762	
6:00 AM		1893				1893			1893	
7:00 AM		2649				2649			2649	
8:00 AM		2924				2924			2924	
9:00 AM		2676				2676			2676	
10:00 AM		2482				2482			2482	
11:00 AM		2518				2518			2518	
12:00 PM		2526				2526			2526	
1:00 PM		2531				2531			2531	
2:00 PM		2760				2760			2760	
3:00 PM		2606				2606			2606	
4:00 PM		2372				2372			2372	
5:00 PM		2168				2168			2168	
6:00 PM		2508				2508			2508	
7:00 PM		2468				2468			2468	
8:00 PM		1726				1726			1726	
9:00 PM		1418				1418			1418	
10:00 PM		989				989			989	
11:00 PM		535				535			535	
Day Total		41188				41188			41188	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		2924				2924			2924	
PM Peak		2:00 PM				2:00 PM			2:00 PM	
Volume		2760				2760			2760	
<i>Comments:</i> none										

LOCATION: 122. Willow Rd SPECIFIC LOCATION: 100 ft from Durham St CITY/STATE: Menlo Park, CA						QC JOB #: 12899489 DIRECTION: EB/WB DATE: Oct 14 2014 - Oct 14 2014				
Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
		14-Oct-14								
12:00 AM		191				191			191	
1:00 AM		73				73			73	
2:00 AM		70				70			70	
3:00 AM		66				66			66	
4:00 AM		179				179			179	
5:00 AM		626				626			626	
6:00 AM		1604				1604			1604	
7:00 AM		2302				2302			2302	
8:00 AM		2463				2463			2463	
9:00 AM		2286				2286			2286	
10:00 AM		2098				2098			2098	
11:00 AM		2070				2070			2070	
12:00 PM		1977				1977			1977	
1:00 PM		2018				2018			2018	
2:00 PM		2214				2214			2214	
3:00 PM		2264				2264			2264	
4:00 PM		2198				2198			2198	
5:00 PM		2104				2104			2104	
6:00 PM		2179				2179			2179	
7:00 PM		1870				1870			1870	
8:00 PM		1245				1245			1245	
9:00 PM		984				984			984	
10:00 PM		648				648			648	
11:00 PM		418				418			418	
Day Total		34147				34147			34147	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		2463				2463			2463	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		2264				2264			2264	
<i>Comments:</i> none										

LOCATION: 123. Chilco St SPECIFIC LOCATION: 100 ft from Terminal Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899490 DIRECTION: EB/WB DATE: Oct 14 2014 - Oct 14 2014				
Start Time	Mon	Tue 14-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		34				34			34	
1:00 AM		11				11			11	
2:00 AM		7				7			7	
3:00 AM		9				9			9	
4:00 AM		13				13			13	
5:00 AM		61				61			61	
6:00 AM		127				127			127	
7:00 AM		442				442			442	
8:00 AM		401				401			401	
9:00 AM		243				243			243	
10:00 AM		181				181			181	
11:00 AM		186				186			186	
12:00 PM		218				218			218	
1:00 PM		221				221			221	
2:00 PM		244				244			244	
3:00 PM		372				372			372	
4:00 PM		434				434			434	
5:00 PM		756				756			756	
6:00 PM		419				419			419	
7:00 PM		273				273			273	
8:00 PM		173				173			173	
9:00 PM		127				127			127	
10:00 PM		99				99			99	
11:00 PM		52				52			52	
Day Total		5103				5103			5103	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		7:00 AM				7:00 AM			7:00 AM	
Volume		442				442			442	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		756				756			756	
<i>Comments:</i> none										

LOCATION: 124. Chrysler Dr **QC JOB #:** 12899491
SPECIFIC LOCATION: 100 ft from Constitution Dr **DIRECTION:** EB/WB
CITY/STATE: Menlo Park, CA **DATE:** Oct 15 2014 - Oct 15 2014

Start Time	Mon	Tue	Wed 15-Oct-14	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM			17			17			17	
1:00 AM			6			6			6	
2:00 AM			12			12			12	
3:00 AM			8			8			8	
4:00 AM			13			13			13	
5:00 AM			83			83			83	
6:00 AM			125			125			125	
7:00 AM			181			181			181	
8:00 AM			247			247			247	
9:00 AM			258			258			258	
10:00 AM			192			192			192	
11:00 AM			202			202			202	
12:00 PM			219			219			219	
1:00 PM			193			193			193	
2:00 PM			162			162			162	
3:00 PM			248			248			248	
4:00 PM			349			349			349	
5:00 PM			347			347			347	
6:00 PM			205			205			205	
7:00 PM			99			99			99	
8:00 PM			35			35			35	
9:00 PM			26			26			26	
10:00 PM			23			23			23	
11:00 PM			19			19			19	
Day Total			3269			3269			3269	
% Weekday Average			100.0%							
% Week Average			100.0%			100.0%				
AM Peak			9:00 AM			9:00 AM			9:00 AM	
Volume			258			258			258	
PM Peak			4:00 PM			4:00 PM			4:00 PM	
Volume			349			349			349	

Comments: none

LOCATION: 125. Chrysler Dr SPECIFIC LOCATION: 100 ft from Independence Dr CITY/STATE: Menlo Park, CA						QC JOB #: 12899492 DIRECTION: EB/WB DATE: Oct 14 2014 - Oct 14 2014				
Start Time	Mon	Tue 14-Oct-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		2				2			2	
1:00 AM		0				0			0	
2:00 AM		3				3			3	
3:00 AM		3				3			3	
4:00 AM		10				10			10	
5:00 AM		3				3			3	
6:00 AM		37				37			37	
7:00 AM		72				72			72	
8:00 AM		154				154			154	
9:00 AM		139				139			139	
10:00 AM		56				56			56	
11:00 AM		53				53			53	
12:00 PM		64				64			64	
1:00 PM		67				67			67	
2:00 PM		34				34			34	
3:00 PM		52				52			52	
4:00 PM		85				85			85	
5:00 PM		108				108			108	
6:00 PM		82				82			82	
7:00 PM		46				46			46	
8:00 PM		15				15			15	
9:00 PM		15				15			15	
10:00 PM		4				4			4	
11:00 PM		6				6			6	
Day Total		1110				1110			1110	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		154				154			154	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		108				108			108	
<i>Comments:</i> none										

LOCATION: 126. Adams Dr SPECIFIC LOCATION: 100 ft from University Dr CITY/STATE: Menlo Park, CA						QC JOB #: 12899493 DIRECTION: NB/SB DATE: Oct 08 2014 - Oct 08 2014				
Start Time	Mon	Tue	Wed 08-Oct-14	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM			14			14			14	
1:00 AM			9			9			9	
2:00 AM			7			7			7	
3:00 AM			36			36			36	
4:00 AM			13			13			13	
5:00 AM			32			32			32	
6:00 AM			73			73			73	
7:00 AM			113			113			113	
8:00 AM			161			161			161	
9:00 AM			89			89			89	
10:00 AM			61			61			61	
11:00 AM			51			51			51	
12:00 PM			64			64			64	
1:00 PM			65			65			65	
2:00 PM			74			74			74	
3:00 PM			62			62			62	
4:00 PM			53			53			53	
5:00 PM			64			64			64	
6:00 PM			63			63			63	
7:00 PM			59			59			59	
8:00 PM			45			45			45	
9:00 PM			26			26			26	
10:00 PM			19			19			19	
11:00 PM			10			10			10	
Day Total			1263			1263			1263	
% Weekday Average			100.0%							
% Week Average			100.0%			100.0%				
AM Peak			8:00 AM			8:00 AM			8:00 AM	
Volume			161			161			161	
PM Peak			2:00 PM			2:00 PM			2:00 PM	
Volume			74			74			74	
<i>Comments:</i> none										

LOCATION: 133. Olive St SPECIFIC LOCATION: 100 ft from Santa Cruz Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899494 DIRECTION: NB/SB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		4				4			4	
1:00 AM		0				0			0	
2:00 AM		0				0			0	
3:00 AM		2				2			2	
4:00 AM		0				0			0	
5:00 AM		6				6			6	
6:00 AM		24				24			24	
7:00 AM		144				144			144	
8:00 AM		223				223			223	
9:00 AM		170				170			170	
10:00 AM		167				167			167	
11:00 AM		169				169			169	
12:00 PM		151				151			151	
1:00 PM		152				152			152	
2:00 PM		148				148			148	
3:00 PM		236				236			236	
4:00 PM		159				159			159	
5:00 PM		219				219			219	
6:00 PM		197				197			197	
7:00 PM		111				111			111	
8:00 PM		79				79			79	
9:00 PM		56				56			56	
10:00 PM		23				23			23	
11:00 PM		9				9			9	
Day Total		2449				2449			2449	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		223				223			223	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		236				236			236	
<i>Comments:</i> none										

LOCATION: 134. Olive St SPECIFIC LOCATION: 100 ft from Middle Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899495 DIRECTION: NB/SB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		5				5			5	
1:00 AM		5				5			5	
2:00 AM		0				0			0	
3:00 AM		1				1			1	
4:00 AM		4				4			4	
5:00 AM		16				16			16	
6:00 AM		33				33			33	
7:00 AM		183				183			183	
8:00 AM		374				374			374	
9:00 AM		209				209			209	
10:00 AM		166				166			166	
11:00 AM		164				164			164	
12:00 PM		161				161			161	
1:00 PM		175				175			175	
2:00 PM		242				242			242	
3:00 PM		353				353			353	
4:00 PM		250				250			250	
5:00 PM		226				226			226	
6:00 PM		211				211			211	
7:00 PM		133				133			133	
8:00 PM		78				78			78	
9:00 PM		36				36			36	
10:00 PM		21				21			21	
11:00 PM		5				5			5	
Day Total		3051				3051			3051	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		374				374			374	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		353				353			353	
<i>Comments:</i> none										

LOCATION: 135. Cambridge Ave SPECIFIC LOCATION: 100 ft from University Dr CITY/STATE: Menlo Park, CA						QC JOB #: 12899496 DIRECTION: EB/WB DATE: Sep 23 2014 - Sep 23 2014				
Start Time	Mon	Tue 23-Sep-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		7				7			7	
1:00 AM		0				0			0	
2:00 AM		3				3			3	
3:00 AM		0				0			0	
4:00 AM		1				1			1	
5:00 AM		3				3			3	
6:00 AM		11				11			11	
7:00 AM		61				61			61	
8:00 AM		112				112			112	
9:00 AM		95				95			95	
10:00 AM		92				92			92	
11:00 AM		97				97			97	
12:00 PM		86				86			86	
1:00 PM		105				105			105	
2:00 PM		78				78			78	
3:00 PM		107				107			107	
4:00 PM		126				126			126	
5:00 PM		264				264			264	
6:00 PM		158				158			158	
7:00 PM		84				84			84	
8:00 PM		40				40			40	
9:00 PM		49				49			49	
10:00 PM		19				19			19	
11:00 PM		5				5			5	
Day Total		1603				1603			1603	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		112				112			112	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		264				264			264	
Comments: none										

LOCATION: 136. Linfield Dr SPECIFIC LOCATION: 100 ft from Middlefield Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899497 DIRECTION: EB/WB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				8		8			8	
1:00 AM				6		6			6	
2:00 AM				1		1			1	
3:00 AM				3		3			3	
4:00 AM				12		12			12	
5:00 AM				23		23			23	
6:00 AM				54		54			54	
7:00 AM				104		104			104	
8:00 AM				143		143			143	
9:00 AM				126		126			126	
10:00 AM				91		91			91	
11:00 AM				94		94			94	
12:00 PM				125		125			125	
1:00 PM				97		97			97	
2:00 PM				103		103			103	
3:00 PM				161		161			161	
4:00 PM				137		137			137	
5:00 PM				160		160			160	
6:00 PM				105		105			105	
7:00 PM				71		71			71	
8:00 PM				44		44			44	
9:00 PM				39		39			39	
10:00 PM				39		39			39	
11:00 PM				14		14			14	
Day Total				1760		1760			1760	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				143		143			143	
PM Peak				3:00 PM		3:00 PM			3:00 PM	
Volume				161		161			161	
<i>Comments:</i> none										

LOCATION: 137. Waverly St SPECIFIC LOCATION: 100 ft from Laurel St CITY/STATE: Menlo Park, CA						QC JOB #: 12899498 DIRECTION: NB/SB DATE: Oct 16 2014 - Oct 16 2014				
Start Time	Mon	Tue	Wed	Thu 16-Oct-14	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM				2		2			2	
1:00 AM				4		4			4	
2:00 AM				4		4			4	
3:00 AM				2		2			2	
4:00 AM				6		6			6	
5:00 AM				21		21			21	
6:00 AM				50		50			50	
7:00 AM				114		114			114	
8:00 AM				155		155			155	
9:00 AM				105		105			105	
10:00 AM				106		106			106	
11:00 AM				78		78			78	
12:00 PM				100		100			100	
1:00 PM				98		98			98	
2:00 PM				92		92			92	
3:00 PM				138		138			138	
4:00 PM				122		122			122	
5:00 PM				153		153			153	
6:00 PM				121		121			121	
7:00 PM				67		67			67	
8:00 PM				41		41			41	
9:00 PM				33		33			33	
10:00 PM				28		28			28	
11:00 PM				12		12			12	
Day Total				1652		1652			1652	
% Weekday Average				100.0%						
% Week Average				100.0%		100.0%				
AM Peak				8:00 AM		8:00 AM			8:00 AM	
Volume				155		155			155	
PM Peak				5:00 PM		5:00 PM			5:00 PM	
Volume				153		153			153	
<i>Comments:</i> none										

LOCATION: 30. Hamilton Ave SPECIFIC LOCATION: 100 ft from Willow Rd CITY/STATE: Menlo Park, CA						QC JOB #: 12899421 DIRECTION: NB/SB DATE: Dec 09 2014 - Dec 09 2014				
Start Time	Mon	Tue 09-Dec-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		25				25			25	
1:00 AM		8				8			8	
2:00 AM		9				9			9	
3:00 AM		5				5			5	
4:00 AM		12				12			12	
5:00 AM		31				31			31	
6:00 AM		82				82			82	
7:00 AM		216				216			216	
8:00 AM		280				280			280	
9:00 AM		118				118			118	
10:00 AM		101				101			101	
11:00 AM		151				151			151	
12:00 PM		130				130			130	
1:00 PM		112				112			112	
2:00 PM		135				135			135	
3:00 PM		183				183			183	
4:00 PM		186				186			186	
5:00 PM		292				292			292	
6:00 PM		285				285			285	
7:00 PM		128				128			128	
8:00 PM		96				96			96	
9:00 PM		106				106			106	
10:00 PM		52				52			52	
11:00 PM		30				30			30	
Day Total		2773				2773			2773	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		280				280			280	
PM Peak		5:00 PM				5:00 PM			5:00 PM	
Volume		292				292			292	
<i>Comments:</i> none										

LOCATION: 43. Menlo Ave SPECIFIC LOCATION: 100 ft from University Ave CITY/STATE: Menlo Park, CA						QC JOB #: 12899430 DIRECTION: EB/WB DATE: Dec 09 2014 - Dec 09 2014				
Start Time	Mon	Tue 09-Dec-14	Wed	Thu	Fri	Average Weekday Hourly Traffic	Sat	Sun	Average Week Hourly Traffic	Average Week Profile
12:00 AM		9				9			9	
1:00 AM		3				3			3	
2:00 AM		5				5			5	
3:00 AM		0				0			0	
4:00 AM		14				14			14	
5:00 AM		33				33			33	
6:00 AM		105				105			105	
7:00 AM		322				322			322	
8:00 AM		605				605			605	
9:00 AM		451				451			451	
10:00 AM		450				450			450	
11:00 AM		434				434			434	
12:00 PM		499				499			499	
1:00 PM		512				512			512	
2:00 PM		510				510			510	
3:00 PM		719				719			719	
4:00 PM		702				702			702	
5:00 PM		611				611			611	
6:00 PM		530				530			530	
7:00 PM		370				370			370	
8:00 PM		239				239			239	
9:00 PM		139				139			139	
10:00 PM		68				68			68	
11:00 PM		30				30			30	
Day Total		7360				7360			7360	
% Weekday Average		100.0%								
% Week Average		100.0%				100.0%				
AM Peak		8:00 AM				8:00 AM			8:00 AM	
Volume		605				605			605	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
Volume		719				719			719	
<i>Comments:</i> none										

A P P E N D I X D

BRIDGE COUNTS





7409 SW Tech Center Dr, Ste B150
 Tigard, OR 97223
 971-223-0003
www.qualitycounts.net

Site Code: 13113009
 Location: Pierce Rd & Ringwood Overcrossing
 Date: 10/14/2014

	Pedestrians		Bikes		Interval Total	Hour Total
	WB	EB	WB	EB		
0:00	0	0	0	0	0	
0:15	0	0	0	0	0	
0:30	0	0	0	0	0	
0:45	0	0	0	0	0	0
1:00	0	0	0	0	0	0
1:15	0	0	0	0	0	0
1:30	0	0	0	0	0	0
1:45	0	0	0	0	0	0
2:00	0	0	0	0	0	0
2:15	0	0	0	0	0	0
2:30	0	0	0	0	0	0
2:45	0	0	0	0	0	0
3:00	0	0	0	0	0	0
3:15	0	0	0	0	0	0
3:30	0	0	0	0	0	0
3:45	0	0	0	0	0	0
4:00	0	0	0	0	0	0
4:15	0	0	0	0	0	0
4:30	0	0	0	0	0	0
4:45	0	0	0	0	0	0
5:00	0	0	0	0	0	0
5:15	0	0	0	0	0	0
5:30	0	0	0	0	0	0
5:45	0	0	0	0	0	0
6:00	0	0	0	0	0	0
6:15	0	0	0	0	0	0
6:30	0	2	1	0	3	3
6:45	0	1	3	3	7	10
7:00	0	1	2	0	3	13
7:15	1	3	3	7	14	27
7:30	0	2	3	7	12	36
7:45	1	4	5	3	13	42
8:00	3	11	6	5	25	64
8:15	2	2	9	2	15	65
8:30	2	17	10	16	45	98
8:45	1	10	10	8	29	114
9:00	3	17	6	13	39	128
9:15	0	2	4	5	11	124
9:30	1	0	5	1	7	86
9:45	1	1	5	5	12	69
10:00	1	1	2	1	5	35
10:15	0	1	1	0	2	26
10:30	0	0	4	3	7	26
10:45	3	0	1	1	5	19
11:00	1	0	0	2	3	17
11:15	0	0	1	1	2	17
11:30	0	0	0	0	0	10
11:45	2	0	0	0	2	7

	Pedestrians		Bikes		Interval Total	Hour Total
	WB	EB	WB	EB		
12:00	0	0	2	0	2	6
12:15	0	0	1	1	2	6
12:30	0	0	0	0	0	6
12:45	0	1	0	0	1	5
13:00	0	2	0	1	3	6
13:15	0	0	0	2	2	6
13:30	0	0	1	1	2	8
13:45	0	1	1	1	3	10
14:00	1	3	1	0	5	12
14:15	0	0	0	0	0	10
14:30	0	1	2	3	6	14
14:45	0	2	0	3	5	16
15:00	2	1	1	0	4	15
15:15	0	2	1	9	12	27
15:30	2	51	1	8	62	83
15:45	1	31	3	5	40	118
16:00	2	7	0	0	9	123
16:15	4	5	2	2	13	124
16:30	0	0	2	1	3	65
16:45	0	3	1	3	7	32
17:00	0	3	7	9	19	42
17:15	0	5	9	5	19	48
17:30	0	1	2	6	9	54
17:45	0	4	6	1	11	58
18:00	2	2	3	2	9	48
18:15	0	2	6	3	11	40
18:30	1	2	3	2	8	39
18:45	0	1	7	3	11	39
19:00	0	5	6	2	13	43
19:15	1	1	4	0	6	38
19:30	0	1	4	0	5	35
19:45	2	0	3	0	5	29
20:00	0	0	3	0	3	19
20:15	2	2	1	0	5	18
20:30	0	0	0	0	0	13
20:45	0	0	0	0	0	8
21:00	0	0	0	0	0	5
21:15	0	0	1	0	1	1
21:30	0	0	1	0	1	2
21:45	0	0	2	2	4	6
22:00	0	0	1	1	2	8
22:15	0	0	2	1	3	10
22:30	0	0	0	2	2	11
22:45	0	0	0	2	2	9
23:00	0	0	0	0	0	7
23:15	0	0	0	0	0	4
23:30	0	0	0	0	0	2
23:45	0	1	0	0	1	1
Total	42	215	171	164		



7409 SW Tech Center Dr, Ste B150
 Tigard, OR 97223
 971-223-0003
www.qualitycounts.net

Site Code: 13113010
 Location: Willow Pl Bike Bridge
 Date: 10/14/2014

	Pedestrians		Bikes		Interval Total	Hour Total
	SB	NB	SB	NB		
0:00	0	0	0	2	2	
0:15	0	0	0	0	0	
0:30	0	0	0	0	0	
0:45	0	0	0	1	1	3
1:00	0	0	0	0	0	1
1:15	0	0	0	0	0	1
1:30	0	0	0	1	1	2
1:45	0	1	0	0	1	2
2:00	0	0	0	0	0	2
2:15	0	0	0	0	0	2
2:30	0	0	0	0	0	1
2:45	0	0	0	0	0	0
3:00	0	0	0	0	0	0
3:15	0	0	0	0	0	0
3:30	0	0	0	0	0	0
3:45	0	0	0	0	0	0
4:00	0	0	0	0	0	0
4:15	0	0	0	0	0	0
4:30	0	0	1	0	1	1
4:45	0	0	0	0	0	1
5:00	0	1	0	0	1	2
5:15	0	0	0	0	0	2
5:30	0	0	1	0	1	2
5:45	2	0	2	0	4	6
6:00	1	0	5	0	6	11
6:15	3	3	4	1	11	22
6:30	1	0	2	3	6	27
6:45	1	1	4	2	8	31
7:00	1	1	3	3	8	33
7:15	4	5	7	9	25	47
7:30	2	8	15	5	30	71
7:45	2	3	14	7	26	89
8:00	5	3	18	10	36	117
8:15	0	6	9	10	25	117
8:30	4	2	18	11	35	122
8:45	8	6	22	4	40	136
9:00	10	15	45	16	86	186
9:15	6	3	12	10	31	192
9:30	1	6	23	3	33	190
9:45	5	4	7	5	21	171
10:00	3	1	6	6	16	101
10:15	2	2	11	2	17	87
10:30	5	1	6	0	12	66
10:45	1	5	3	4	13	58
11:00	2	2	5	3	12	54
11:15	5	2	3	1	11	48
11:30	3	4	2	4	13	49
11:45	6	0	1	0	7	43

	Pedestrians		Bikes		Interval Total	Hour Total
	SB	NB	SB	NB		
12:00	5	5	2	5	17	48
12:15	4	2	5	4	15	52
12:30	2	4	4	4	14	53
12:45	1	8	1	4	14	60
13:00	3	4	2	1	10	53
13:15	1	6	3	2	12	50
13:30	2	4	1	1	8	44
13:45	3	5	3	4	15	45
14:00	1	5	2	2	10	45
14:15	3	0	1	4	8	41
14:30	6	1	1	2	10	43
14:45	1	5	1	3	10	38
15:00	3	1	4	3	11	39
15:15	4	2	4	5	15	46
15:30	4	3	3	8	18	54
15:45	1	4	4	4	13	57
16:00	5	2	8	7	22	68
16:15	2	1	8	5	16	69
16:30	6	7	6	6	25	76
16:45	8	4	3	7	22	85
17:00	3	2	6	9	20	83
17:15	3	1	5	9	18	85
17:30	3	1	8	14	26	86
17:45	4	2	10	16	32	96
18:00	0	2	11	13	26	102
18:15	4	2	6	16	28	112
18:30	4	4	6	16	30	116
18:45	1	5	9	16	31	115
19:00	2	5	3	12	22	111
19:15	1	1	4	9	15	98
19:30	5	1	2	10	18	86
19:45	2	1	3	4	10	65
20:00	1	2	0	2	5	48
20:15	0	1	4	5	10	43
20:30	0	2	3	3	8	33
20:45	0	0	1	3	4	27
21:00	0	3	2	3	8	30
21:15	0	3	0	2	5	25
21:30	0	0	2	2	4	21
21:45	0	1	1	2	4	21
22:00	0	2	0	3	5	18
22:15	0	5	0	2	7	20
22:30	0	0	0	1	1	17
22:45	0	1	2	2	5	18
23:00	0	0	0	1	1	14
23:15	0	0	0	1	1	8
23:30	1	1	0	0	2	9
23:45	0	1	0	1	2	6
Total	182	207	403	381		



7409 SW Tech Center Dr, Ste B150
 Tigard, OR 97223
 971-223-0003
www.qualitycounts.net

Site Code: 13113011
 Location: San Mateo Bike Bridge
 Date: 10/21/2014

	Pedestrians		Bikes		Interval Total	Hour Total
	NB	SB	NB	SB		
0:00	0	0	0	0	0	
0:15	0	0	0	0	0	
0:30	0	0	0	0	0	
0:45	0	0	0	0	0	0
1:00	0	0	0	0	0	0
1:15	0	0	0	0	0	0
1:30	0	0	0	0	0	0
1:45	0	0	0	0	0	0
2:00	0	0	0	0	0	0
2:15	0	0	0	0	0	0
2:30	0	0	0	0	0	0
2:45	0	0	0	0	0	0
3:00	0	0	0	0	0	0
3:15	0	0	0	0	0	0
3:30	0	0	0	0	0	0
3:45	0	0	0	0	0	0
4:00	0	0	0	0	0	0
4:15	0	0	0	0	0	0
4:30	0	0	0	0	0	0
4:45	0	0	0	0	0	0
5:00	0	0	1	1	2	2
5:15	0	0	0	0	0	2
5:30	0	0	3	1	4	6
5:45	0	0	0	0	0	6
6:00	0	0	1	0	1	5
6:15	0	0	1	2	3	8
6:30	1	1	0	0	2	6
6:45	0	0	2	3	5	11
7:00	0	0	1	1	2	12
7:15	0	1	2	1	4	13
7:30	0	0	1	0	1	12
7:45	0	1	6	1	8	15
8:00	0	0	8	0	8	21
8:15	0	0	3	3	6	23
8:30	0	0	5	1	6	28
8:45	0	1	5	1	7	27
9:00	0	0	2	0	2	21
9:15	1	2	5	2	10	25
9:30	0	0	7	0	7	26
9:45	2	0	3	1	6	25
10:00	0	0	2	0	2	25
10:15	0	2	1	1	4	19
10:30	0	0	0	0	0	12
10:45	0	0	2	0	2	8
11:00	0	0	0	0	0	6
11:15	0	0	2	0	2	4
11:30	0	0	1	0	1	5
11:45	0	0	0	0	0	3

	Pedestrians		Bikes		Interval Total	Hour Total
	NB	SB	NB	SB		
12:00	1	0	0	0	1	4
12:15	0	0	0	0	0	2
12:30	0	0	0	1	1	2
12:45	1	0	0	2	3	5
13:00	0	0	0	0	0	4
13:15	2	0	2	0	4	8
13:30	0	0	0	0	0	7
13:45	0	2	0	0	2	6
14:00	0	0	0	0	0	6
14:15	0	0	0	0	0	2
14:30	0	0	0	0	0	2
14:45	0	1	0	1	2	2
15:00	0	0	1	2	3	5
15:15	0	0	0	0	0	5
15:30	0	0	0	1	1	6
15:45	0	0	0	1	1	5
16:00	0	2	0	0	2	4
16:15	2	0	0	2	4	8
16:30	0	0	2	1	3	10
16:45	0	0	0	0	0	9
17:00	0	0	1	1	2	9
17:15	0	0	3	5	8	13
17:30	0	0	0	9	9	19
17:45	0	0	3	10	13	32
18:00	0	1	0	3	4	34
18:15	0	0	4	5	9	35
18:30	0	1	0	4	5	31
18:45	2	0	0	2	4	22
19:00	0	0	1	3	4	22
19:15	0	0	0	2	2	15
19:30	0	0	0	0	0	10
19:45	0	0	0	0	0	6
20:00	0	0	0	1	1	3
20:15	1	0	0	1	2	3
20:30	0	0	1	0	1	4
20:45	0	0	0	0	0	4
21:00	0	0	0	0	0	3
21:15	0	0	0	0	0	1
21:30	0	1	0	0	1	1
21:45	0	0	0	1	1	2
22:00	0	0	0	0	0	2
22:15	0	0	0	0	0	2
22:30	0	0	0	0	0	1
22:45	0	0	0	0	0	0
23:00	0	0	0	0	0	0
23:15	0	0	0	0	0	0
23:30	0	0	0	0	0	0
23:45	0	0	0	0	0	0
Total	13	16	82	77		



7409 SW Tech Center Dr, Ste B150
 Tigard, OR 97223
 971-223-0003
www.qualitycounts.net

Site Code: 13113012
 Location: Alma St Bike Bridge
 Date: 10/14/2014

	Pedestrians		Bikes		Interval Total	Hour Total
	SB	NB	SB	NB		
0:00	0	0	0	2	2	
0:15	0	0	0	1	1	
0:30	0	1	0	1	2	
0:45	1	1	0	0	2	7
1:00	0	0	0	1	1	6
1:15	0	0	0	0	0	5
1:30	0	0	0	0	0	3
1:45	0	0	0	0	0	1
2:00	0	0	0	0	0	0
2:15	0	0	0	0	0	0
2:30	0	0	0	0	0	0
2:45	0	0	0	0	0	0
3:00	0	0	0	0	0	0
3:15	0	0	0	0	0	0
3:30	0	0	0	0	0	0
3:45	0	0	0	0	0	0
4:00	0	0	0	0	0	0
4:15	0	0	0	0	0	0
4:30	0	0	0	0	0	0
4:45	0	0	0	0	0	0
5:00	0	0	0	0	0	0
5:15	0	0	2	0	2	2
5:30	1	2	0	1	4	6
5:45	1	0	2	0	3	9
6:00	2	3	2	0	7	16
6:15	14	3	4	1	22	36
6:30	1	0	0	0	1	33
6:45	5	4	5	2	16	46
7:00	3	2	2	2	9	48
7:15	7	0	2	7	16	42
7:30	4	4	7	7	22	63
7:45	9	2	7	5	23	70
8:00	3	7	10	0	20	81
8:15	7	3	9	3	22	87
8:30	6	6	14	6	32	97
8:45	1	1	14	7	23	97
9:00	9	2	16	3	30	107
9:15	1	4	6	5	16	101
9:30	10	1	10	5	26	95
9:45	2	1	10	9	22	94
10:00	6	0	11	1	18	82
10:15	6	0	4	3	13	79
10:30	2	1	5	4	12	65
10:45	4	1	4	0	9	52
11:00	1	4	2	1	8	42
11:15	4	1	1	2	8	37
11:30	8	6	3	2	19	44
11:45	4	1	3	2	10	45

	Pedestrians		Bikes		Interval Total	Hour Total
	SB	NB	SB	NB		
12:00	3	3	1	5	12	49
12:15	3	0	2	3	8	49
12:30	2	7	2	4	15	45
12:45	4	4	4	1	13	48
13:00	1	1	1	0	3	39
13:15	4	3	5	2	14	45
13:30	2	4	6	3	15	45
13:45	1	2	4	2	9	41
14:00	2	3	1	4	10	48
14:15	2	1	1	2	6	40
14:30	1	1	2	7	11	36
14:45	4	0	3	5	12	39
15:00	0	0	3	4	7	36
15:15	0	0	6	2	8	38
15:30	3	1	4	4	12	39
15:45	1	1	0	6	8	35
16:00	4	1	2	6	13	41
16:15	1	4	5	4	14	47
16:30	1	1	6	11	19	54
16:45	1	1	5	13	20	66
17:00	3	3	4	14	24	77
17:15	4	2	3	10	19	82
17:30	4	11	10	10	35	98
17:45	1	7	4	12	24	102
18:00	4	7	7	8	26	104
18:15	4	6	3	19	32	117
18:30	5	7	3	9	24	106
18:45	7	8	3	16	34	116
19:00	5	2	4	5	16	106
19:15	2	3	3	8	16	90
19:30	1	6	2	12	21	87
19:45	0	5	2	1	8	61
20:00	1	3	1	6	11	56
20:15	0	1	0	3	4	44
20:30	2	5	1	3	11	34
20:45	1	2	3	4	10	36
21:00	3	3	4	4	14	39
21:15	3	0	0	1	4	39
21:30	2	1	1	1	5	33
21:45	0	1	1	2	4	27
22:00	1	2	1	0	4	17
22:15	3	0	1	2	6	19
22:30	1	0	0	3	4	18
22:45	0	3	1	1	5	19
23:00	0	0	0	2	2	17
23:15	1	0	1	0	2	13
23:30	0	0	0	1	1	10
23:45	0	1	0	1	2	7
Total	220	188	281	329		